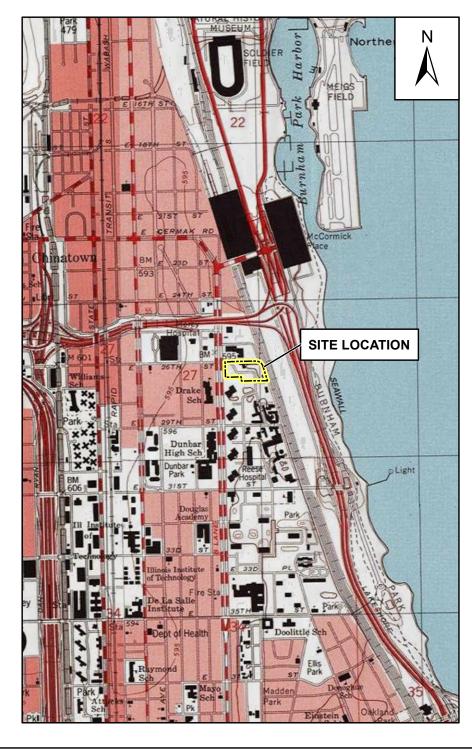
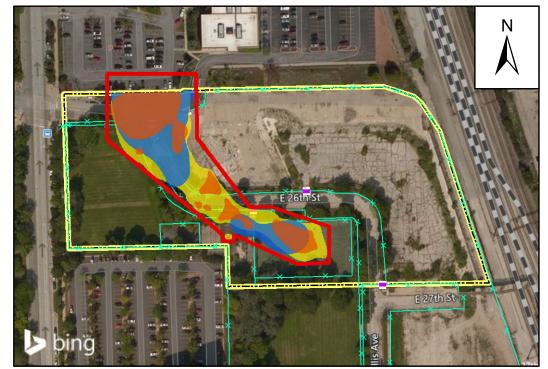


FORMER CARNOTITE REDUCTION COMPANY SITE EXCAVATION PLAN CHICAGO, ILLINOIS

DESCRIPTION
COVER SHEET
EXISTING CONDITIONS (TOPOGRAPHIC AND UTILITY SURVEY)
SOIL ANALYTICAL RESULTS ABOVE REMEDIATION CRITERIA
EXTENT OF SUBSURFACE EXCEEDANCE
GEOLOGICAL CROSS SECTIONS
DECOMMISSIONING PLAN
REMEDIATION EXCAVATION PLAN
PROPOSED REMEDIATION SITE LAYOUT
PROPOSED EROSION CONTROL PLAN
EROSION CONTROL DETAIL
GRADING PLAN AND PROPOSED STORM AND SANITARY SEWER
STORM AND SANITARY SEWER DETAILS
RESTORATION PLAN
SIGN LOCATION AND MAINTENANCE OF TRAFFIC PLAN
SIGN TYPE 1
SIGN TYPE R11-2 AND OM4-1
SIGN MOUNTING - BAND MOUNT AND FENCE MOUNT
SIGN MOUNTING - NEW SIGN POSTS
SIGN MOUNTING AT CONCRETE BARRIER
SITE PLAN
EXCAVATION LIMITS
EXCAVATION CROSS SECTIONS
SEWER PLAN AND PROFILE
CITY OF CHICAGO STANDARD DETAILS





NOTES:

1. NO CITY BRNCHMARK COULD BE USED FOR THIS LOCATION.

2. LEGEND FOR ABBREVIATIONS AND SYMBOLS ARE SPECIFIC FOR EACH SHEET.

DESIGNED: C. NISSEN
DRAWN: M.BANH
PROJECT NO. 103S328401004



DEPARTMENT OF ASSETS, INFORMATION AND SERVICES 30 NORTH LASALLE ST. SUITE 300 CHICAGO, IL 60602 312.744.3900



FORMER CARNOTITE REDUCTION COMPANY SITE 434 E. 26th STREET CHICAGO, ILLINOIS

SCALES:

HORIZONTAL SCALE:

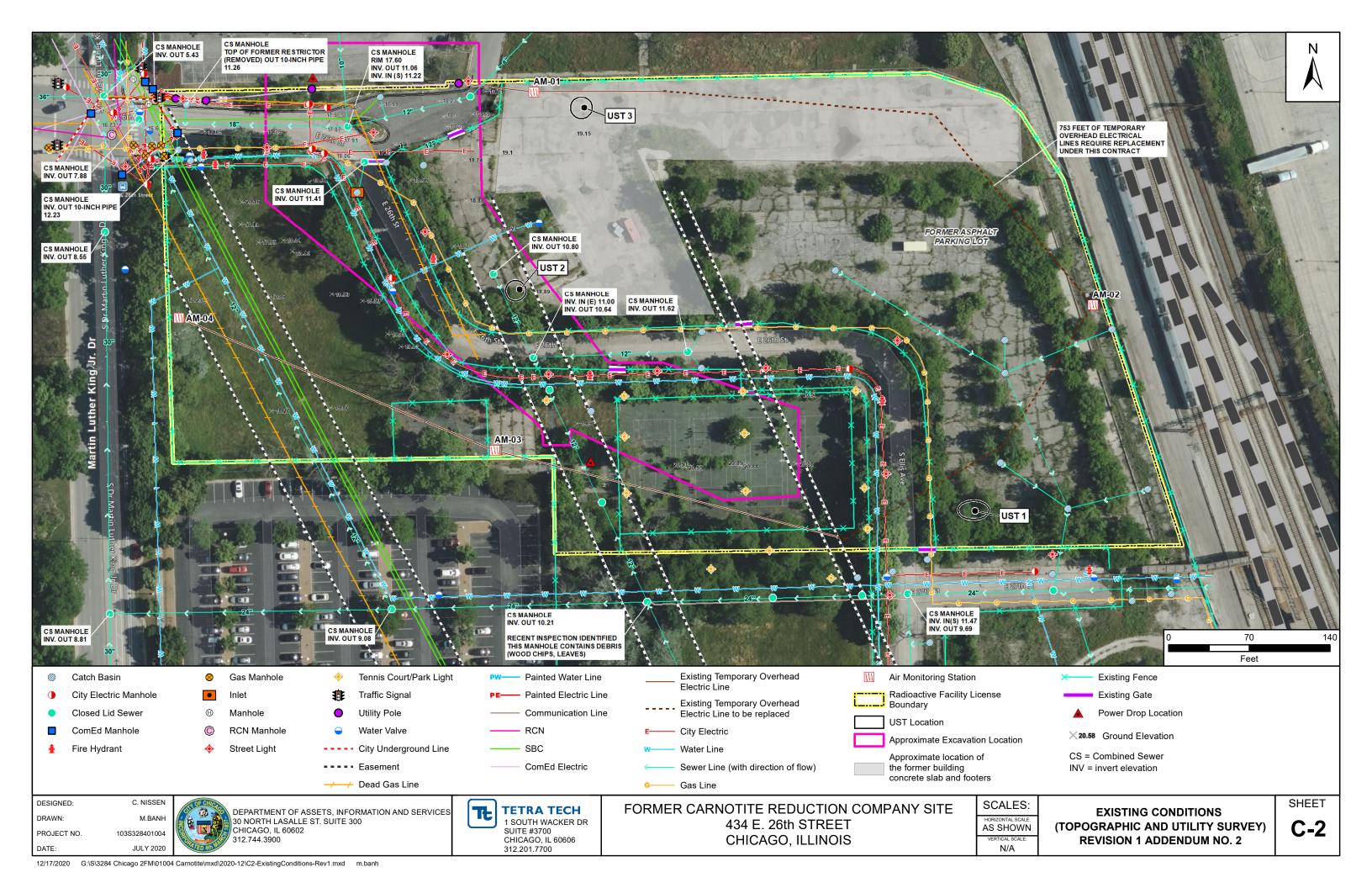
AS SHOWN

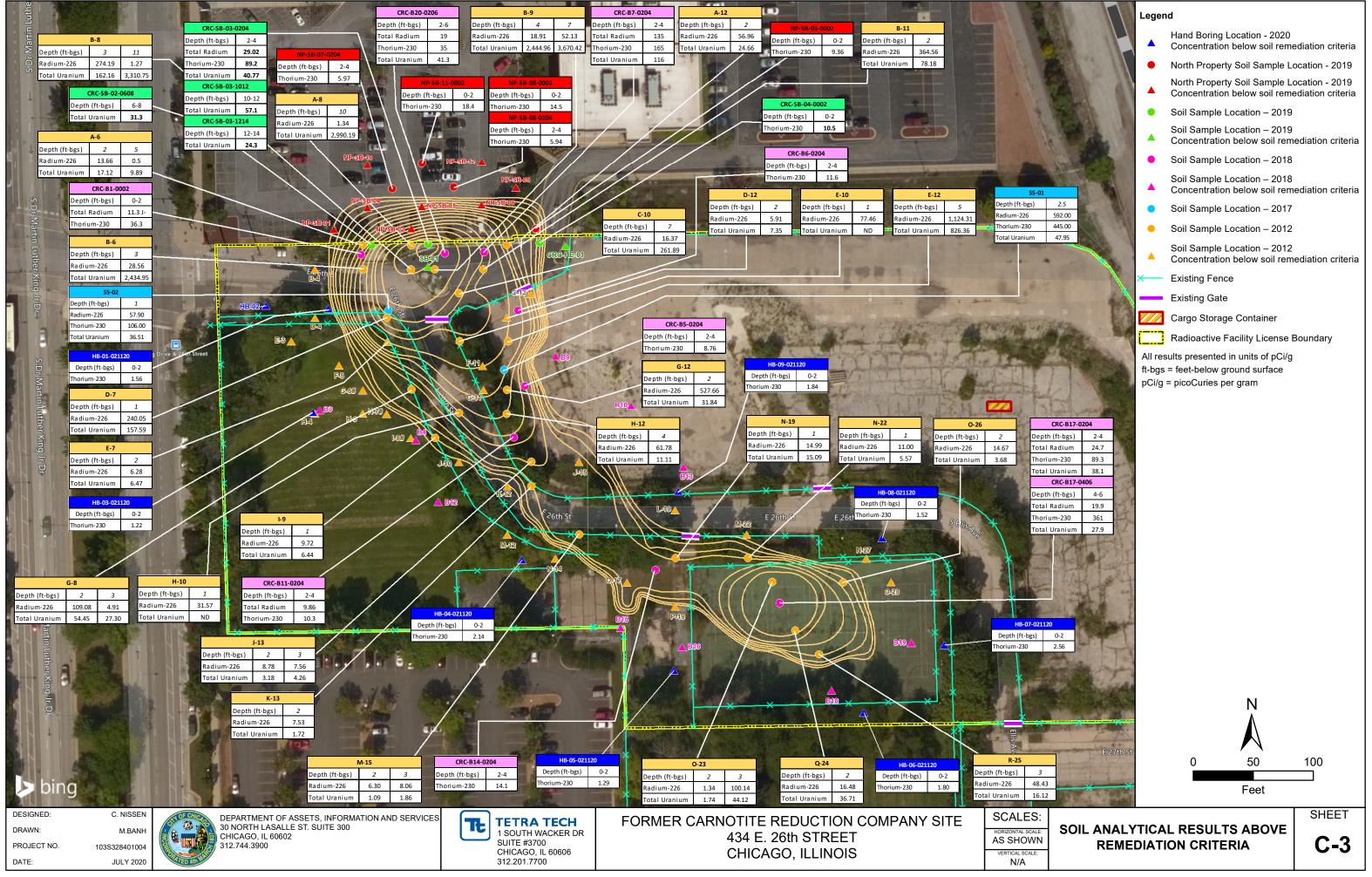
VERTICAL SCALE:

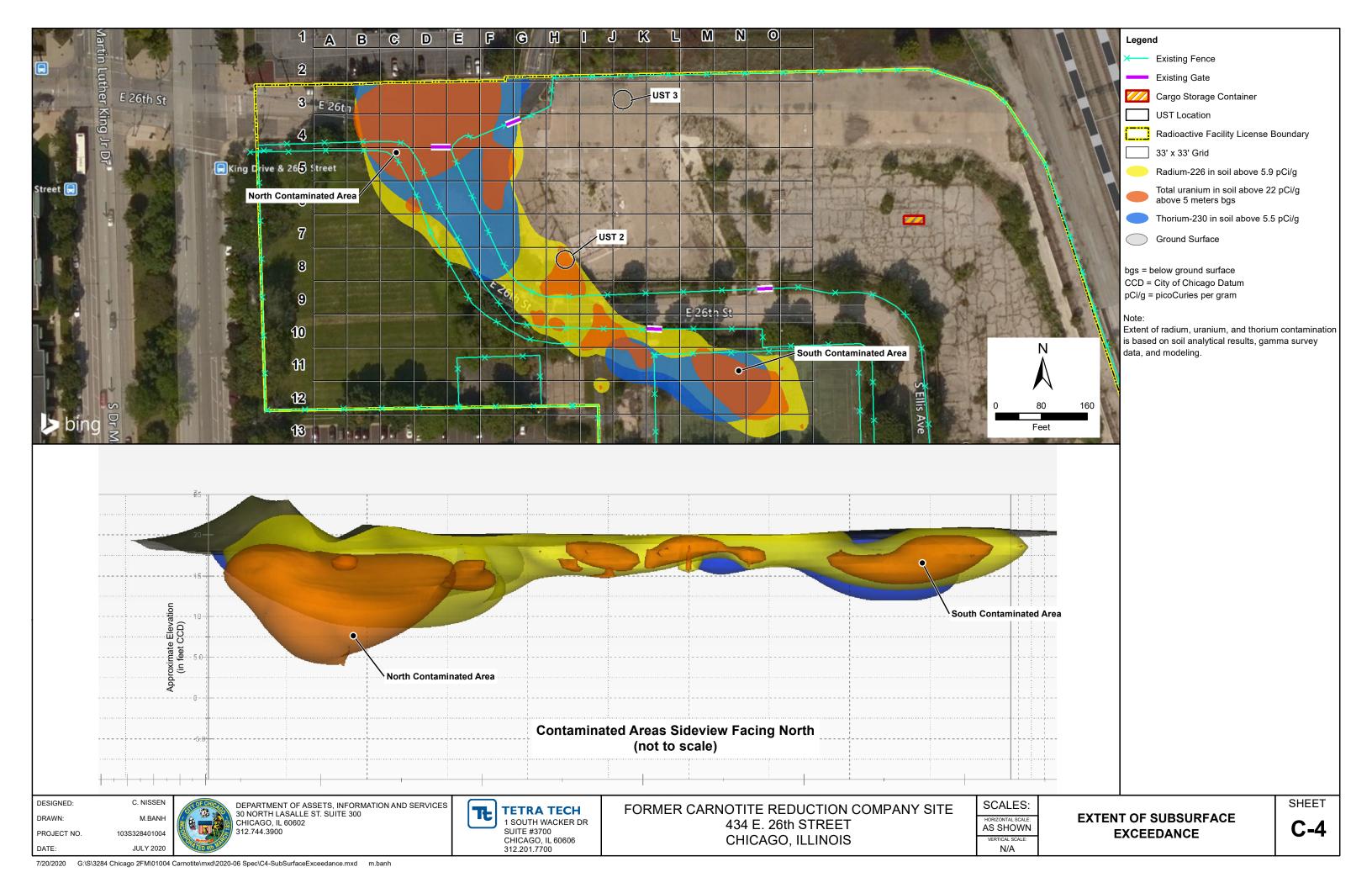
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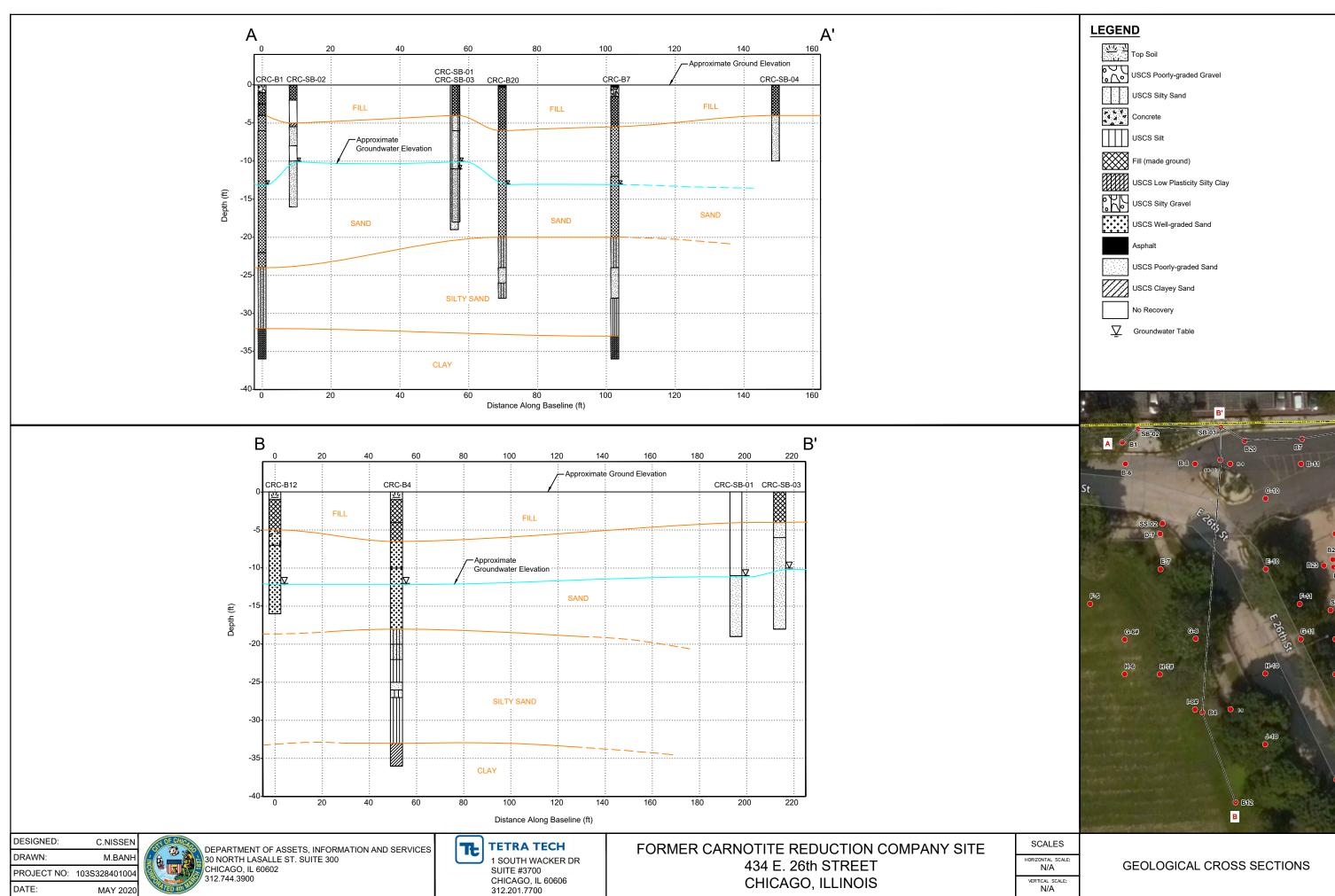
COVER SHEET

SHEET C-1





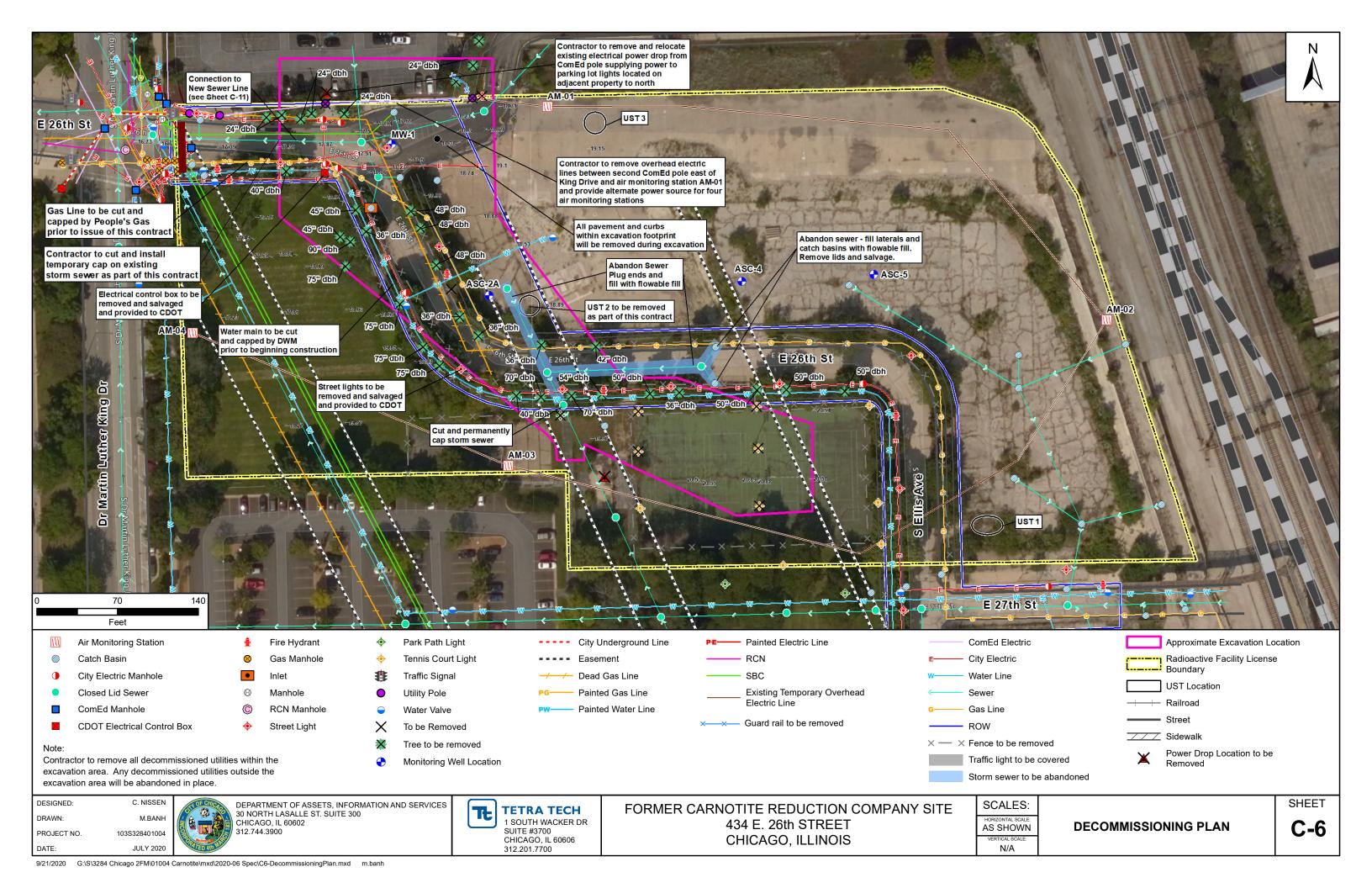


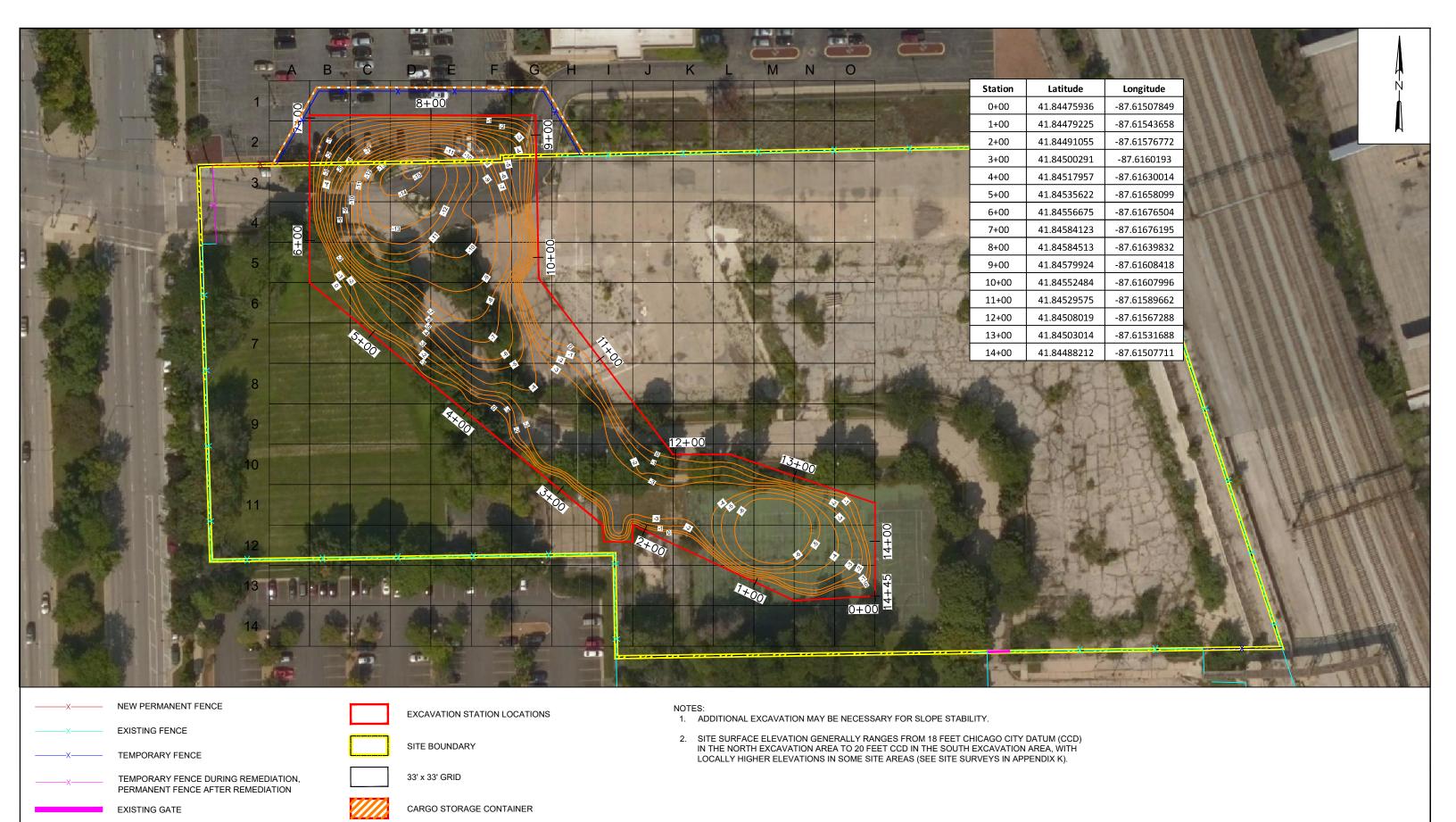


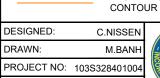
G:\S\3284 Chicago 2FM\01004 Carnotite\dwg\2020-04 Spec\C5-Geological Cross Sections.dwg

C-5

SHEET







DEPARTMENT OF ASSETS, INFORMATION AND SERVICES
30 NORTH LASALLE ST. SUITE 300
CHICAGO, IL 60602 312.744.3900

Tt

CONCRETE JERSEY BARRIER

TETRA TECH

CHICAGO, IL 60606 312.201.7700

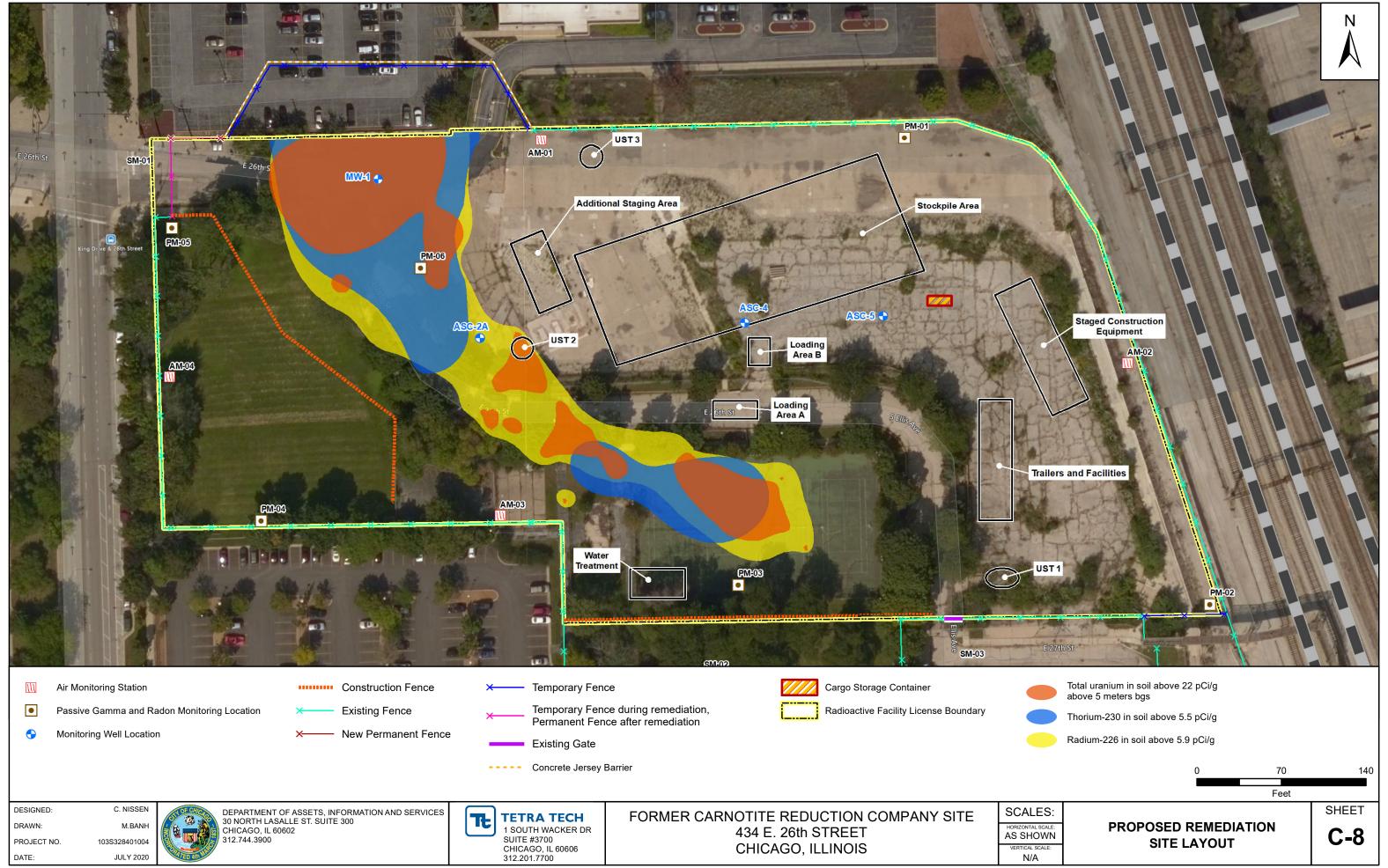
FORMER CARNOTITE REDUCTION COMPANY SITE 434 E. 26th STREET CHICAGO, ILLINOIS

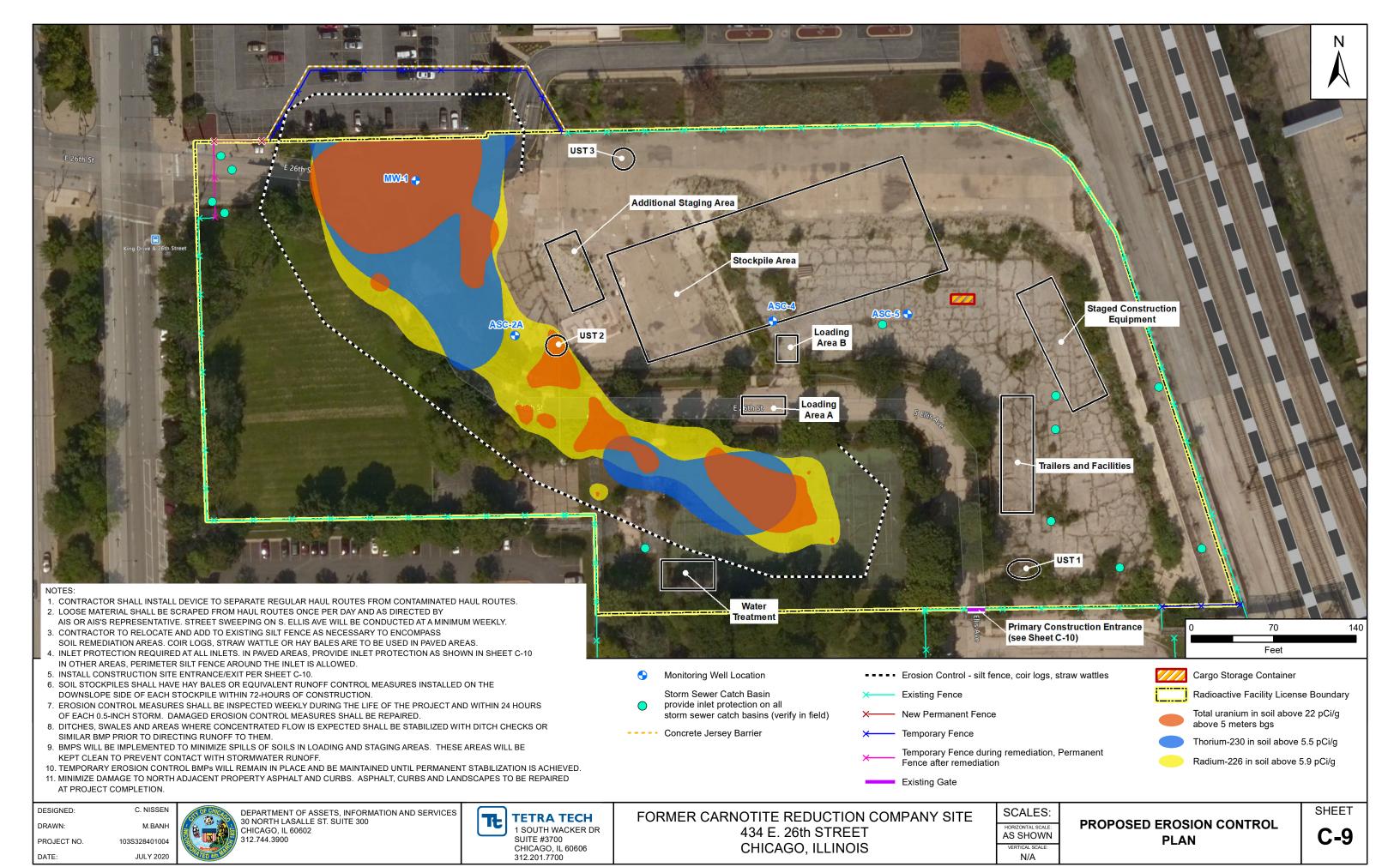
SCALES HORIZONTAL SCALE: VERTICAL SCALE:

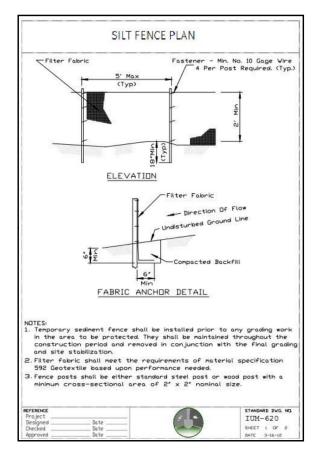
REMEDIATION EXCAVATION PLAN

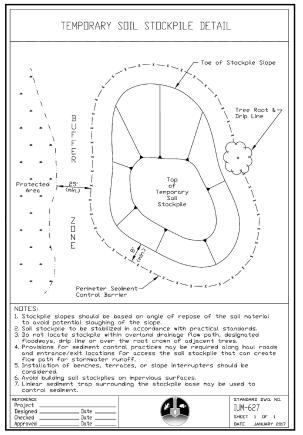
SHEET

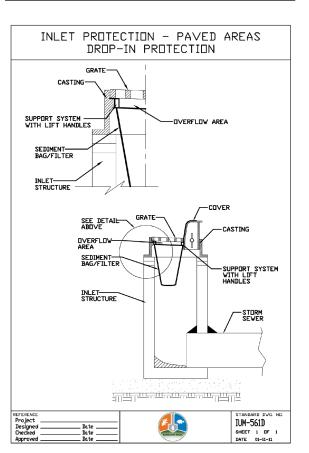
EXCAVATION DEPTH

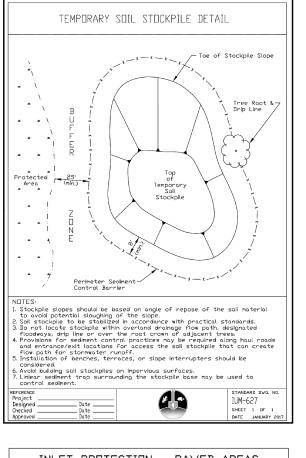


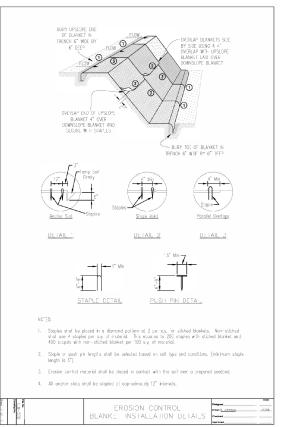




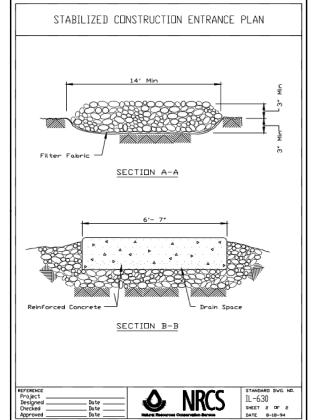












DESIGNED: DRAWN. M BANH PROJECT NO. 103S328401004

MAY 2020

DATE:



DEPARTMENT OF ASSETS, INFORMATION AND SERVICES 30 NORTH LASALLE ST. SUITE 300 CHICAGO, IL 60602 312.744.3900



FORMER CARNOTITE REDUCTION COMPANY SITE 434 E. 26th STREET CHICAGO, ILLINOIS

STABILIZED CONSTRUCTION ENTRANCE PLAN

| Wash Rack | (Optional)

10' Min

PLAN VIEW

Α-SIDE ELEVATION

NOTES:

1. Filter fabric shall meet the requirements of material specification 592 GEDTEXTILE, Table I or 2, Class I, II or IV and shall be placed over the cleared area prior to the placing of rock.

2. Rock or rectained concrete shall meet one of the following IDOT coarse aggregate gradation, CA-1, CA-2, CA-3 or CA-4 and be placed according to construction specification 25 ROCKFILL using placement Method I and Class III compaction.

3. Any drainage facilities required because of washing shall be constructed according to manufacturers specifications.

IL-630

constructed according to manufacturers specifications.

4.If wash racks are used they shall be installed according to the

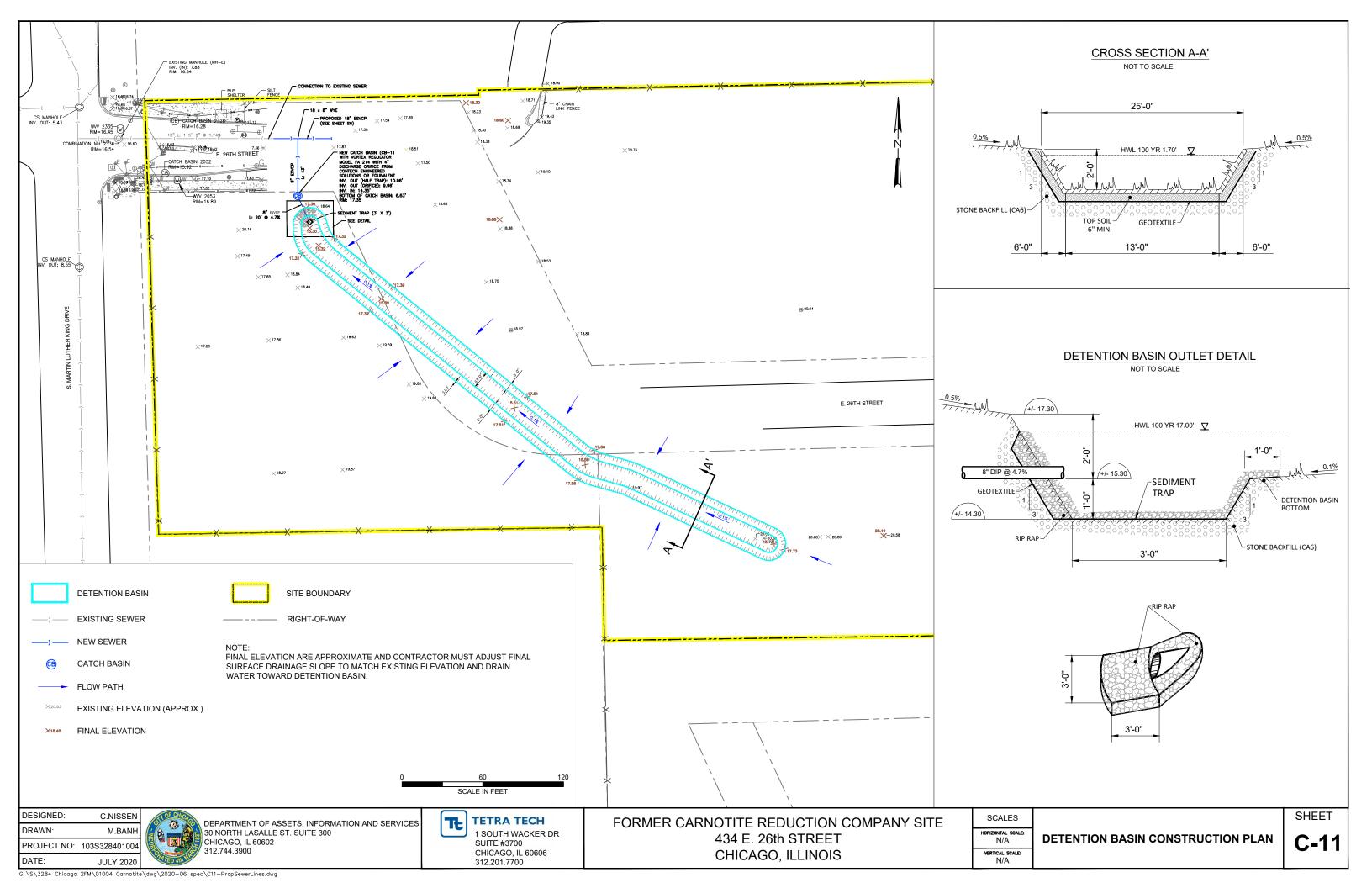
manufacturer's specifications

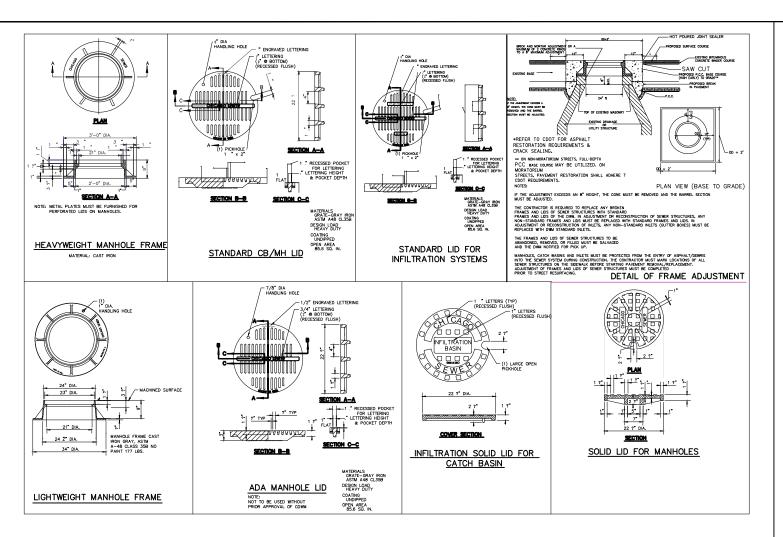
Positive Drainage To Sediment Trapping Device.

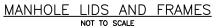
Must Extend Full Width
Of Ingress And Egress
Operation.

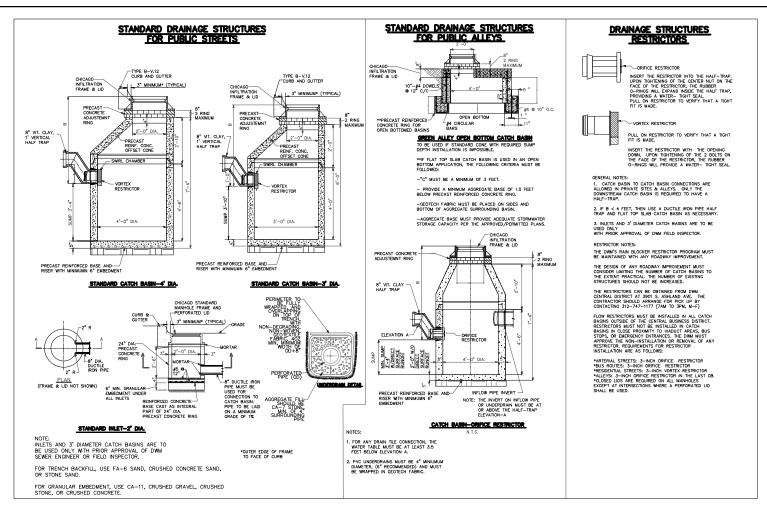
SCALES: AS SHOWN VERTICAL SCALE: N/A

EROSION CONTROL DETAIL

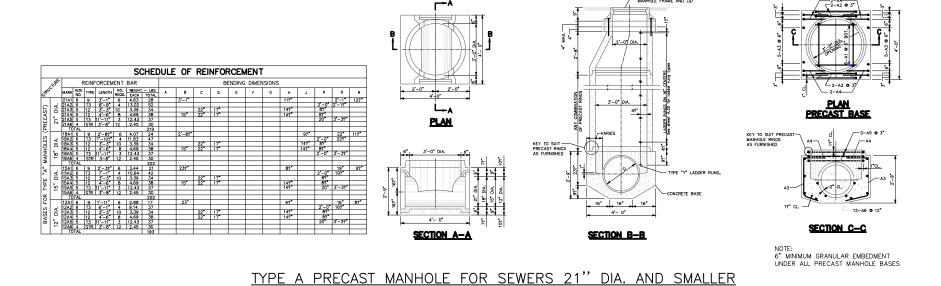








DRAINAGE STRUCTURE DETAILS NOT TO SCALE



NOT TO SCALE

STORM AND SANITARY DETAILS ARE DOWNLOADED FROM THE CITY OF CHICAGO DEPARTMENT OF WATER MANAGEMENT BUREAU OF ENGINEERING SERVICES

DESIGNED C.NISSEN M.BANH DRAWN: PROJECT NO: 103S328401004 DATE: MAY 2020



TETRA TECH 1 SOUTH WACKER DR SUITE #3700

312.201.7700

CHICAGO, IL 60606

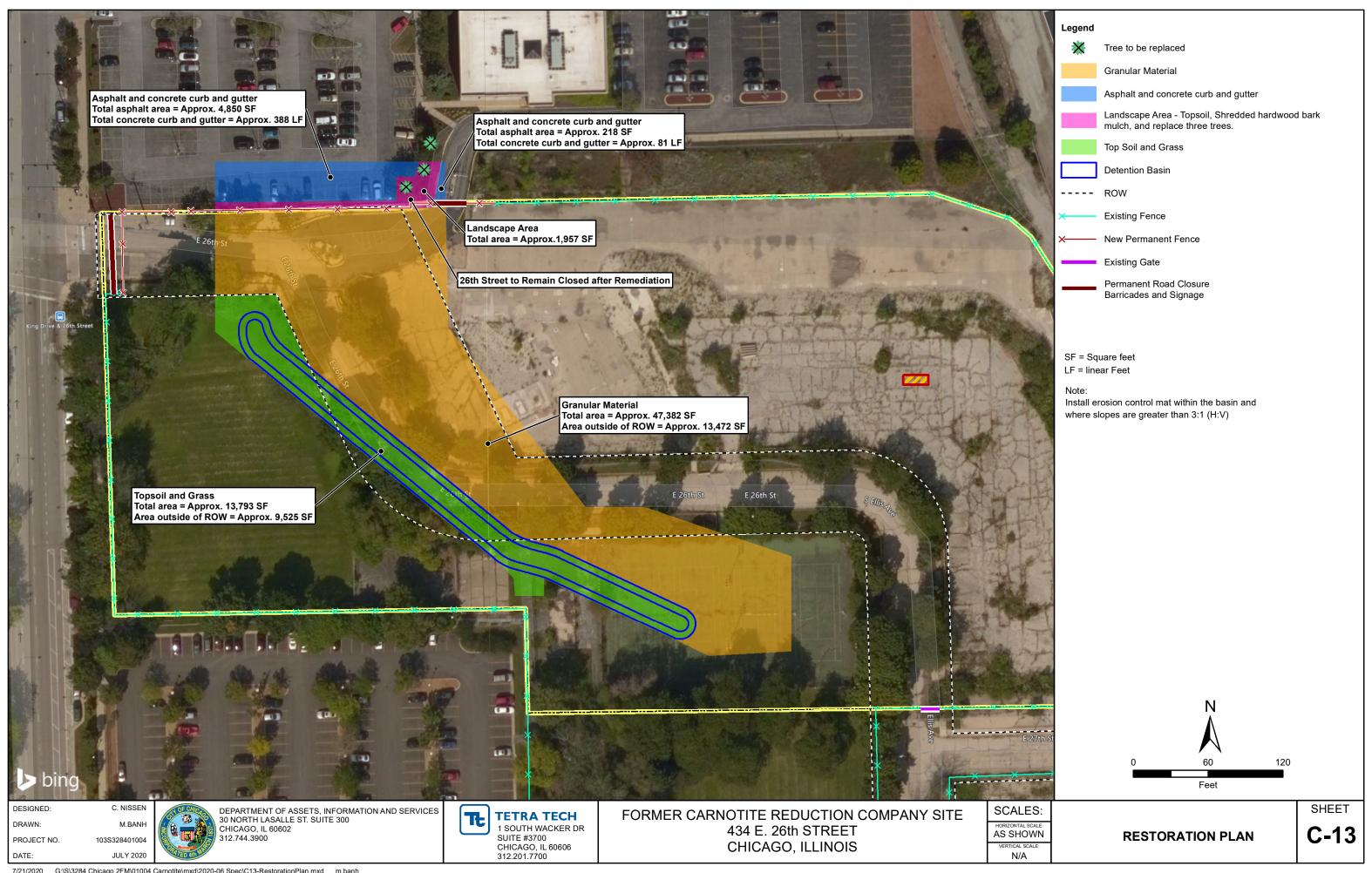
FORMER CARNOTITE REDUCTION COMPANY SITE 434 E. 26th STREET CHICAGO, ILLINOIS

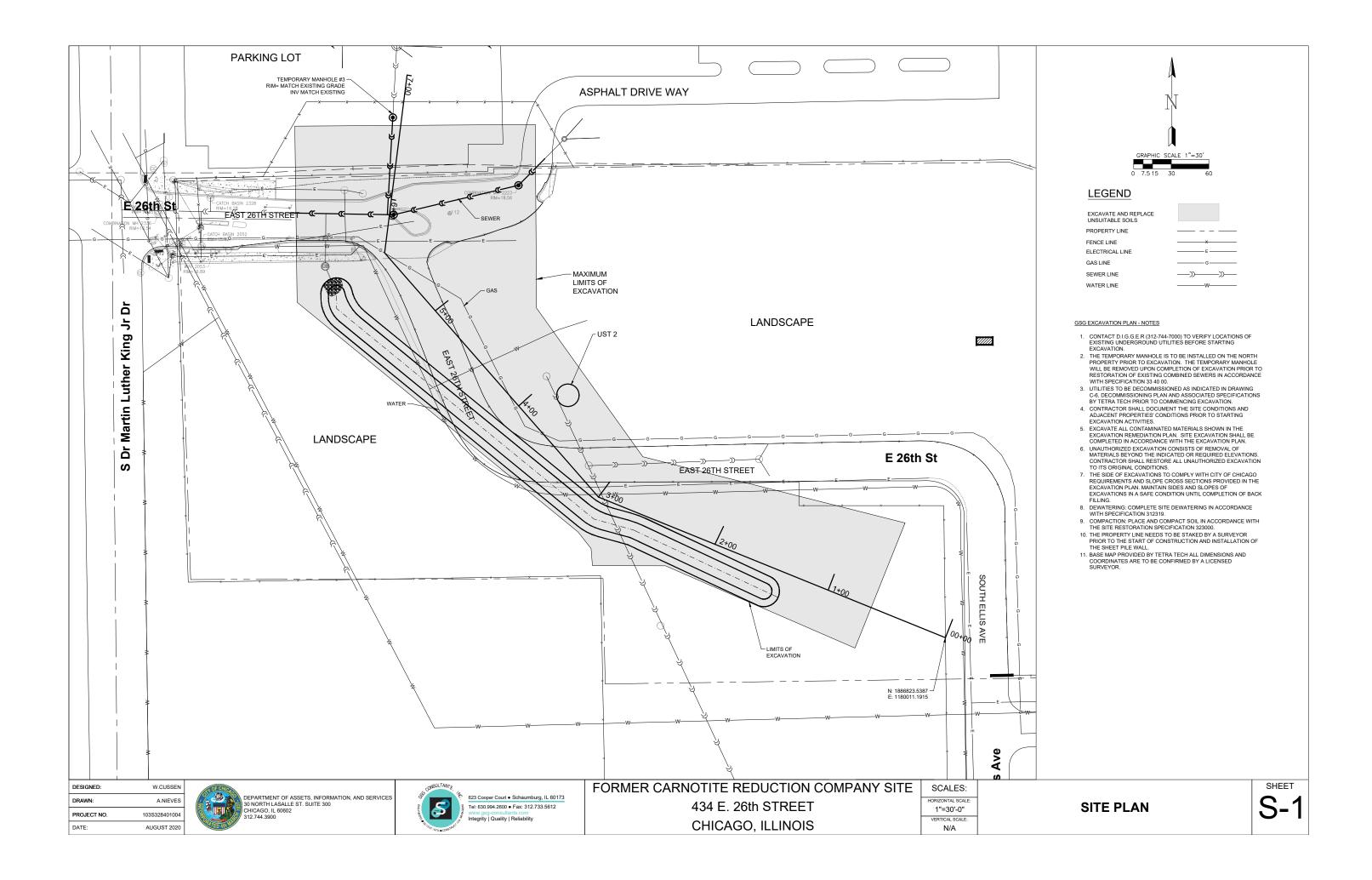
SCALES HORIZONTAL SCALE N/A VERTICAL SCALE: N/A

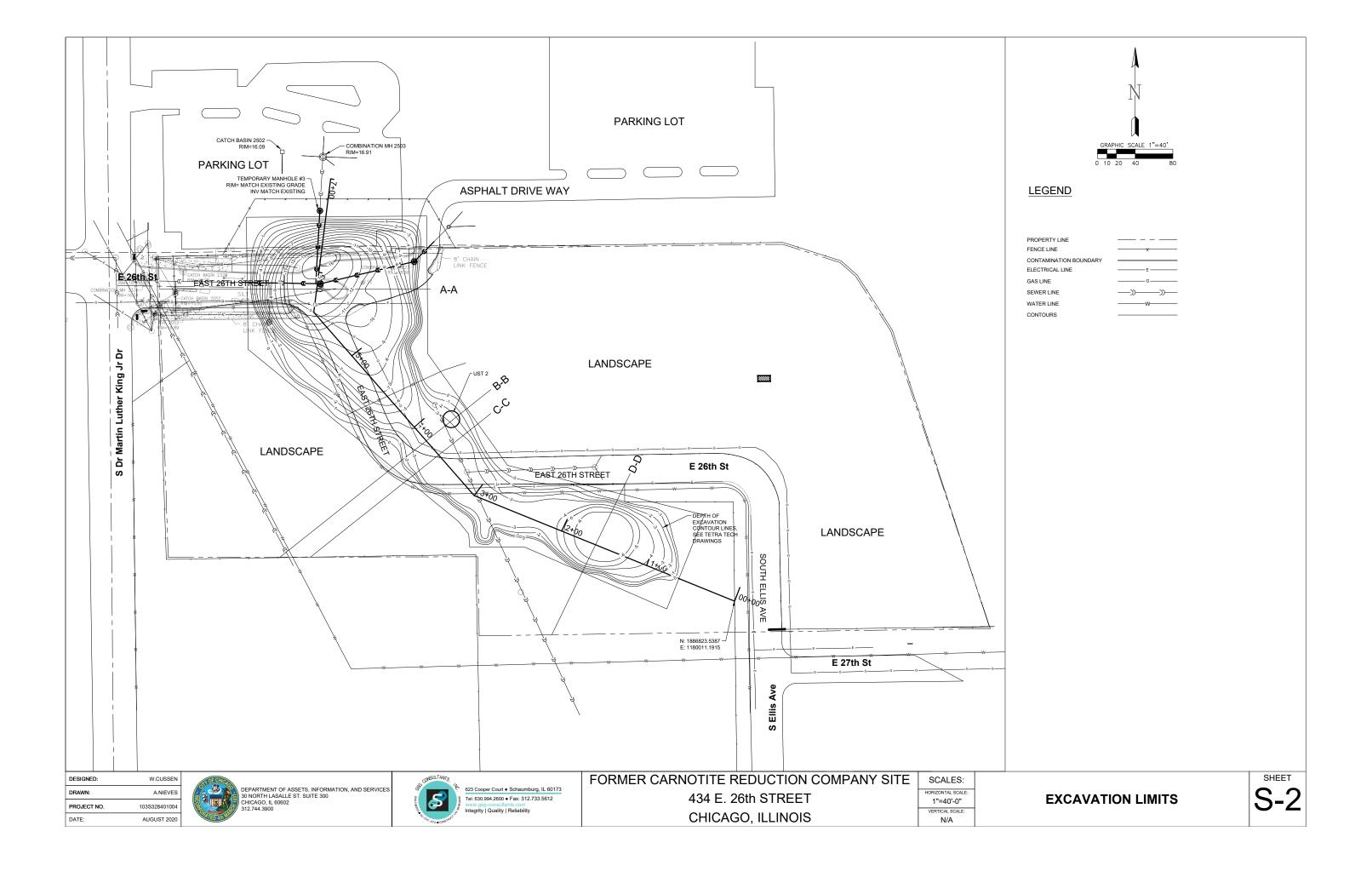
STORM AND SANITARY SEWER DETAILS

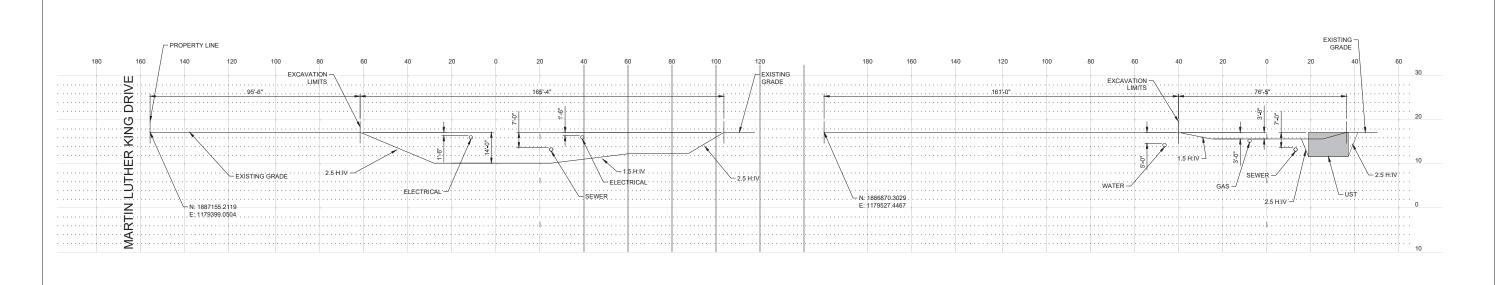
SHEET

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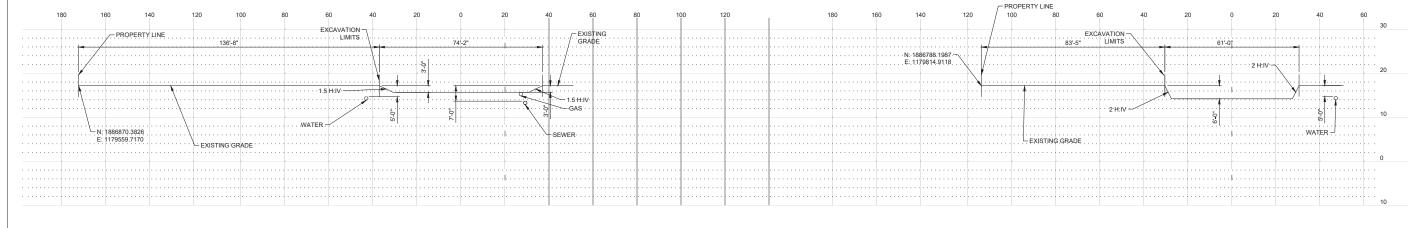












 CROSS SECTION C-C
 CROSS SECTION D-D

 3+58
 1+59

NOTE: UTILITY DEPTHS AND LOCATIONS ARE ASSUMED AND SHOULD BE FIELD VERIFIED.

GRAPHIC SCALE 1"=20'
0 5 10 20 40

DESIGNED:	W.CUSSEN
DRAWN:	A.NIEVES
PROJECT NO.	103S328401004
DATE:	AUGUST 2020





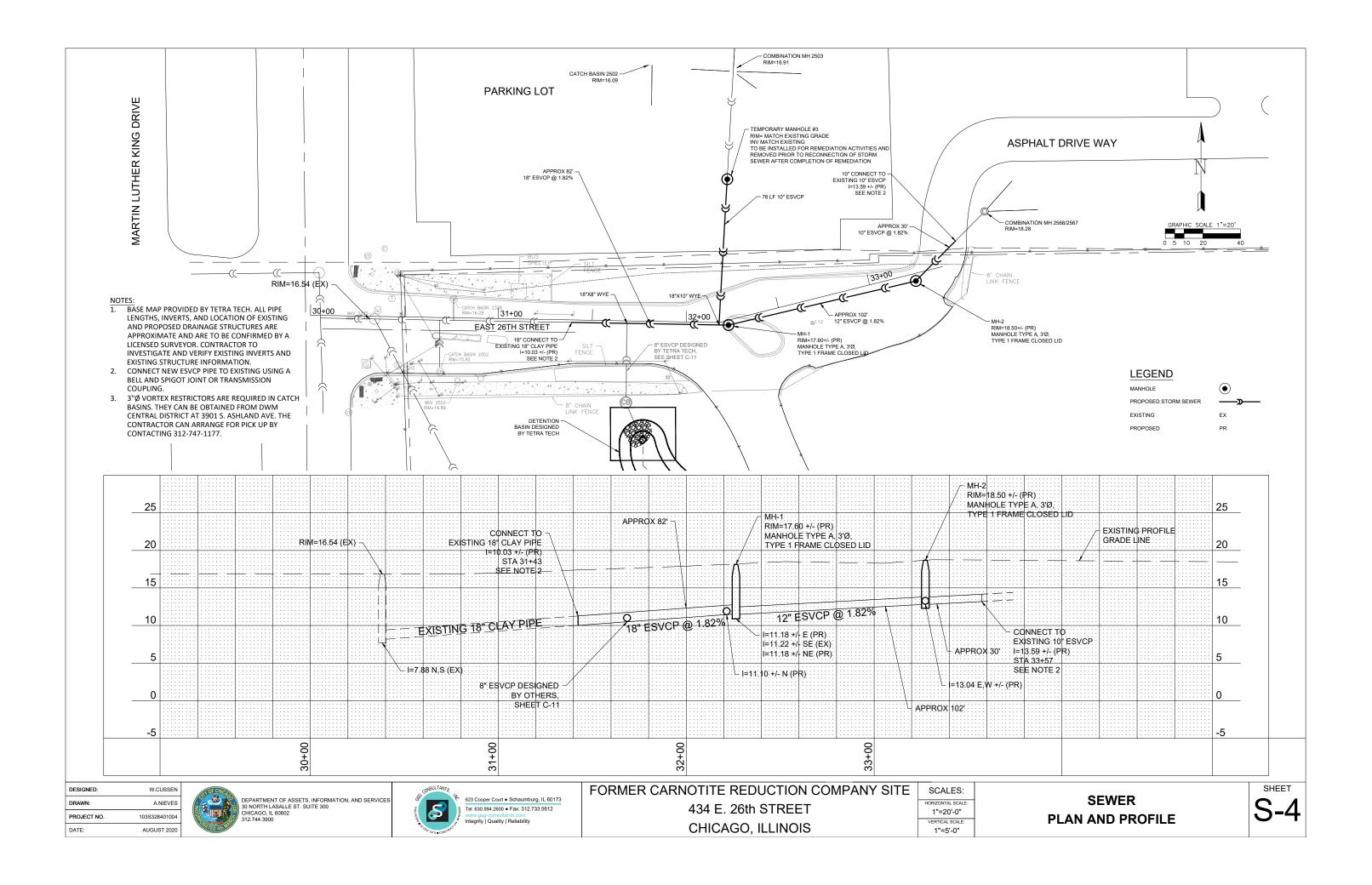
CROSS SECTION A-A

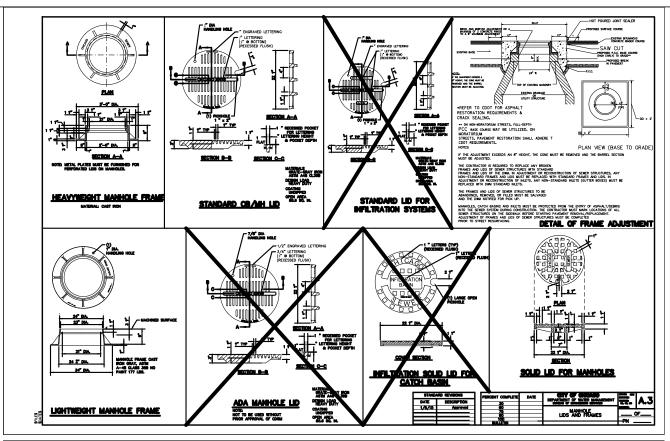
FORMER CARNOTITE REDUCTION COMPANY SITE	
434 E. 26th STREET	F
CHICAGO, ILLINOIS	

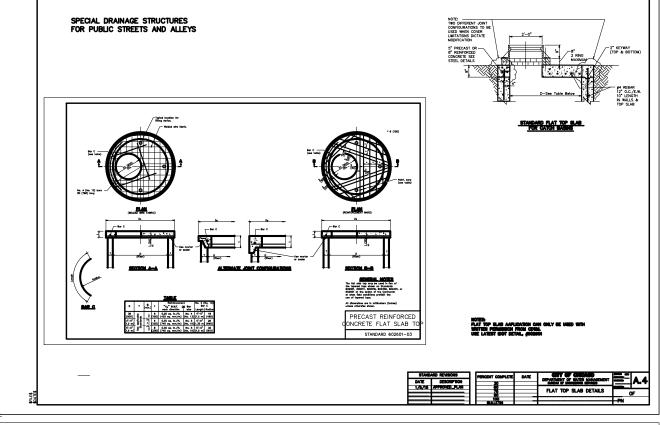
SCALES:	
HORIZONTAL SCALE:	
1"=20'-0"	
VERTICAL SCALE:	
1"=5'-0"	

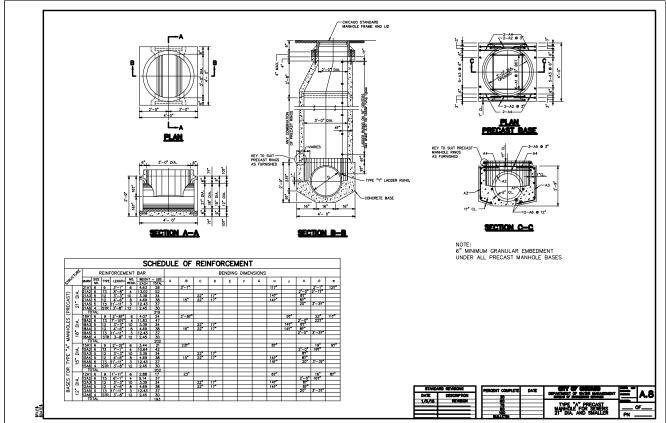
CROSS SECTION B-B

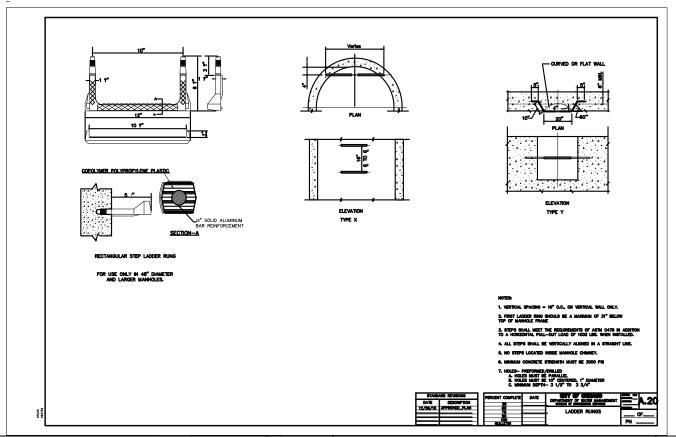
S-3











DESIGNED:	W.CUSSEN
DRAWN:	A.NIEVES
PROJECT NO.	103S328401004
DATE:	AUGUST 2020





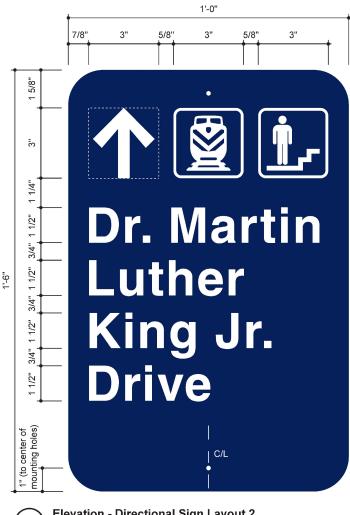
FORMER CARNOTITE REDUCTION COMPANY SITE
434 E. 26th STREET
CHICAGO, ILLINOIS

SCALES:	
HORIZONTAL SCALE:	C
VERTICAL SCALE: NTS	

CITY OF CHICAGO STANDARD DETAILS



1'-0" Metra 27th St Station **Elevation - Directional Sign Layout 1** Scale: 3" = 1'-0"





Sign Type 1

12" x 18" Directionals

Aluminum Sign Panel

The sign substrate is a .080" thick solid aluminum panel.

2 Background

The overall background of the sign and the white text and graphics shall be an exterior-grade, premium opaque cast white printable graphic film. Sign panels shall have the printed film applied to the face side of the sign. The edges and back side of the sign shall be painted to match PMS 281C.

3 Digitally Printed Graphics

The graphics shall be digitally printed at high resolution directly onto the graphic film using custom formulated, exterior grade, UV-resistant, opaque inks. The inks shall be formulated to match the colors specified and to be compatible with the graphic film. Protect printed graphics with a clear protective anti-graffiti overlaminate that is compatible with the graphic film and the printed graphics. The printed graphic film and overlaminate shall be applied to cover the entire sign face and trimmed flush to the edges of the sign panel.

4 Holes for Mounting Hardware

7/16" diameter holes positioned as shown. All holes shall be drilled in the shop.

6 Mounting Hardware

See the Sign Mounting drawings for additional information on sign posts and sign mounting

See the Specifications for additional information and requirements.

Elevation - Directional Sign Layout 2

Scale: 3" = 1'-0"

DESIGNED:

103S328401004 PROJECT NO. DATE:

DRAWN:

M BANH

DEPARTMENT OF FLEET AND FACILITY MANAGEMENT 30 NORTH LASALLE ST. SUITE 300 CHICAGO, IL 60602



FORMER CARNOTITE REDUCTION COMPANY SITE 434 E. 26th STREET CHICAGO, ILLINOIS

SCALES: N/A VERTICAL SCALE: N/A

SIGN TYPE 1

30 **2 C** 48 R11-2

ROAD CLOSED

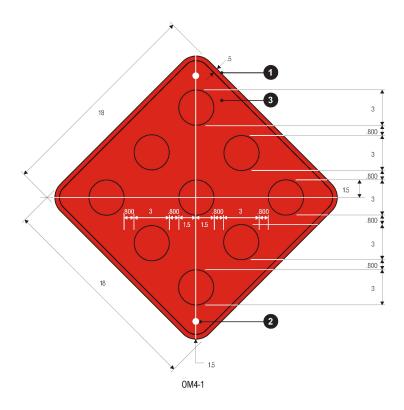
COLORS: LEGEND — BLACK

BACKGROUND— WHITE (RETROREFLECTIVE)

FONT: STANDARD ALPHABETS FOR TRAFFIC CONTROL DEVICES SERIES D 2000

Elevation - MUTCD Standard Sign R11-2

Scale: 1" = 1'-0"



REFLECTORS - RED (RETROREFLECTIVE)

BACKGROUND — RED PAINT TO MATCH RED RETROREFLECTIVE SHEETING (FRONT ONLY)
BORDER — RED (RETROREFLECTIVE)

Elevation - MUTCD Standard Sign OM4-1

DESIGNED: DRAWN:

PROJECT NO.

DATE:

C. NISSEN M BANH 103S328401004



DEPARTMENT OF FLEET AND FACILITY MANAGEMENT 30 NORTH LASALLE ST. SUITE 300 CHICAGO, IL 60602



FORMER CARNOTITE REDUCTION COMPANY SITE 434 E. 26th STREET CHICAGO, ILLINOIS

SCALES: AS SHOWN

N/A

SIGN TYPE R11-2 AND OM4-1

Sign Type R11-2 and

DOT Standard "Road Closed" Signs

Graphic layouts for sign types R11-2 and OM4-1 shall be as per the U.S. Department of Transportation Federal Highway Administration's Standard Highway Signs and Markings and applicable sections of the Manual of Uniform

See the Specifications for additional information

Sign Type OM4-1

and Object Marker

aluminum panel.

3 Graphics

and requirements.

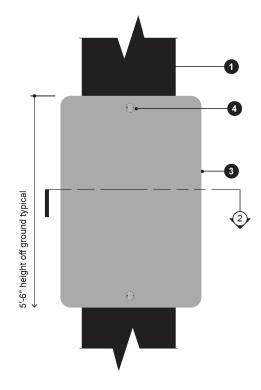
1 Aluminum Sign Panel

The sign substrate is a .080" thick solid

2 Holes for Mounting Hardware 7/16" diameter holes, coordinate location of holes with sign posts and mounting hardware. See the Sign Mounting drawings for additional information on sign posts and sign mounting.

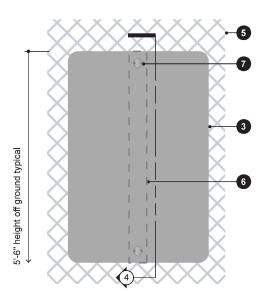
Traffic Control Devices (MUTCD).

7/16/2019 G:\S\3284 Chicago 2FM\01004 Carnotite\mxd\2019-07\Signage Sheets\T3-Type-R11-2.mxd m.banh



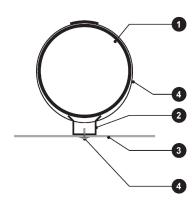
Elevation - Band Mounting to Existing Post

Scale: 1 1/2" = 1'-0"



Elevation - Mounting to Existing Fence

Scale: 1 1/2" = 1'-0"



Section - Band Mounting to Existing Post

′ Scale: 1 1/2" = 1'-0"



Section - Mounting to Existing Fence

Scale: 1 1/2" = 1'-0"

Sign Mounting

Sign mounting Information for locations where signs are band-mounted to existing posts and locations where signs are mounted to chain-link fencing.

1 Existing Post

Existing concrete or steel light post.

2 Mounting Bracket

The bracket shall be a standard stainless steel sign bracket suitable for mounting signs centered on light posts and other structures.

3 Aluminum Sign Panel

See the Sign Type 1 drawing for sign panel details.

 Mounting Screws/Hardware for Band Mounting

Provide stainless steel screws, flat washers, lock washers, and nylon washers as needed to properly, safely, and securely mount the aluminum sign panel to the bracket. Install washers in the following order: 1) screw head, 2) lock washer, 3) flat washer, 4) nylon washer, 5) sign panel. The bracket and aluminum sign panel assembly shall be securely strap mounted to the light post or other structure using heavy-duty stainless steel sign straps. All mounting hardware and components shall be vandal-resistant and suitable for exterior use. The mounting hardware shall allow for removal of the sign panels for maintenance, repairs, and updates.

5 Existing Fence

Existing chain-link fence.

6 Backer Plate for Fence Mounting

Provide 1-1/2" x 18" painted aluminum backer plate for mounting sign panel to fence. Paint backer plate to match back of sign panel.

Mounting Bolts/Hardware for Fence Mounting

Provide stainless steel bolts, flat washers, lock washers, and nylon washers as needed to properly, safely, and securely mount the aluminum sign panel to the fence. Install washers in the following order: 1) bolt head, 2) nylon washer, 3) sign panel, 4) fence, 5) backer plate, 6) flat washer, 7) lock washer, 8) nut. All mounting hardware and components shall be vandal-resistant and suitable for exterior use. The mounting hardware shall allow for removal of the sign panels for maintenance, repairs, and updates.

See the Specifications for additional information and requirements.

DESIGNED: C. NISSEN
DRAWN: M.BANH
PROJECT NO. 103S328401004

DATE:

E CONTRACTOR

DEPARTMENT OF FLEET AND FACILITY MANAGEMENT 30 NORTH LASALLE ST. SUITE 300 CHICAGO, IL 60602 312 744 3000



FORMER CARNOTITE REDUCTION COMPANY SITE 434 E. 26th STREET CHICAGO, ILLINOIS SCALES:

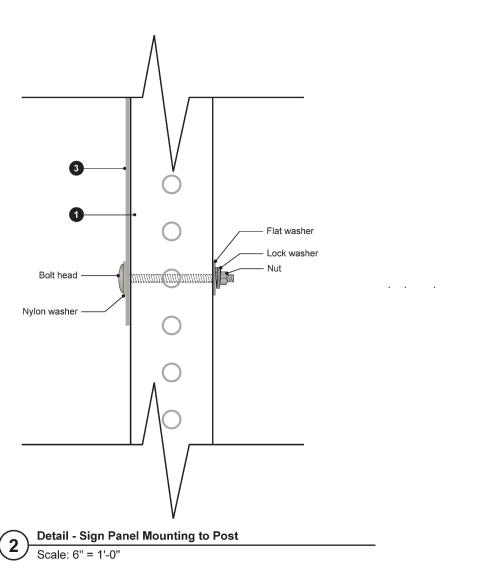
HORIZONTAL SCALE:
AS SHOWN

N/A

SIGN MOUNTING
BAND MOUNT AND FENCE MOUNT

SHEET

Elevation - New Direct Bury Sign Post Mounting Scale: 1" = 1'-0"



Sign Mounting

Sign Mounting Information for Locations With New Sign Posts

1 Perforated Sign Post

Galvanized steel 1 3/4" square perforated sign nost

2 Perforated Anchor Post

Galvanized steel 2" square perforated anchor post

3 Aluminum Sign Panel

See the Sign Type 1 drawing for sign panel

4 Mounting Bolts/Hardware

Provide stainless steel bolts, flat washers, lock washers, and nylon washers as needed to properly, safely, and securely mount the aluminum sign panel to the sign post. Install washers in the following order: 1) bolt head, 2) nylon washer, 3) sign panel, 4) sign post, 5) flat washer, 6) lock washer, 7) nut. All mounting hardware and components shall be vandal-resistant and suitable for exterior use. The mounting hardware shall allow for removal of the sign panels for maintenance, repairs, and updates.

5 Break-away Coupler

NCHRP 350 compliant break-away coupler designed for use with square sign posts.

6 Existing Soil

Verify the existing conditions at the installation location. Verify if the sign can be safely, securely, and properly installed.

See the Specifications for additional information and requirements.

DESIGNED: C. NIS

DRAWN:

PROJECT NO. 103S328401004

DATE: JULY 2019

TOP CHICAGO

M BANH

DEPARTMENT OF FLEET AND FACILITY MANAGEMENT 30 NORTH LASALLE ST. SUITE 300 CHICAGO, IL 60602 312 744 3900

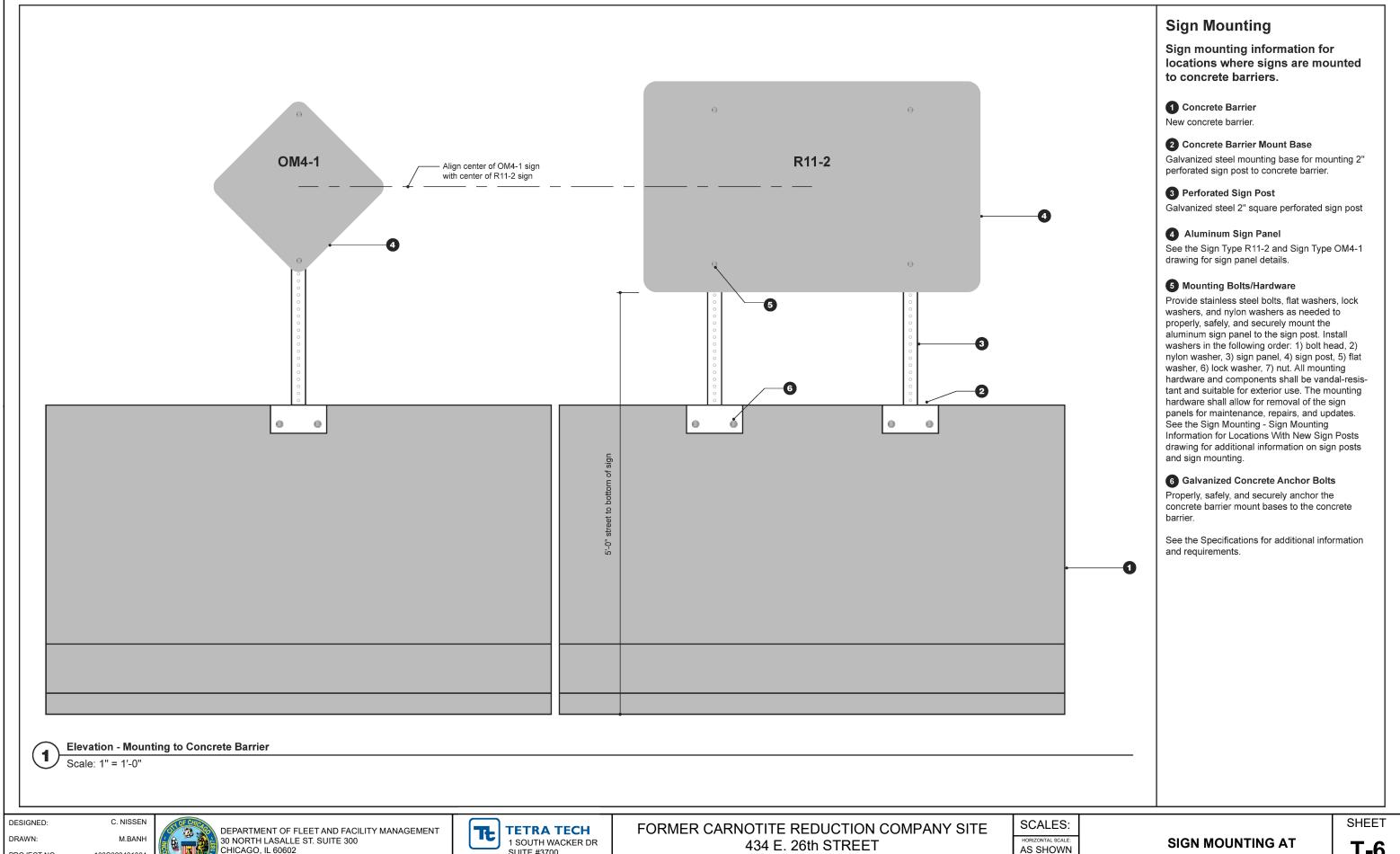


312.201.7700

FORMER CARNOTITE REDUCTION COMPANY SITE 434 E. 26th STREET CHICAGO, ILLINOIS SCALES:

HORIZONTAL SCALE:
AS SHOWN
VERTICAL SCALE:
N/A

SIGN MOUNTING NEW SIGN POSTS SHEET



CHICAGO, ILLINOIS

SUITE #3700

312.201.7700

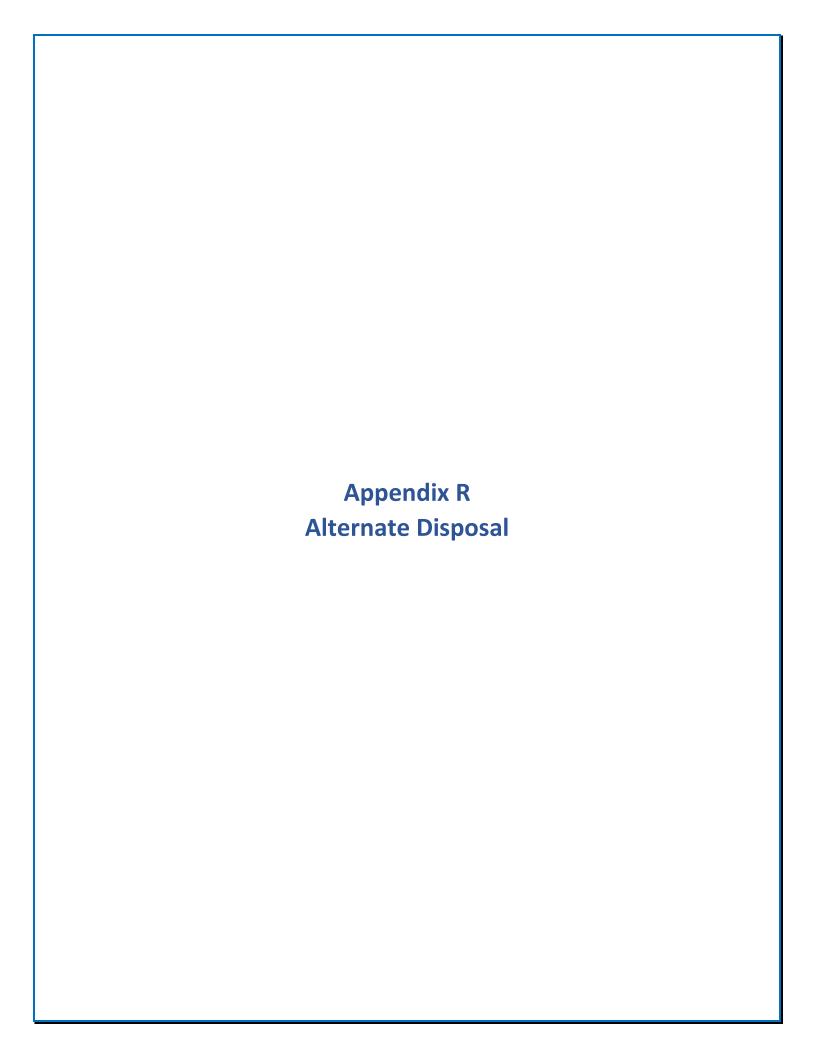
CHICAGO, IL 60606

PROJECT NO.

DATE:

103S328401004

N/A





ILLINOIS EMERGENCY MANAGEMENT AGENCY

JB Pritzker
Governor

Alicia Tate-Nadeau
Acting Director

November 25, 2019

Radioactive Material License IL-02467-01

Abby Mazza, P.E.
Environmental Engineer III
City of Chicago, Department of Fleet and Facility Management
Bureau of Environmental, Health & Safety Management
30 N. LaSalle Street, Suite 300, Chicago, Illinois 60602-2572

Re: Alternative Disposal Methodology Application for the Former Carnotite Reduction Company Site, 434 E. 26th Street, Chicago, IL

Dear Ms. Mazza,

Agency personnel have received and reviewed your transmittal letter dated October 24, 2019 regarding an alternative disposal methodology for waste material to be generated at the former Carnotite Reduction Company (Carnotite) site as the result of decommissioning. As we have discussed, Subpart K of 32 Illinois Administrative Code (IAC) 340 sets forth the regulations for waste disposal of licensed radioactive material. As such, Subpart K only allows licensed material to be disposed of at a licensed facility provided the material does not meet other conditional disposal methodologies or unless an alternative methodology is granted by the Agency.

In your request, you specifically request soils containing radium-226 meeting the Agency's definition of byproduct material (32 IAC 310.20) be disposed of in accordance with Title 10 Code of Federal Regulations (CFR) 20.2008(b) Disposal of Certain Byproduct Material, which states:

A licensee may dispose of byproduct material, as defined in paragraphs (3) and (4) of the definition of Byproduct material set forth in § 20.1003, at a disposal facility authorized to dispose of such material in accordance with any Federal or State solid or hazardous

waste law, including the Solid Waste Disposal Act, as authorized under the Energy Policy Act of 2005.

Since the Agency's definition of byproduct material pertaining to radium-226 is consistent with the U.S. Nuclear Regulatory Commission's definition of byproduct material (10 CFR 20.1003), the Agency approves your alternative disposal methodology request in accordance with 10 CFR 20.2008(b) provided the specific conditions outlined in your request are met.

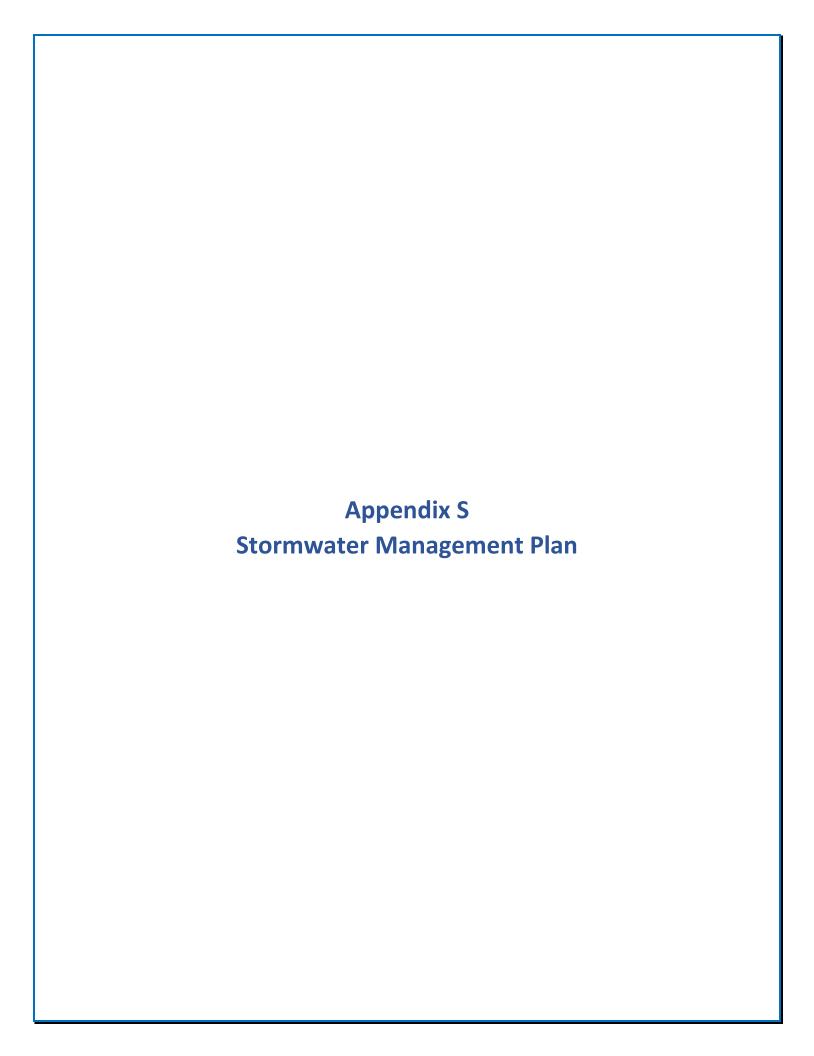
Further, you request soils containing uranium and thorium not exceeding the unimportant source material quantity of 0.05% by weight also be granted an alternative disposal method. In your request you cite unimportant quantities of source material in accordance with 10 CFR 40.13. As with past instances, I caution you to not cite NRC regulation in place of the applicable Agency regulation. In this case, the appropriate citation should be 32 IAC 330.30(a).

Provided the disposal facility's waste acceptance criteria allows for uranium and thorium not exceeding the unimportant source material quantity, and the specific conditions outlined in your request are met, the Agency approves of this methodology but reserves final approval of the actual final disposition of the material until a specific disposal site is selected.

Should you have questions or concerns please do not hesitate to contact me.

Sincerely

Kelly Horn, Section Head Environmental Management Division of Nuclear Safety





March 18, 2020

Mr. Andrew Billing Lead Stormwater Reviewer Mackie Consultants, LLC, consultant to: City of Chicago Department of Buildings City Hall, Room 906 121 N. LaSalle Street Chicago, IL 60602

Subject: Former Carnotite Reduction Company Site Remediation

Stormwater Management Plan – Revision 1

Dear Mr. Billing:

On behalf of the City of Chicago Department of Assets, Information & Services (AIS) (formerly, Department of Fleet and Facility Management), Tetra Tech is submitting the revised Stormwater Management Plan (SWP) for the Former Carnotite Reduction Company (Carnotite) site remediation for review by the City of Chicago Department of Buildings (DOB), the designated authorized agent for the City of Chicago Department of Water Management (DWM).

This revised version addresses your comments received by Tetra Tech on December 3, 2019 (see Attachment 14). The review comments and Tetra Tech's responses are summarized below:

1. Attachment 7 Detention Basin Volume Calculations: Since the volume control component of the Stormwater Ordinance is proposed to be met using oversized detention, the detention basin must be sized to provide 14,641 cu ft (12,664 + 1,977), the rate control plus the volume control volumes.

Response: Comment was address in our email correspondence December 5, 2019, attached to this submittal (see Attachment 15). The volume was approved as initially calculated and was not revised.

2. Attachment 5 Stormwater Calculations: Add a cut sheet and rating curve for a custom vortex restrictor that will discharge 0.22 cfs under the proposed head conditions.

Response: Revised submittal includes a cut sheet and a rating curve for a F1214 Vortex regulator with a 4" discharge orifice from Contech Engineered Solutions LLC, a City of Chicago approved vendor (see Attachment 16).

3. C-11 Grading and Storm Sewer Plan: Specify the manufacturer, model and size of the custom vortex restrictor.

Response: A revised version of Sheet C-11 is included in this submittal, showing the manufacturer, model and size of the Vortex regulator.



4. The removal of public sewer and installation of public sewer and public MHs must be approved directly by DWM, Sewer Design Section. Please submit this design to Sid Osakada or provide evidence that you are already in the review process with them.

Response: Design have been submitted to DWM and is under review. An email from Abigail Mazza from the Department of Assets, Information and Services (AIS) confirming the submittal is attached (see Attachment 17).

5. C-11 Grading and Storm Sewer Plan: Move CB-1 a few feet south outside the right-of-way.

Response: Catch basin CB-1 have been moved outside the right-of-way on revised Sheets C-11 and S-6.

6. C-14 O&M Plan: Add the owner's certification statement (DWM Standard Detail A.108) with owner's signature and notary.

Response: Owner's certification statement with owner's signature and notary was submitted with the paper copy of the original submittal. However, an updated version has been included with this submittal.

7. C-14 O&M Plan: List the name and contact information for the individual responsible for ongoing maintenance following construction.

Response: O&M Plan and Sheet C-14 have been revised, including contact information for the individual responsible for ongoing maintenance following the construction.

8. C-14 O&M Plan: Changes to the vortex restrictor and CB-1 location must be reflected on this plan. The custom vortex restrictor manufacturer, model and size must be clearly called out.

Response: A revised version of sheet C-14 is included in this submittal, showing the manufacturer, model and size of the Vortex regulator and the change of location outside the right-of-way of catch basin CB-1. The information has also been added in revised O&M plan attached.

9. Provide the SESC Affidavit and Infiltration Affidavit when submitting final documents.

Response: AIS has not yet retained a general contractor. The completed and signed SESC Affidavit and Infiltration Affidavit will be submitted once a general contractor is retained.

10. Revise and resubmit a hard copy of the entire plan set and calculations.

This revised Stormwater Management Plan includes the following attachment:

- Attachment 1: Copy of \$1,000 review fee payment for DOB review of regulated development outside of right-of-way area smaller than 50,000 square feet. (unchanged)
- Attachment 2: Sewer and drain atlases of the area (unchanged)
- Attachment 3: Surveys of the area (unchanged)
- Attachment 4: CDOT restoration waiver approval (unchanged)
- Attachment 5: Chicago stormwater calculation spreadsheet, including tabs 0.0 "release rate," 1.0 "rate control," 2.0 "volume control" and 2.1.9 "oversized detention" (unchanged)



- Attachment 6: Watershed limits (unchanged)
- Attachment 7: Detention basin volume calculation (unchanged)
- Attachment 8: Pervious and impervious proposed areas (Plan Sheet C-13) (unchanged)
- Attachment 9: Geotechnical report (unchanged)
- Attachment 10: Appendix II-C "Affidavit in support of soil erosion and sediment control Measures during construction" (unchanged, to be submitted once general contractor is retained)
- Attachment 11: Operation and Maintenance Plan, including owner's certification statement *Revised*
- Attachment 12 Tetra Tech Memo "Summary of Sewer Cleanout and Investigation" (unchanged)
- Attachment 13: Plan Sheets
 - Sheet C-1 Cover Sheet
 - o Sheet C-2 Existing Conditions (Topographic and Utility Survey) (unchanged)
 - Sheet C-3 Soil Analytical Results (not submitted for SWP)
 - o Sheet C-4 Extent of Subsurface Exceedance (not submitted for SWP)
 - o Sheet C-5 Geological Cross Sections (not submitted for SWP)
 - o Sheet C-6 Decommissioning Plan (Demolition Plan) (unchanged)
 - o Sheet C-7 Excavation Plan (Demolition Plan) (unchanged)
 - o Sheet C-8 Proposed Remediation Site Layout (not submitted for SWP)
 - o Sheet C-9 Proposed Erosion Control Plan (unchanged)
 - o Sheet C-10 Erosion Control Detail (unchanged)
 - o Sheet C-11 Grading Plan and Proposed Storm Sewer (Utility Plan) Revised
 - Sheet C-12 Storm Sewer Details (unchanged)
 - o Sheet C-13 Restoration Plan (unchanged)
 - Sheet C-14 Operation and Maintenance Plan Revised
 - Sheet T-1 Sign Location and Maintenance of Traffic Plan (not submitted for SWP)
 - o Sheet T-2 Sign Type 1 (not submitted for SWP)
 - o Sheet T-3 Sign Type R11 and OM4 (not submitted for SWP)
 - Sheet T-4 Sign Mounting Band Mount and Fence Mount (not submitted for SWP)
 - o Sheet T-5 Sign Mounting New Sign Posts (not submitted for SWP)
 - o Sheet T-6 Sign Mounting at Concrete Barrier (not submitted for SWP)
 - o Sheet S-1 (Sheetpile) Site Plan (not submitted for SWP)
 - o Sheet S-2 (Sheetpile) Excavation Plan (not submitted for SWP)
 - o Sheet S-3 (Sheetpile) Excavation Cross Sections (not submitted for SWP)
 - Sheet S-4 (Sheetpile) Temporary Retaining Wall (not submitted for SWP)
 - Sheet S-5 (Sheetpile) Temporary Retaining Wall Details (not submitted for SWP)
 - Sheet S-6 (Sheetpile) Sewer Plan and Profile Revised
 - o Sheet S-7 (Sheetpile) City of Chicago Standard Details (unchanged)
- Attachment 14: Memorandum from the City of Chicago Department of Buildings "Review of Design Plans, dated: July 2019", from December 03, 2019
- Attachment 15: Email addressing Comment No. 1 between Tetra Tech and the City of Chicago Department of Buildings
- Attachment 16: Vortex regulator details, rating curve and cut sheet



• Attachment 17: Email from Department of Assets, Information and Assets (AIS) to Tetra Tech confirming the submittal to DWM for review

We appreciate your timely review of the revised SWP. Please contact me at (312) 201-7480 or kris.schnoes@tetratech.com if you have any questions or require additional information.

Sincerely,

Kristine Schnoes

Project Manager

cc: Abigail Mazza, 2FM

Kristine K. Schnoes

Stacey Durley, Tetra Tech Marika Couture, Tetra Tech Carol Nissen, Tetra Tech <u>ATTACHMENT 1 – COPY OF \$1,000 REVIEW FEE PAYMENT</u>

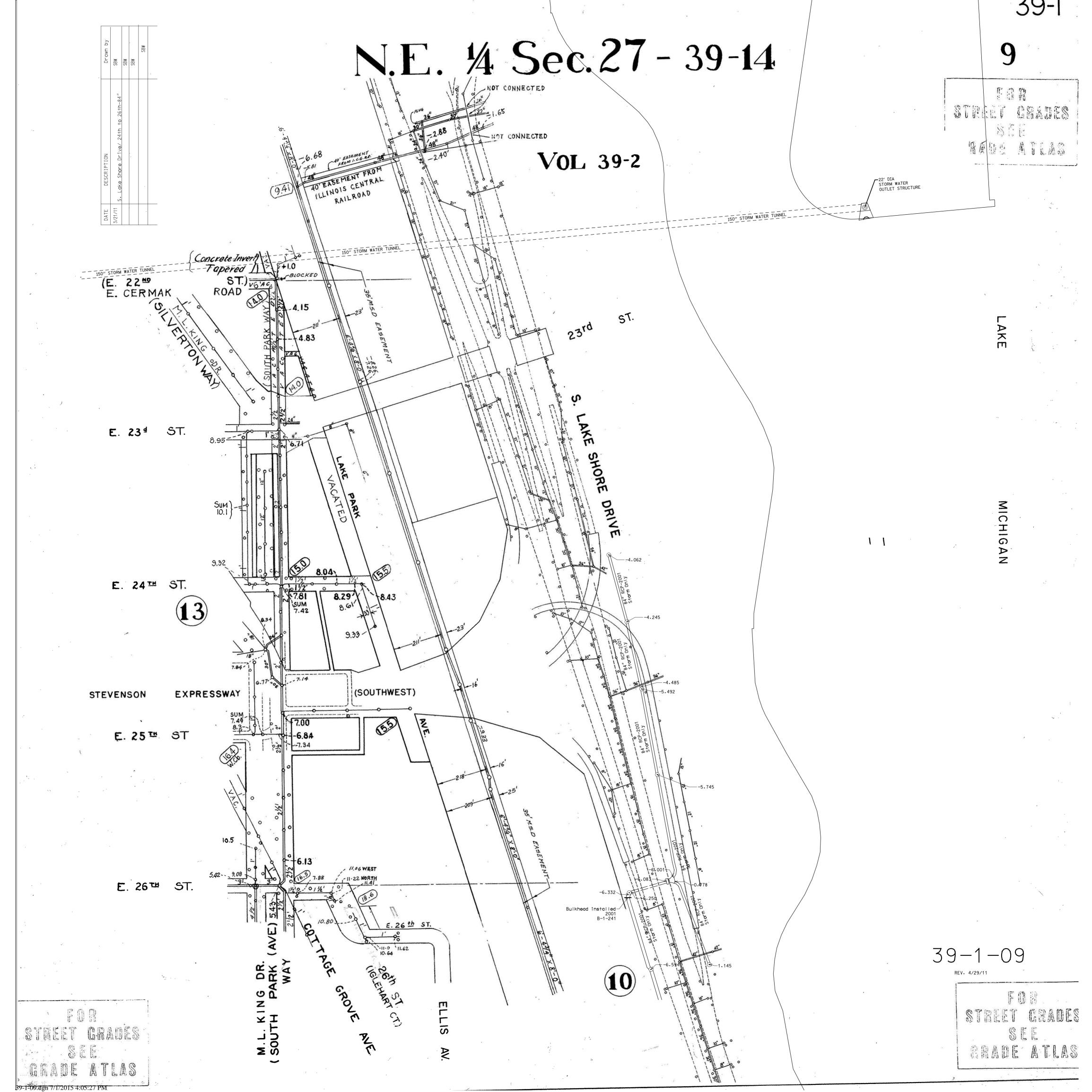


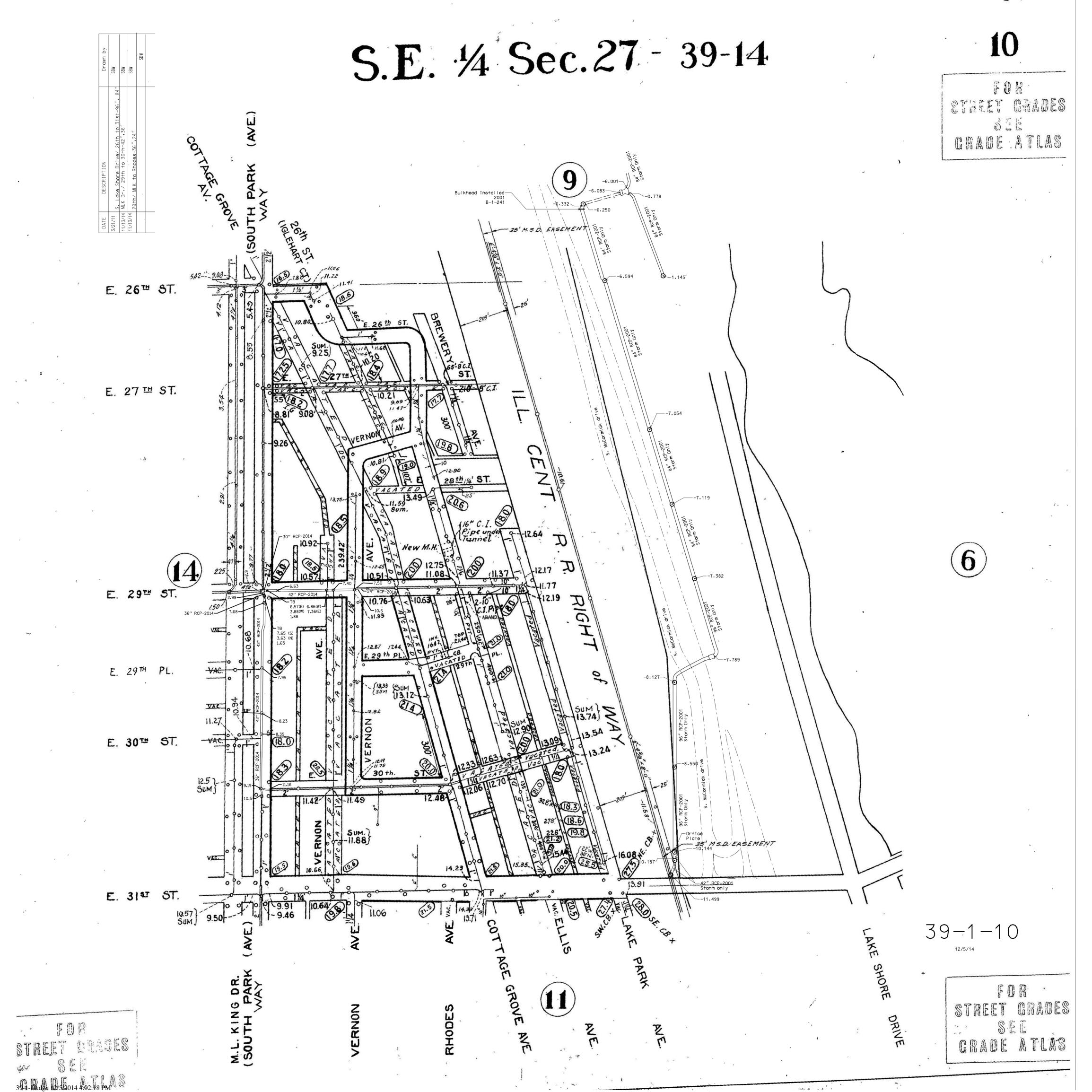
THE FACE OF THIS DOCUMENT CONTAINS A VOID PANTOGRAPH AND MICROPRIMTING WELLS FA GO BA K, 197022897 TETRA TECH, INC 3475 E. Foothill Blvd. Pasadena CA 91107-6024 626.470.2300 Positive Pay rotected 56-382/412 DATE 07/11/2019 Pay One Thousand Only Dollars ****\$1,000.00 CITY OF CHICAGO TO **DEPT OF BLDGS** THE 2240 W OGDEN AVE 2ND FL ORDER CHICAGO, IL 60612

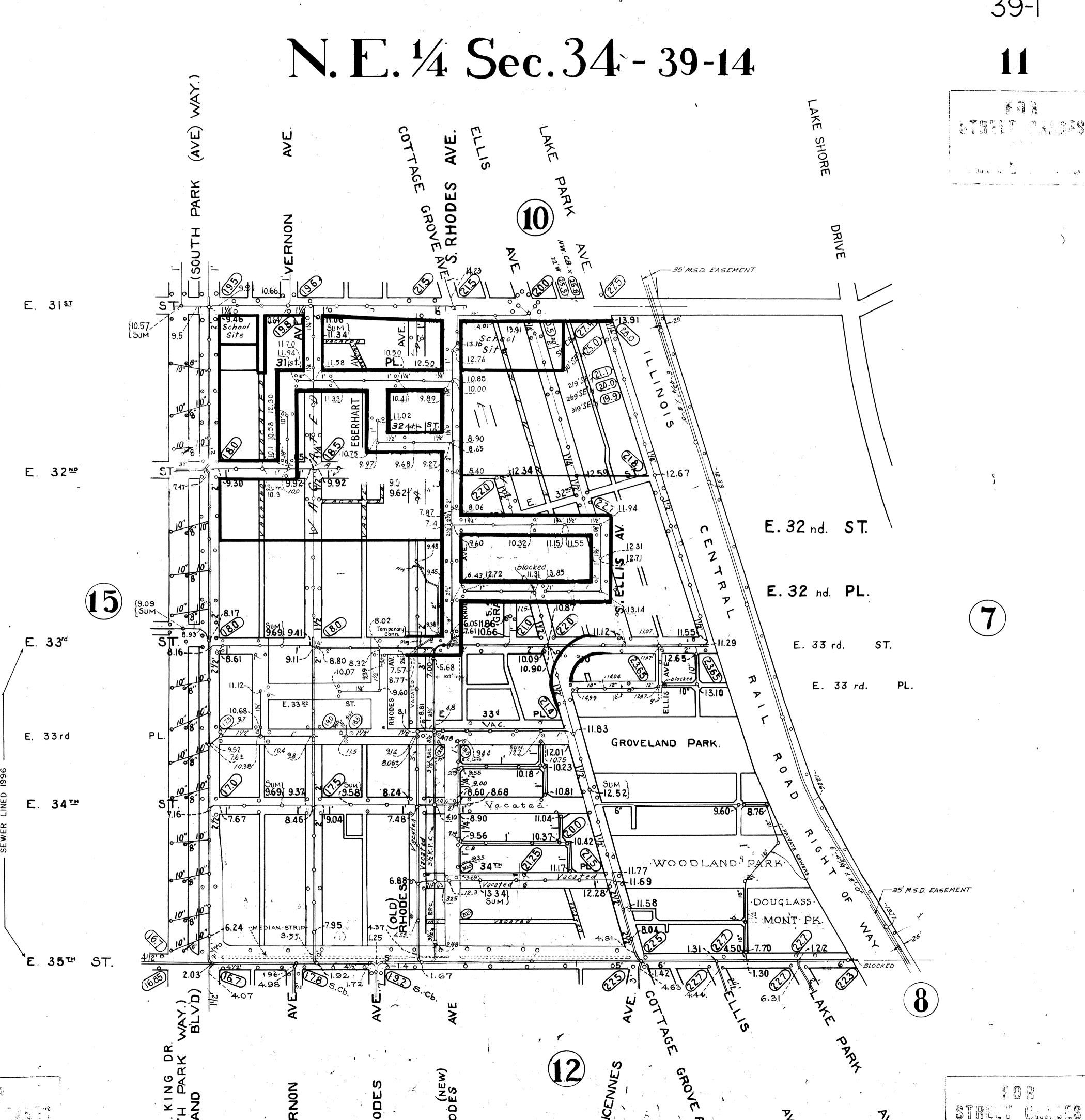
197022897# #O41203824#9600048505#

 $\underline{ATTACHMENT\ 2-SEWER\ AND\ DRAIN\ ATLASES\ OF\ THE\ AREA}$







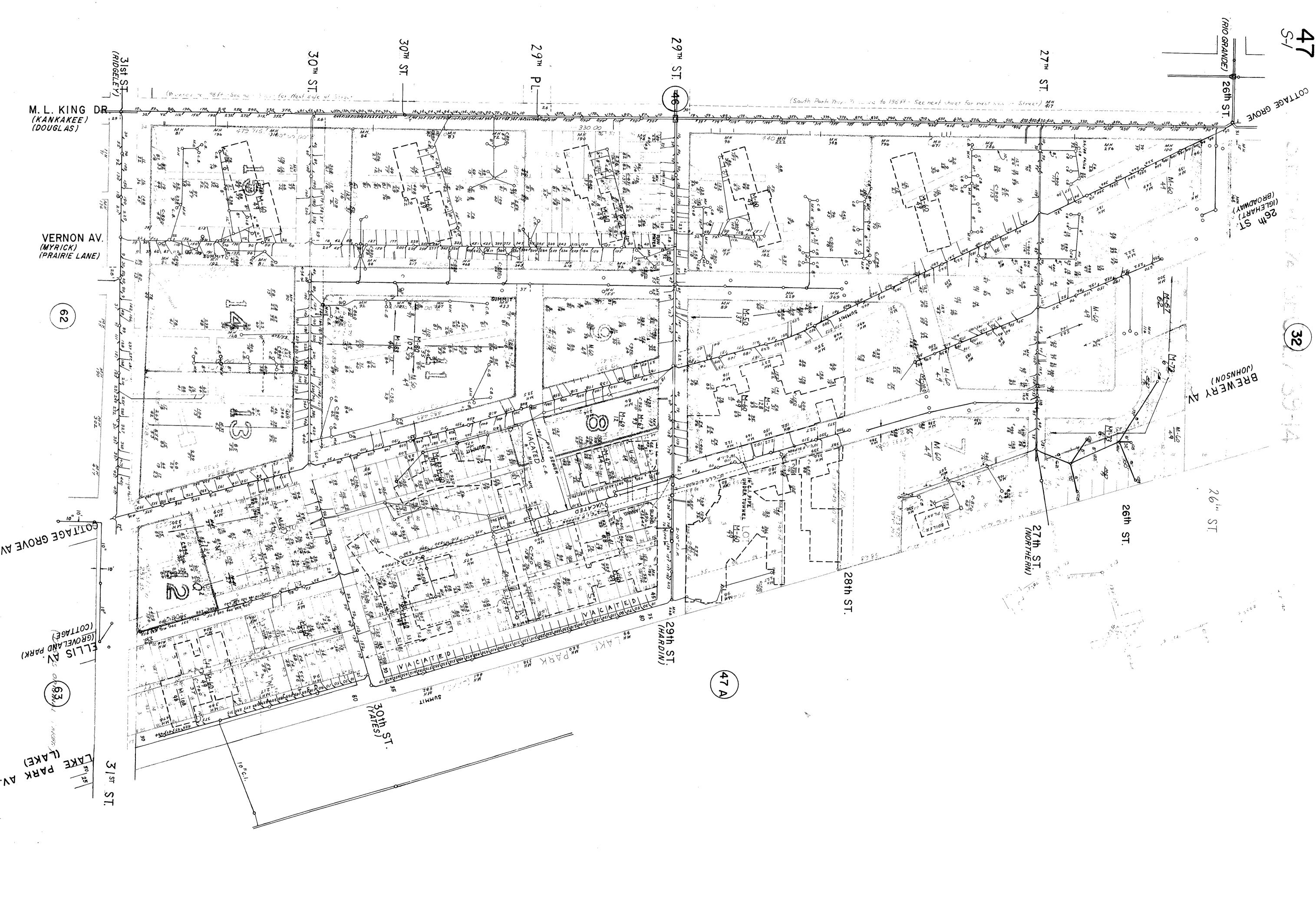


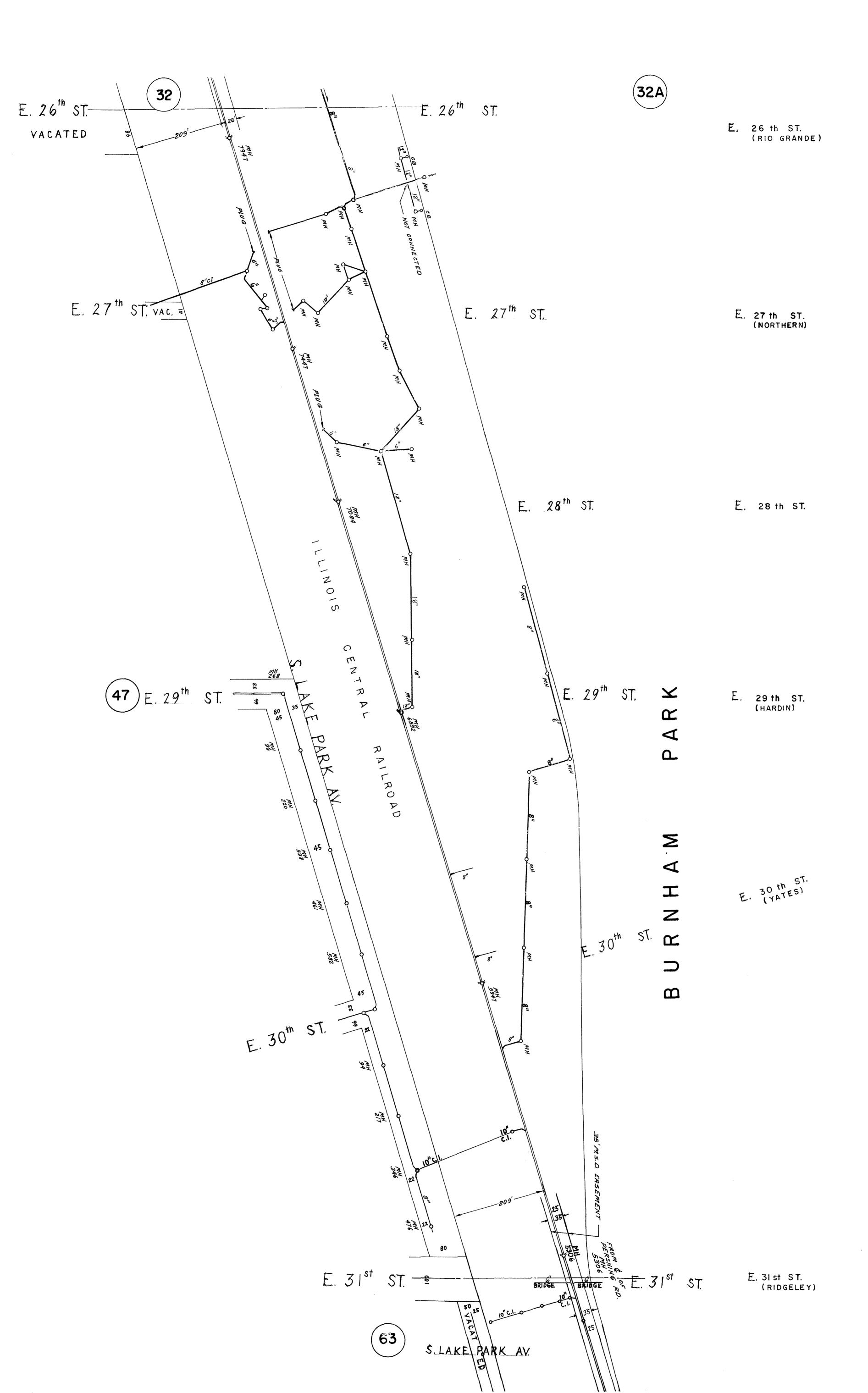


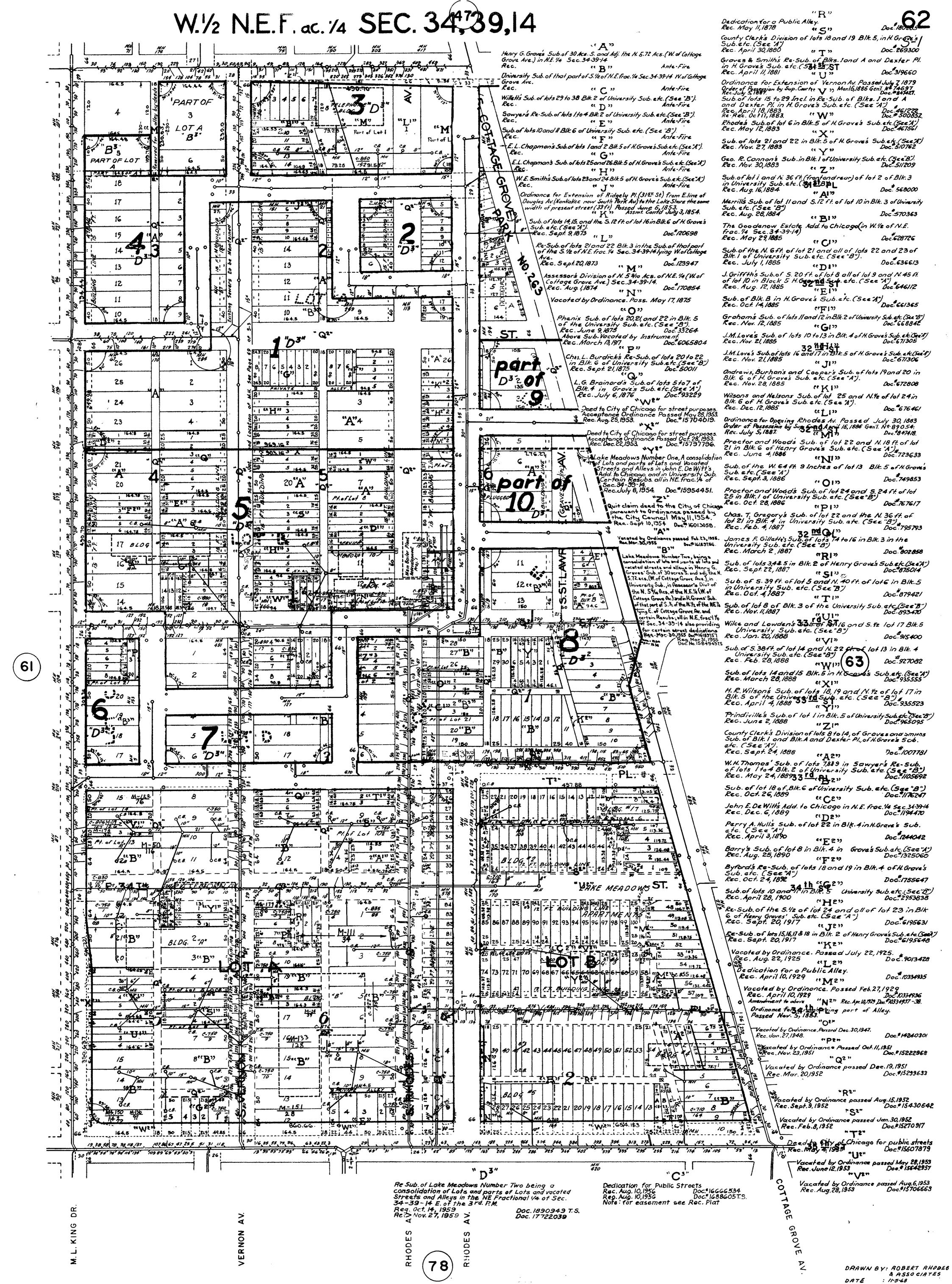
LAKE MICHIGAN

(47)

DRAWN BY: BUREAU OF SEWERS DATE : 11-17-1969 CHECKED BY:







& ASSOCIATES

CHECKED BY: C.ME DATE: 10-1-68

<u>ATTACHMENT 3 – SURVEYS OF THE AREA</u>

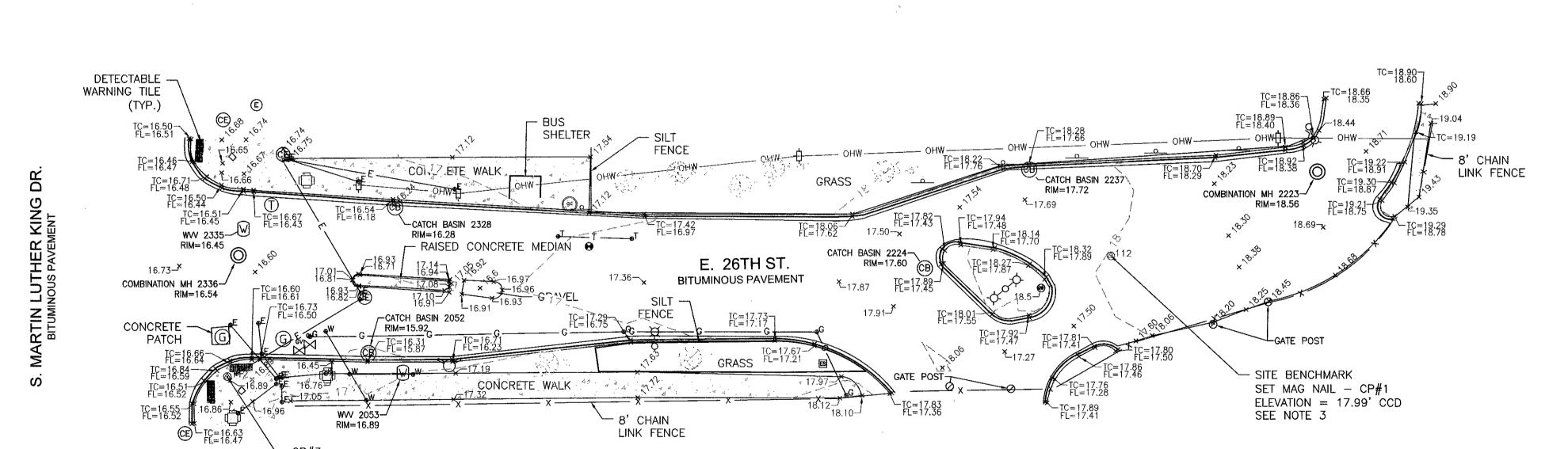


PLAT OF TOPOGRAPHY

SURVEYOR'S NOTES

- 1. ALL DIMENSIONS ARE GIVEN IN FEET AND DECIMAL PARTS THEREOF.
- 2. BEARINGS BASED ON ILLINOIS STATE PLANE COORDINATES, EAST ZONE, NAD83(2011), GPS DERIVED. 3. VERTICAL DATUM IS BASED ON NAVD88 VALUES CONVERTED TO CITY OF CHICAGO DATUM BY A
- 4. ONLY THOSE BUILDING LINE SETBACKS AND EASEMENTS WHICH ARE SHOWN ON THE RECORDED PLAT OF SUBDIVISION ARE SHOWN HEREON, UNLESS OTHERWISE INDICATED. REFER TO THE DEED, TITLE INSURANCE POLICY AND LOCAL ORDINANCES FOR OTHER RESTRICTIONS WHICH MAY OR MAY NOT
- 5. COMPARE DEED DESCRIPTION AND SITE CONDITIONS WITH THE DATA GIVEN ON THIS PLAT AND REPORT ANY DISCREPANCIES TO THE SURVEYOR AT ONCE.
- 6. NO DIMENSIONS SHALL BE DERIVED FROM SCALE MEASUREMENT.
- 7. DISTANCES ALONG CURVES ARE ARC DISTANCES UNLESS OTHERWISE NOTED. 8. THIS SURVEY WAS PERFORMED ON THE GROUND AND COMPLETED 12/20/18.
- 9. ONLY THE IMPROVEMENTS THAT WERE VISIBLE FROM ABOVE GROUND AT TIME OF SURVEY AND THROUGH A NORMAL SEARCH AND WALK THROUGH OF THE SITE ARE SHOWN ON THE FACE OF THIS
- PLAT. LAWN SPRINKLER SYSTEMS, IF ANY, ARE NOT SHOWN ON THIS SURVEY. 10. SURFACE INDICATIONS OF UTILITIES ON THE SURVEYED PARCEL HAVE BEEN SHOWN, UNDERGROUND AND OFFSITE OBSERVATIONS HAVE NOT BEEN MADE TO DETERMINE THE EXTENT OF UTILITIES SERVING OR EXISTING ON THE PROPERTY. PUBLIC AND/OR PRIVATE RECORDS HAVE NOT BEEN SEARCHED TO PROVIDE ADDITIONAL INFORMATION. OVERHEAD WIRES, IF ANY, ARE EXISTING AND THEIR POLES HAVE
- BEEN SHOWN, HOWEVER THEIR FUNCTION AND DIMENSIONS HAVE NOT BEEN NOTED.

11. OTHER THAN VISIBLE OBSERVATIONS NOTED HEREON, THIS SURVEY MAKES NO STATEMENT
REGARDING THE ACTUAL PRESENCE OR ABSENCE OF ANY SERVICE OR UTILITY LINE. CONTROLLED
UNDERGROUND EXPLORATORY EFFORT TOGETHER WITH DIGGER IS RECOMMENDED TO DETERMINE
THE FULL EXTENT OF UNDERGROUND SERVICE AND UTILITY LINES. CONTACT DIGGER AT 1-312-744-7000.



CONTROL POINT BENCHMARK LOCATION ∆BM MONITORING WELL **BORING LOCATION WATER BUFFALO BOX** ВW WATER METER \bowtie WATER VALVE W WATER VALVE VAULT 8 FIRE HYDRANT SPRINKLER CONTROL VALVE **ELECTRIC SERVICE BOX ELECTRIC METER** ∞ LIGHT POLE **POWER POLE** _____o TRAFFIC SIGNAL W/ MAST ARM TRAFFIC SIGNAL POLE **GAS VALVE** ⊗GAS **GAS BUFFALO BOX CATCH BASIN ROUND** COMBINATION SEWER MANHOLE COMED MANHOLE CE COMED VAULT Œ CITY ELECTRIC MANHOLE G **GAS MANHOLE** (T) COMMUNICATION MANHOLE PAINTED ELECTRIC LINE PAINTED WATER LINE **PAINTED GAS LINE** PAINTED TELEPHONE LINE BOLLARD (::) **DECIDUOUS TREE W/SIZE *** 0" CONIFEROUS TREE W/SIZE X 100.00 SPOT GRADE TOP OF CURB FL FLOW LINE T/D TOP OF DEBRIS TOP OF VALVE T/P TOP OF PIPE BOTTOM **OVERHEAD WIRES** ——— oнw WATERMAIN ------W------COMMUNICATION LINE _____T ____ ----- G -----GAS MAIN ELECTRIC LINE -----E-----CONCRETE CURB & GUTTER

LEGEND

SYMBOL

DESCRIPTION

STATE OF ILLINOIS)

COUNTY OF COOK)

WE, ENVIRONMENTAL DESIGN INTERNATIONAL, INC., ILLINOIS PROFESSIONAL DESIGN FIRM NO. 184-001224, HEREBY CERTIFY THAT THE TOPOGRAPHIC IMPROVEMENTS DEPICTED HEREON WERE SURVEYED UNDER THE DIRECT SUPERVISION OF AN ILLINOIS PROFESSIONAL LAND SURVEYOR, AND THAT THIS PLAT REPRESENTS THE CONDITIONS FOUND AT THE TIME

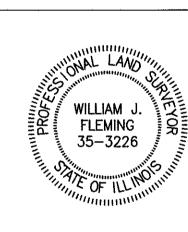
LICENSE EXPIRES: 11/30/2020

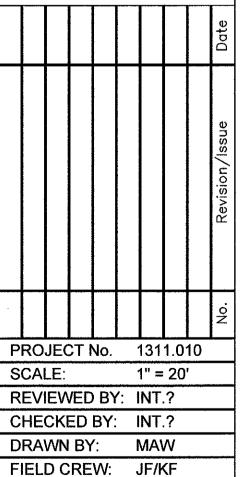
GIVEN UNDER MY HAND AND SEAL THIS 25TH DAY OF FEBRUARY, 2019 IN CHICAGO, ILLINOIS.

WILLIAM J. FLEMING, IPLS NO. 035.003226

THIS PROFESSIONAL SERVICE CONFORMS TO THE CURRENT ILLINOIS MINIMUM STANDARDS FOR TOPOGRAPHIC SURVEYS.

THIS PLAT IS VALID ONLY WITH AN ORIGINAL SIGNATURE AND WITH A RAISED SEAL.



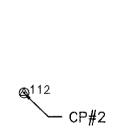


12/20/18

FIELD DATE: DATE: 2/25/19

PLAN TYPE TOPOGRAPHIC SURVEY

SHEET NUMBER 1 of 1



CONTROL POINTS

CP#1 SET MAG NAIL

CP#3 SET CUT CROSS

1887162.249

1179615.184

CP#2 SET CUT CROSS IN MEDIAN 1887121.567

1179291.564

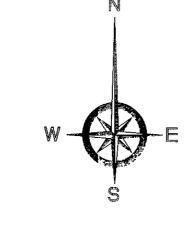
1887129.805

1179379.558

PLAT OF TOPOGRAPHY

SURVEYOR'S NOTES

- 1. ALL DIMENSIONS ARE GIVEN IN FEET AND DECIMAL PARTS THEREOF.
- 2. BEARINGS BASED ON ILLINOIS STATE PLANE COORDINATES, EAST ZONE, NAD83(2011), GPS DERIVED. 3. VERTICAL DATUM IS BASED ON NAVD88 VALUES CONVERTED TO CITY OF CHICAGO DATUM BY A
- DIFFERENCE OF -579.16'. 4. ONLY THOSE BUILDING LINE SETBACKS AND EASEMENTS WHICH ARE SHOWN ON THE RECORDED PLAT OF SUBDIVISION ARE SHOWN HEREON, UNLESS OTHERWISE INDICATED. REFER TO THE DEED, TITLE INSURANCE POLICY AND LOCAL ORDINANCES FOR OTHER RESTRICTIONS WHICH MAY OR MAY NOT
- 5. COMPARE DEED DESCRIPTION AND SITE CONDITIONS WITH THE DATA GIVEN ON THIS PLAT AND REPORT ANY DISCREPANCIES TO THE SURVEYOR AT ONCE.
- 6. NO DIMENSIONS SHALL BE DERIVED FROM SCALE MEASUREMENT.
- 7. DISTANCES ALONG CURVES ARE ARC DISTANCES UNLESS OTHERWISE NOTED.
- 8. THIS SURVEY WAS PERFORMED ON THE GROUND AND COMPLETED 8/20/18. THROUGH A NORMAL SEARCH AND WALK THROUGH OF THE SITE ARE SHOWN ON THE FACE OF THIS
- PLAT. LAWN SPRINKLER SYSTEMS, IF ANY, ARE NOT SHOWN ON THIS SURVEY 10. SURFACE INDICATIONS OF UTILITIES ON THE SURVEYED PARCEL HAVE BEEN SHOWN, UNDERGROUND AND OFFSITE OBSERVATIONS HAVE NOT BEEN MADE TO DETERMINE THE EXTENT OF UTILITIES SERVING
- REGARDING THE ACTUAL PRESENCE OR ABSENCE OF ANY SERVICE OR UTILITY LINE. CONTROLLED UNDERGROUND EXPLORATORY EFFORT TOGETHER WITH DIGGER IS RECOMMENDED TO DETERMINE THE FULL EXTENT OF UNDERGROUND SERVICE AND UTILITY LINES. CONTACT DIGGER AT 1-312-744-7000.





0'	;	30'	60'
	72	and the state of t	
	Scale:	1"=30'	

©	CONTROL POINT
≜ ВМ	BENCHMARK LOCATION
€	MONITORING WELL
0	BORING LOCATION
⊗as	WATER BUFFALO BOX
ĠW	WATER METER
₩	WATER VALVE
W	WATER VALVE VAULT
۵	FIRE HYDRANT
SCA	SPRINKLER CONTROL VALVE
ES	ELECTRIC SERVICE BOX
ΩE	ELECTRIC METER
¤- 0	LIGHT POLE
†	POWER POLE
<u>IXIXXI</u>	TRAFFIC SIGNAL W/ MAST ARM
0	TRAFFIC SIGNAL POLE
ev S	GAS VALVE
⊗GAS	GAS BUFFALO BOX
(CB)	CATCH BASIN ROUND
0	COMBINATION SEWER MANHOLE
Œ	COMED MANHOLE
Œ	COMED VAULT
(E)	CITY ELECTRIC MANHOLE
G	GAS MANHOLE
Ū	COMMUNICATION MANHOLE
⊕ E.	PAINTED ELECTRIC LINE
⊕W	PAINTED WATER LINE
⊕G	PAINTED GAS LINE
ΦŢ	PAINTED TELEPHONE LINE
•	BOLLARD
- -	SIGN
	SHRUB
ૄ 0"	DECIDUOUS TREE W/SIZE
* 0"	CONIFEROUS TREE W/SIZE
X 100.00	SPOT GRADE
тс	TOP OF CURB
FL	FLOW LINE
T/D	TOP OF DEBRIS
T/V	TOP OF VALVE
Т/Р	TOP OF PIPE
вот	воттом
——— OH₩———	OVERHEAD WIRES
W	
——— T ———	WATERMAIN
	COMMUNICATION LINE
G	
G	COMMUNICATION LINE
	COMMUNICATION LINE GAS MAIN

LEGEND

DESCRIPTION

FLEMING



1" = 20'

PROJECT No. 1311.010 SCALE: REVIEWED BY: WJF

CHECKED BY:

DRAWN BY: MAW FIELD CREW: JF/JW | FIELD DATE: 8/20/19

DATE: 10/10/19 PLAN TYPE TOPOGRAPHIC SURVEY

1 of 1

SHEET NUMBER

-CATCH BASIN 2502 RIM=16.09 COMBINATION MH 2503 COMBINATION MH 2626-EDGE OF CONCRETE FOUNDATION (PARTIALLY BURIED) RIM = 18.28STRUCTURE # STRUCTURE TYPE RIM ELEVATION STRUCTURE INFO W-WV 2335 INV=12.23 10" UNKNOWN MAT. (NW) T/PIPE=11.56 12" DIP (E/W) WATER VALVE VAULT T/VALVE NUT=13.87 INV=12.07 8" VCP (W) COMBINATION MH SITE BENCHMARK INV=12.05 8" VCP (NE) SET MAG NAIL - CP#1 INV=10.89 18" VCP (NW) ELEVATION = 17.99' CCD INV=12.09 10" VCP (NE) SEE NOTE 3 2224 COMBINATION MH 17.60 LINK FENCE INV=10.91 18" VCP (E) INV=11.03 15" VCP (SE) INV=13.59 10" VCP (SW) 2237 CATCH BASIN INV=11.26 12" RESTRICTOR CAP (S) CATCH BASIN T/VALVE NUT=13.36 2335 WATER VALVE VAULT T/DEBRIS=11.46 PIPE COVERED BY DEBRIS INV=7.85 18" VCP (W) **COMBINATION MH** INV=7.73 18" VCP (E) INV=11.78 8" VCP (S) CATCH BASIN STATE OF ILLINOIS) T/DEBRIS=11.28 INV=11.60 8" VCP (W) COUNTY OF COOK) INV=11.49 8" VCP (S)

COMBINATION MH

COMBINATION MH

CATCH BASIN

SANITARY MH

COMBINATION MH

18.28

17.11

INV=11.56 8" VCP (SE)

INV=11.56 8" VCP (E)

INV=11.58 8" VCP (NE)

INV=12.57 8" VCP (SW)

INV=13.26 (6" VCP (NE)

INV=12.57 8" VCP €

INV=11.846" VCP (E)

T/DEBRIS=12.45

INV=11.159" VCP (N)

INV=11.35 10" VCP (E) INV=11.22 8" VCP (SE)

INV=11.55 8" VCP (N)

INV=11.65 8" VCP (W)

2503

2585

2625

CONTROL POINTS

CP#1 SET MAG NAIL 1887162.249

CP#2 SET CUT CROSS IN MEDIAN

1179615.184

COMBINATION MH 2336-

RIM = 16.54

1887121.567 1179291.564

1887129.805

CP#3 SET CUT CROSS 1179379.558

THIS PROFESSIONAL SERVICE CONFORMS TO THE CURRENT ILLINOIS MINIMUM STANDARDS FOR TOPOGRAPHIC SURVEYS.

WE, ENVIRONMENTAL DESIGN INTERNATIONAL, INC., ILLINOIS PROFESSIONAL DESIGN FIRM

HEREON WERE SURVEYED UNDER THE DIRECT SUPERVISION OF AN ILLINOIS PROFESSIONAL

LAND SURVEYOR, AND THAT THIS PLAT REPRESENTS THE CONDITIONS FOUND AT THE TIME

GIVEN UNDER MY HAND AND SEAL THIS 10TH DAY OF OCTOBER, 2019 IN CHICAGO, ILLINOIS.

NO. 184-001224, HEREBY CERTIFY THAT THE TOPOGRAPHIC IMPROVEMENTS DEPICTED

GREMLEY & BIEDERMANN

PLCS Corporation
LICENSE No. 184-005332

PROFESSIONAL LAND SURVEYORS

4505 NORTH ELSTON AVENUE, CHICAGO, IL 60630
TELEPHONE: (773) 685-5102 FAX: (773) 286-4184 EMAIL: INFO@PLCS-SURVEY.COM

Parcel 1: Plat of Survey

That part of the Northeast fractional Quarter of Section 27, Township 39
North, Range 14, East of the Third Principal Meridian, bounded and described as follows:

Beginning at a point of intersection with the South line of said fractional Quarter and the Westerly line of Lot 1 in Chicago Land Clearance Commission No. 2, being a consolidation of Lots and parts of Lots and vacated streets and alleys in the Southeast fractional Quarter of said Section 27; Thence Northwesterly along the Northwest extension of the Westerly line of said Lot 1, a distance of 10.95 feet to a point 9.82 feet North of the South line of said Northeast fractional Quarter and 33.00 feet East of the West line of said Northeast fractional Quarter; Thence North along a line 33.00 feet East of and parallel with the West line of said Northeast fractional Quarter, being the East line of South Dr. Martin Luther King, Jr. Drive, a distance of 389.38 feet; Thence East along a line 175.00 feet South of and parallel with the North line of the South 8.70 chains of said Northeast fractional Quarter, a distance of 275.00 feet; Thence North along a line 275.00 feet East of and parallel with the East line of said South Dr. Martin Luther King, Jr. Drive, a distance of 175.00 feet to the North line of said South 8.70 chains; Thence East along the North line of said South 8.70 chains, a distance of 321.62 feet; Thence Southeasterly along the Southwesterly line of the Illinois Central Gulf Railroad, a distance of 599.17 feet; Thence West along the South line of said Northeast fractional Quarter, a distance of 762.98 feet to the point of beginning, in Cook County, Illinois.

Parcel 2:

That part of Lot 1 in the Assessor's Division of unsubdivided lands in the Southeast fractional Quarter of Section 27, Township 39 North, Range 14, East of the Third Principal Meridian, lying West of the Illinois Central Railroad Company's right of way, bounded and described as follows:

Beginning at a point of intersection of the North line of said Lot 1 and the Illinois Central railroad Company's West right of way line, said point being the Northeast corner of said Lot 1; Thence Southeasterly along said right of way line, a distance of 102.28 feet to the Southeast corner of said Lot 1; Thence Northwesterly on a straight line forming an interior angle of 18° 7' 30" a distance of 75.50 feet to the Northeast corner of the building line of the Swigart Paper Company; Thence Northwesterly on an angle to the left 21° 56' 56" along said building line a distance of 22.84 feet; Thence Westerly on an angle to the left of 13° 3' 30" along said building line, a distance of 65.47 feet to a point in the North line of said Lot 1, which is 93.78 feet West from the Northeast corner of said Lot 1; Thence East along said North line of Lot 1, a distance of 93.78 feet to the point of beginning.

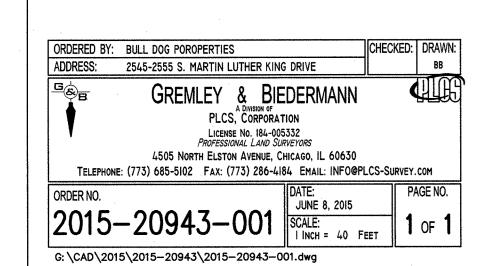
Parcel 3:

Lot 1 Chicago Land Clearance Commission No. 2, being a Consolidation of Lots and parts of Lots and vacated streets and alleys in the Southeast fractional Quarter of Section 27, Township 39 North, Range 14, East of the Third Principal Meridian, in Cook County, Illinois.

Parcel 4:

Lots 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16 except the west 15 feet thereof and all of the vacated 18 ft. public lying west of and adjoining lots 1 through 5 and lying south of and adjoining lots 7 through 16 except the west 15 feet, all in block 4 in Walker Brothers addition to Chicago, being a subdivision of the Northeast Quarter of Section 27, Township 39 North, Range 14, East of the Third Prinicpal Merdian, in Cook County, Iliinois.

AREA OF PROPERTY = 414,857 SQ. FT. OR 9.52 ACRES



SURVEY NOTES:

Note P. & M. denotes Pecord and Measured distances resi

Note R. & M. denotes Record and Measured distances respectively.

Distances are marked in feet and decimal parts thereof. Compare all points BEFORE building by same and at once report any differences BEFORE damage is done.

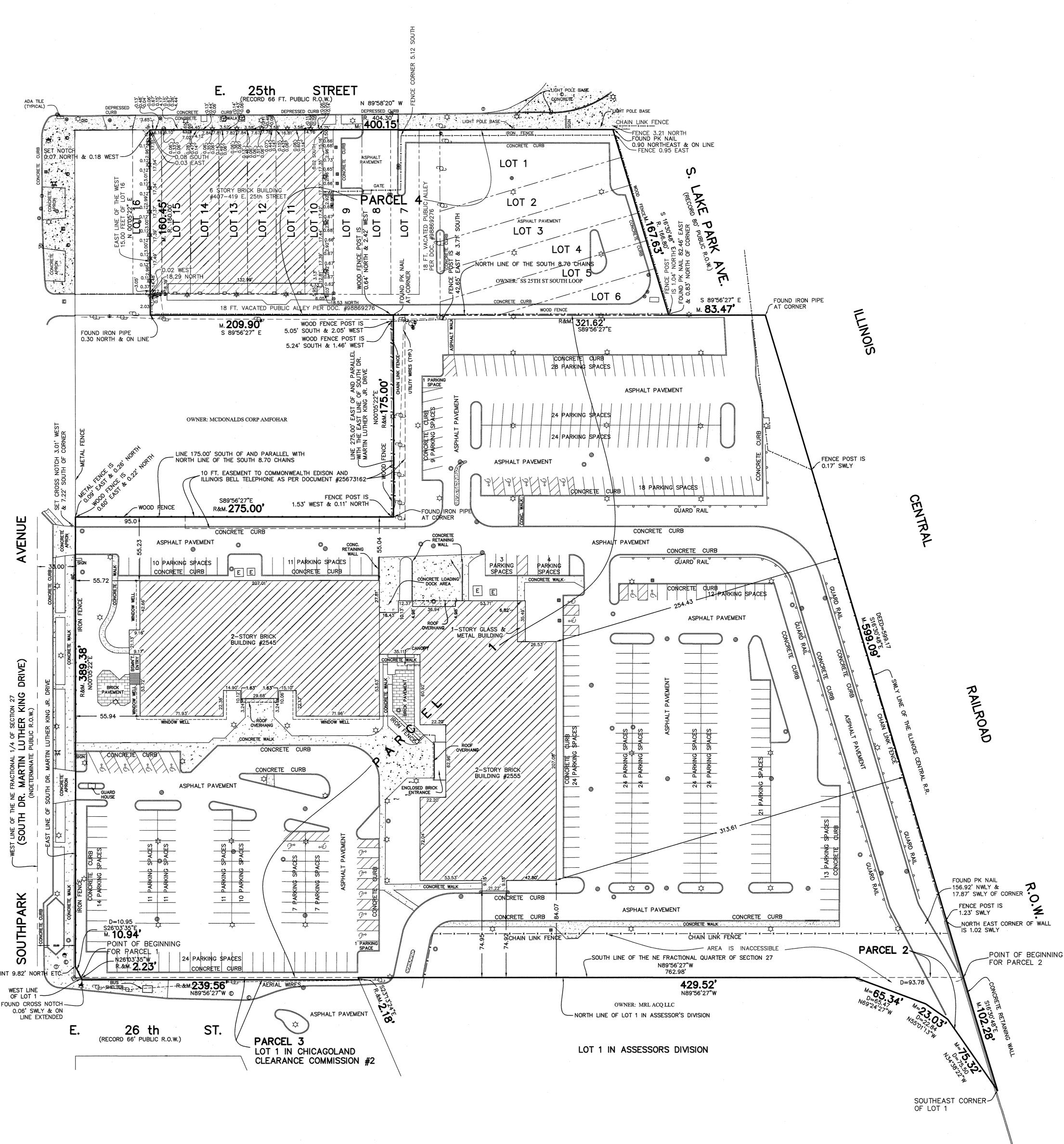
For easements, building lines and other restrictions not shown on survey plat refer to your abstract, deed,

contract, title policy and local building line regulations.

NO dimensions shall be assumed by scale measurement upon this plat.

Unless otherwise noted hereon the Bearing Basis, Elevation Datum and Coordinate Datum if used is ASSUMED.

COPYRIGHT GREMLEY & BIEDERMANN, INC. 2015 "All Rights Reserved"



GRAPHIC SCALE

1" = 40'

Legend:

Storm MH Storm CB Storm Inlet San MH San Storm Combo MH San Clean Out Water Valve Vault Water Buffalo Box Water Hand Hole 📆 Water Meter Water Fire Hydrant Telephone MH Telephone Vault Utility Pole Guy Anchor E Electric Manhole © Electric MH Electric Vault 👸 💮 Electric Hand Hole Electric Pad Electric Meter Electric Light Pole α Electric Traffic Signal Electric Light Pole with Traffic Signal ☑ Electric Traffic Control Box Electric Traffic Vault ★ Electric Ground Light 📽 Gas Buffalo Box 👸 Gas Hand Hole ⊚ Gas Meter ₩ Gas Valve © Gas MH G Gas Vault PPBI Parking Pay Box Sign Post ⊗ Bumper Post MM Bike Rack Unclassified Manhole Auto Sprinkler Me Hose Connection A Fire Alarm

→ Flag Pole

State of Illinois) County of Cook)ss

We, GREMLEY & BIEDERMANN, INC. hereby certify that we have surveyed the above described property and that the plat hereon drawn is a correct representation of said survey corrected to a temperature of 62° Fahrenheit.

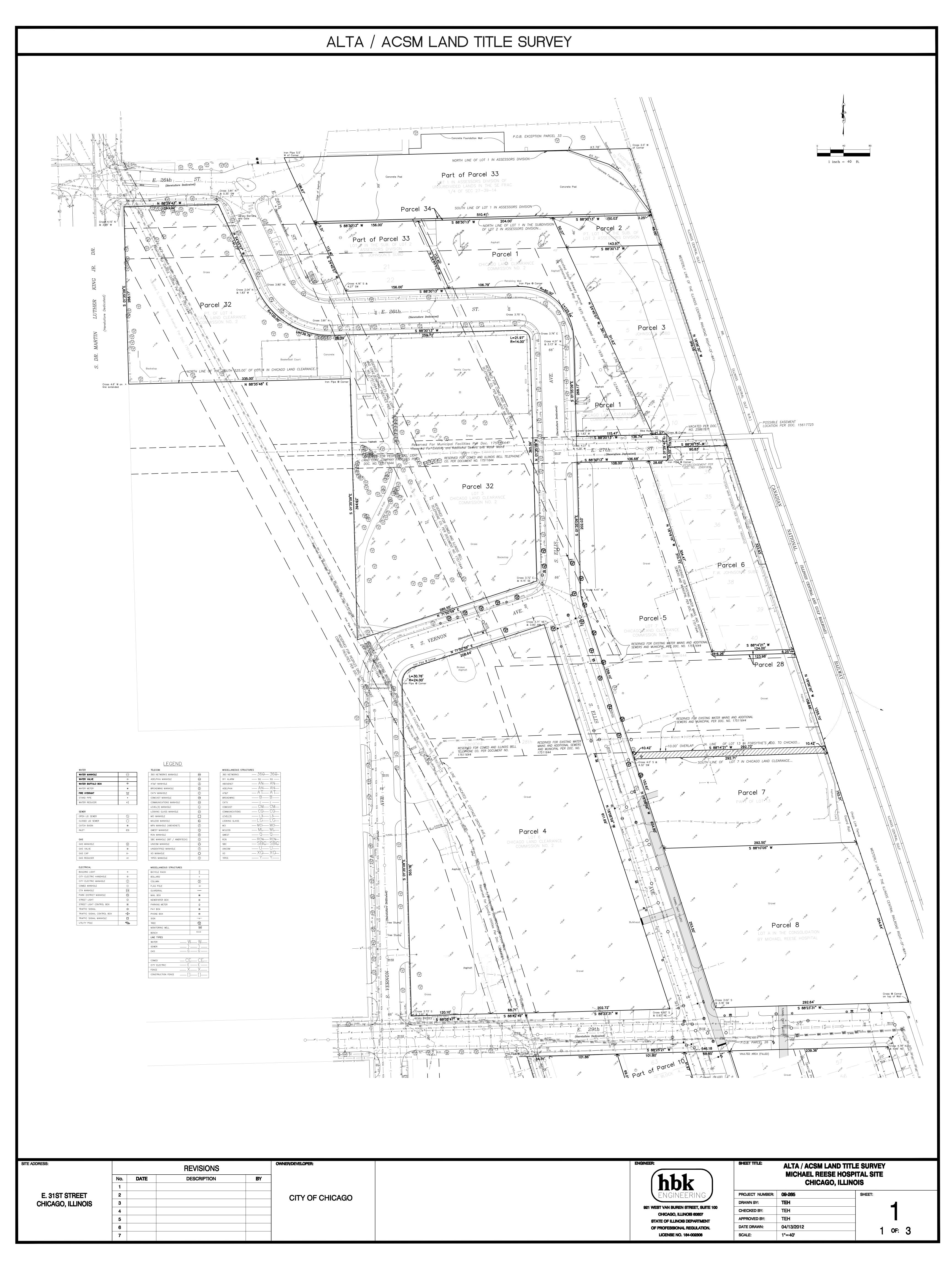
This professional service conforms to the current Illinois minimum standards for a boundary survey.

Field measurements completed on June 8, 2015.

Signed on <u>June 16, 2015</u>.

Professional Illinois Land Surveyor No. <u>Z802</u>
My license expires November 30, 2016





 $\underline{\textbf{ATTACHMENT 4} - \textbf{CDOT RESTORATION WAIVER APPROVAL}}$



Abigail Mazza

From: Michael Simon

Sent: Tuesday, June 11, 2019 4:51 PM

To: Abigail Mazza

Cc: Kimberly Worthington; gkeck2@cdotutilitypmo.org

Subject: Re: Restoration Waiver Request for Carnotite Reduction Company Remediation Project

434 E. 26th Street

Abby:

CDOT - DIM approves the restoration waiver

Mike Simon

Sent from my iPhone

On Jun 11, 2019, at 4:44 PM, Abigail Mazza < Abby. Mazza@cityofchicago.org > wrote:

Good afternoon Mike,

2FM is preparing construction bid documents for remediation of the radiologically contaminated material in the vicinity of 434 E. 26th Street (the Project). The Project requires excavating within the north end of the City-owned former Michael Reese Hospital site (MRH site) as well as within the 26th Street right-of-way (see attached, general excavation area shown in pink).

Currently, the stretch of 26th Street east of Dr. Martin Luther King Jr. Drive (MLK Drive) is open to vehicles only for accessing the private property to the north (2545 S. Dr. Martin Luther King Jr. Drive) via an unpermitted driveway opening (confirmed by CDOT's permit division) and for accessing the northern MRH site entrance gate. The private property to the north also has two permitted driveway entrances along MLK Drive that provide access to their property. Pedestrians can also currently use 26th Street to access the 27th Street Metra station from the north. This station can also be accessed from the south via 29th Street and S. Vernon Avenue, which will continue to be available during and after construction.

DPD is in negotiations with a developer for the future redevelopment of the MRH site, which will include reconfiguring 26th Street. Once remediation is complete, the MRH site will remain vacant until this development occurs. Therefore, 2FM is requesting a waiver from CDOT for the requirement to restore the impacts to 26th Street, including pavement, lighting and sidewalk repair as indicated on the attached general exhibit. If this waiver is approved, 26th Street will be temporarily closed to both vehicle and pedestrian traffic east of S. Dr. Martin Luther King Jr. Drive during and after the Project until development occurs.

2FM and CDOT (Jeff Sriver and Luann Hamilton) met with Metra in April to discuss closing 26th Street and based on the data they shared, the majority of riders come from the south and there was no objection to closing access to the station from the north.

We will be submitting a full set of detailed drawings to OUC in the upcoming weeks as part of our existing facility protection/deep excavation review and are requesting written approval of this waiver to include with the OUC submittal.

Thanks,

Abby Mazza, P.E. | Environmental Engineer III

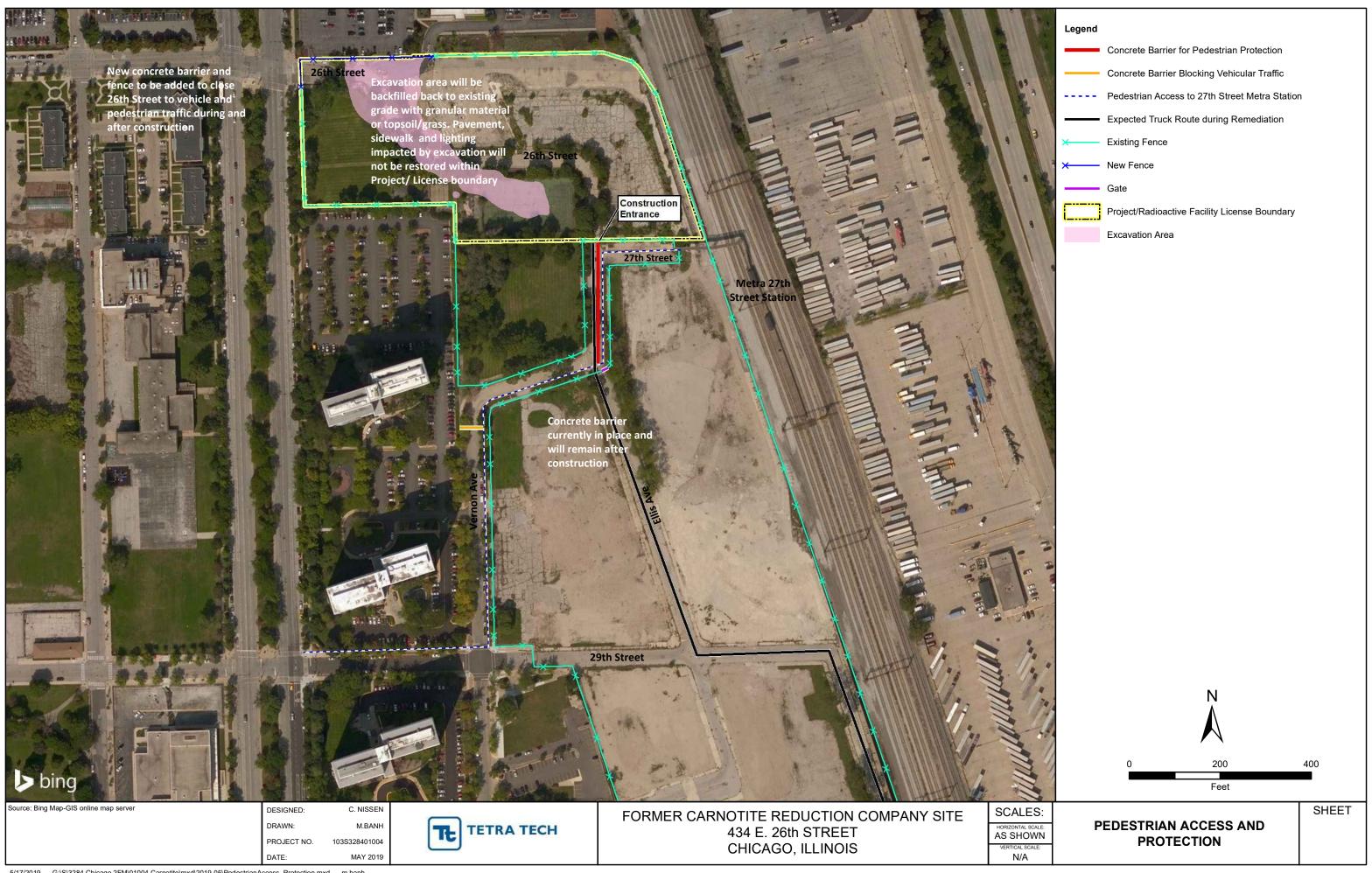
City of Chicago | Department of Fleet and Facility Management (2FM)

Bureau of Environmental, Health & Safety Management (EHS)

30 N. LaSalle St., Suite 300 | Chicago, Illinois 60602-2575

Tel: 312.744.3161 | Fax: 312.744.6451

<Carnotite Waiver Request Exhibit_434 E 26th St_2019-06-05.pdf>



 $\underline{\textbf{ATTACHMENT 5}} - \underline{\textbf{CHICAGO STORMWATER CALCULATION SPREADSHEET}}$



Date:	11/4/19
Rev. Date:	

Stormwater Spreadsheet Tool

Release 3.1 effective January 1, 2016

1. DOB Tracking/Permit Number:
2. Name of Project: Carnotite Radiation Site
3. Address of Site: 434 East 26th Street Chicago, Illinois 60616
Architect / Engineer of Record: Jim Wescott Phone No.: 312-201-7781
Description of Proposed Work: Excavation and removal of contaminated soils, backfill and restoration of excavation area
5. Use of Building (if applicable): N/A
6. Sewer Altas & Drain Atlas Referenced:
7. Area of Site: Square feet (Square Feet = Acres * 43560) 1.406 acres (Acres = Square Feet / 43560)
This spreadsheet tool has been prepared to assist the applicant in preparing calculations for simple sites. The applicant is responsible for ensuring that submitted calculations are correct. If necessary, supporting hand calculations should be prepared and submitted.
Color Coding - Cell Contents Computed by Spreadsheet - Cell for User Entry - Cell Includes Comment (when cursor is over it)

Stormwater Spreadsheet Tool

INDEX OF SPREADSHEETS

- · ·		00//50
Required>>	X	COVER
Required>>	X	INDEX
	X	0.0 RELEASE RATE
Required>>	X	1.0 RATE CONTROL
		1.1 Dry Weather Flow
		1.2 BMPs-Rate Control Credit
		1.3 Orifice Sizing Calculation
Required>>	X	2.0 VOLUME CONTROL
		2.1 BMP Volume Summary
		2.1.1 Bioinfiltration
		2.1.2 Drainage Swales
		2.1.3 Green Roof
		2.1.4 Infiltration Vault
		2.1.5 Trees
		2.1.6 Permeable Pavement
		2.1.7.1 Roof Runoff BMPs - Planter Boxes
		2.1.7.2 Roof Runoff BMPs - Rain Barrels / Cisterns
		2.1.8 Filter Strips
	X	2.1.9 Oversized Detention

 Name of Project:
 Carnotite Radiation Site

 Address:
 434 East 26th Street

 A/E of Record:
 Jim Wescott

0.0 Release Rate

Step 1: Sewer Capacity of Each Sewer Segment						
Sewer Segment:	Segment 1	Segment 2	Segment 3	Segment 4	Segment 5	Segment 6
Street Name:	E 26th St	E 26th St	S MLK Jr Dr	S MLK Jr Dr	S MLK Jr Dr	
Upstream End (street name):	E 26th St	E 26th St	S MLK Jr Dr	S MLK Jr Dr	S MLK Jr Dr	
Downstream End (street name):	E 26th St	S MLK Jr Dr	S MLK Jr Dr	S MLK Jr Dr	S MLK Jr Dr	
Upstream Invert (ft):	11.41	11.06	8.55	6.13	5.43	
Downstream Invert (ft):	11.06	7.88	5.43	5.43	4.12	
Pipe Segment Length (ft):	40	220	150	98	40	
Pipe Slope (S):	0.8750%	1.4455%	2.0800%	0.7143%	3.2750%	
Pipe Characteristics:						
Pipe Size (in):	15	18	30	30	36	
Pipe Area (sq ft):	1.2272	1.7671	4.9087	4.9087	7.0686	0.0000
Wetted Perimeter (ft):	3.9270	4.7124	7.8540	7.8540	9.4248	0.0000
Hydraulic Radius (ft):	0.3125	0.3750	0.6250	0.6250	0.7500	
Roughness Coefficient (n):	0.011	0.011	0.015	0.015	0.015	
Flow Conveyance (K):	62.2099	70.2501	72.4182	72.4182	81.7777	
Manning's Equation:						
Velocity (fps):	5.82	8.45	10.44	6.12	14.80	
Hydraulic Capacity (cfs):	7.14	14.93	51.27	30.04	104.61	
Roughness Coefficient (n):						
VCP: use 0.011,	VCP: use 0.011, typical for pipe <= 21 in					
RCP: use 0.013,	for pipe >=24 in v	vhen pipe size sho	wn on atlas in inch	ies		
brick sewer: use 0.015, for pipe >=24 in when pipe size shown on atlas in feet						

Step 2: Tributary Area to Each Sewer Segment						
	Segment 1			Segment 2		
Total Tributary Area (ac):	0.65	Adj. Factor	Adjusted Area	1.14	Adj. Factor	Adjusted Area
Residential Area (ac):	0.00	1.0	0.00	0.00	1.0	0.00
Commercial Area (ac):	0.65	1.3	0.85	1.14	1.3	1.48
Industrial Area (ac):	0.00	1.5	0.00	0.00	1.5	0.00
	Tota	l Adjusted Area:	0.85	Tota	l Adjusted Area:	1.48
		Segment 3			Segment 4	
Total Tributary Area (ac):	21.20	Adj. Factor	Adjusted Area	17.50	Adj. Factor	Adjusted Area
Residential Area (ac):	0.00	1.0	0.00	0.00	1.0	0.00
Commercial Area (ac):	21.20	1.3	27.56	17.50	1.3	22.75
Industrial Area (ac):		1.5	0.00	0.00	1.5	0.00
	Tota	l Adjusted Area:	27.56	Tota	l Adjusted Area:	22.75
		Segment 5			Segment 6	
Total Tributary Area (ac):	38.70	Adj. Factor	Adjusted Area		Adj. Factor	Adjusted Area
Residential Area (ac):	0.00	1.0	0.00	0.00	1.0	0.00
Commercial Area (ac):	38.70	1.3	50.31		1.3	0.00
Industrial Area (ac):	0.00	1.5	0.00		1.5	0.00
	Tota	l Adjusted Area:	50.31	Tota	l Adjusted Area:	0.00
Note: Total tributary areas entered for segments 1 through 6 must include the cumulative tributary area for each segment. All upstream tributary areas must be included.						

Step 3: Determine Release Rates of Each Segment					
Segment 1	Segment 2	Segment 3	Segment 4	Segment 5	Segment 6
8.45	10.07	1.86	1.32	2.08	
	Segment 1	Segment 1 Segment 2	Segment 1 Segment 2 Segment 3	Segment 1 Segment 2 Segment 3 Segment 4	Segment 1 Segment 2 Segment 3 Segment 4 Segment 5

Critical Local Sewer Capacity (cfs/ac): 1.32

Step 4: Compare Outlet Sewer Capacity and Determine Release Rate

Name of Outlet Drainage Basin (as shown on the map):

Outlet Sewer Capacity (cfs/ac):

0.27

Maximum Allowable Release Rate (cfs/ac): 0.27

Name of Project: Camolite Radiation Site
Address: 434 East 26th Street
A/E of Record: Jim Wescott

1.0 Rate Control (Sheet 1 of 2)

Step	1:
------	----

Runoff Calculation		Proposed Area (sq. ft.)	C-Value 100- Year	Storage Volume (cu. ft.)
	Lawns - Sandy soil, flat, 0% to 2%	3,299	0.18	
	Lawns - Sandy soil, avg, 2% to 7%	0	0.27	
	Lawns - Sandy soil, steep, >7%	0	0.36	
	Lawns - Heavy soil, flat, 0% to 2%	0	0.30	
	Lawns - Heavy soil, avg, 2% to 7%	0	0.42	
Pervious Land	Lawns - Heavy soil, steep, >7%	0	0.47	
	Woodlands, flat, 2%	0	0.39	
	Native Vegetation with prepared soils	0	0.10	
	Dry bottom basins to HWL	10,494	0.75	
	Wetland	0	0.80	
	Green Roof	0	0.50	
	Gravel	47,449	0.70	
	Pavement	0	0.95	
	Roofs (conventional)	0	0.95	
Impervious Land	Building sidewalls connected by side gutters (enter 25% of the face of the sidewall) Wet bottom basins to HWL	0	0.95 1.00	
	BMPs providing storage that WILL COUNT toward detention storage (from Worksheet 1.2)	0	1.00	
BMP areas	BMPs providing volume control storage that WILL NOT BE COUNTED toward detention (from Worksheet 1.2)	0	Storage Provided will be used to factor the adjusted C-value in Cell D38	0

	Total pervious area (sq ft)	13,793	Ī
	Total impervious area (sq ft)	47,449	
	Total BMP area (sq ft)	0	
Summary	Total site area (sq ft)	61.242	
	Weighted C- value (non BMP areas)	0.68	
	Adjusted C-value (accounts for		
	BMPs)	0.00	
•	Notes:	Make note of any adjustments made for purposes of detention calcs here (such as removal of roof area that will discharge dire to Waters)	

	able Release		Type Yes or	
Step 2: Rate A	Assessment		No for all	
		1	that apply	Notes
	Question 1:	Does the site drain directly to Waters?	No	
		Does the site only include residential land use for	No	
		detached single-family and two-family dwellings?	No	
		detaction only of the two turning of the tings.		
	Question 3:	Is the Regulated Development a Lot to Lot Buillding (85%	No	
		or more of site footprint is occupied by buildings)?		
				Complete Tab 0.0 Release Rate to calculate the allowable
		Do you plan to use the standard maximum release rate (only available to sites less than 1.75 acres)?	No	release rate for the site unless a 1 cfs/ac release rate to waters
		(only available to sites less than 1.75 acres)?		will be used
		Is the site more than 75 percent of substantially contiguous at-grade open space that is conducive to	Yes	Detention Release Rate must be 0.75 cfs per acre or less unless total release rate is limited to minimum practical rate (0.15 cfs)
		ponding of surface waters (Answer "No" if site discharges		total release rate is littlifed to minimum practical rate (c. 15 cis)
		to waterway or is a service station)?		
		Does the development involve flow diversions (existing	No	
		sewer connection to be relocated to a different main) or	NO	
		multiple sewer connections (only available to sites over		
		1.75 acres)?		
	Question 7:	Are there widespread contaminated soils on the site, high	Yes	Oversized detention is allowed to meet volume control
		ground water table, or is this development classified as a		requirements. After completing this worksheet, fill out Tab 2.1.9
		lot-to-lot building?		to design oversized detention.

City of Chicago Department of Water Mangement

Name of Project: Camotite Radiation Site
Address: 434 East 26th Street
A/E of Record: Jim Wescott

1.0 Rate Control (Sheet 2 of 2)

Step 3: Achieving Rate Control Measures

Unadjusted Detention Release Rate =	0.380	cfs	0.0. To override, enter value in the cell to the right ->	0.380
Dry Weather Flow Rate = (From dry weather flow worksheet)	0.000	cfs	Waiting for Dry Weather Flo to be completed	w worksheet
Infiltration Facility Release Rate (to be added to eligible release rate when computing required storage)	0.000	cfs	No BMPs with infiltration bed BMP Summary Worksheet of infiltration rate is less than 0	r soil's
Release rate for detention storage computations:	0.380	cfs		
Required Storage Volume =	12,664	cubic feet		

Detention Storage Calculations (Based on Bulletin 70 Rainfall Data) STORM EVENT (5,10,25,50 or 100)

		100		Allowable rele	ase rate	0.380	cfs	
Storm	Runoff	Rainfall	Drainage	Inflow	Total	Release	Storage	Storage
Duration	Coefficient	Intensity	Area A	Rate	Storm Vol	Rate Qo	Rate (Qi-Qo)	Volume Rate
(minute)	C	(in/hr)	(acres)	Q=CIA	(cf)	(cfs)	(cfs)	(Qi-Qo)*t*60 (cf
5	0.68	10.920	1.41	10.45	3,135	0.380	10.07	3,021
10	0.68	10.020	1.41	9.59	5.752	0.380	9.21	5.525
15	0.68	8.200	1.41	7.85	7,061	0.380	7.47	6,720
30	0.68	5.600	1.41	5.36	9.645	0.380	4.98	8.961
60	0.68	3.560	1.41	3.41	12,262	0.380	3.03	10,896
120	0.68	2.235	1.41	2.14	15,397	0.380	1.76	12,664
180	0.68	1.617	1.41	1.55	16,706	0.380	1.17	12,606
360	0.68	0.947	1.41	0.91	19,565	0.380	0.53	11,365
720	0.68	0.549	1.41	0.53	22,699	0.380	0.15	6,301
1080	0.68	0.387	1.41	0.37	24,008	0.380	-0.01	-590
1440	0.68	0.316	1.41	0.30	26.109	0.380	-0.08	-6.688
2880	0.68	0.170	1.41	0.16	28,107	0.380	-0.22	-37,488
4320	0.68	0.122	1.41	0.12	30.243	0.380	-0.26	-68.149
7200	0.68	0.083	1.41	0.08	34,307	0.380	-0.30	-129,679
14400	0.68	0.046	1.41	0.04	38,372	0.380	-0.34	-289,602
							Required	
							Detention	
							Volume (cf)	12.664

Note: 1) the calculation assumes that the rising and recessing limb of inflow and outflow hydrograph are vertical

 Name of Project:
 Carnotite Radiation Site

 Address:
 434 East 26th Street

 A/E of Record:
 Jim Wescott

2.0 Volume Control

Step 1:	Runoff Calculation		Existing Area (sq ft)	Proposed Area (sq ft)
		Bare Earth	0	0
	Pervious Surface or Land Cover not Counted	Lawn or Landscaped Areas	11,900	3,299
	as Impervious for Volume Control Calculations	Woodlands	0	0
		Wetland	0	0
		Gravel	0	47,449
		Pavement	49,342	0
	Impervious Land	Roofs (conventional)	0	
		Water (including Wet Bottom Basin to		
		HWL)	0	
		Green Roof	-	
		Permeable Pavement	-	
		Bioinfiltration	-	
	BMPs	Swales	-	
	DIVIFS	Stormwater Trees	-	
		Roof Runoff Planters	-	
		Filter Strips	-	
		Dry Bottom Basins to HWL	-	10,494
		Total pervious area (sq ft)	11,900	3,299
		Total impervious area (sq ft)	49,342	47,449
		Total BMP areas treated as impervious		
	Summary	area (sq ft)	-	0
	Summary	Total BMP areas treated as pervious area		
		(sq ft)	-	10,494
		Total site area (sq ft)	61,242	61,242
		Imperviousness percentage (%)	80.6	77.5

Step 2: <u>Volume Control Assessment</u>

	Type Yes or No for all that apply	Note
Question 1: Does the site drain directly to Waters?	No	
Question 2: Are infiltration BMPs allowable? (See Chapter III Sections 4.1.2 of the Regulations.)		Infiltration BMPs are not allowed. Achieve volume control requirement through 15% impervious area reduction or by Oversized Detention.
Question 3: Do you wish to use permeable pavement only as a pervious surface to achieve impervious surface reduction goal?		Areas of permeable pavement are included as an impervious surface for the computation made in Cell C48. Storage will be counted toward volume control goal.

Step 3: Achieving Volume Control Measures

Achieve I. or II. below in accordance with the Ordinance.

	Capture 0.5" of runoff from impervious surfaces. Storage required =	1,977		Summary if electing volume control storage option
or, II.	Reduce proposed imperviousness to:	65.6	percent	Imperviousness reduction not met

 Name of Project:
 Carnotite Radiation Site

 Address:
 434 East 26th Street

 A/E of Record:
 Jim Wescott

2.1.9 Oversized Detention Worksheet

You must first complete Tab 1.0 Rate Control and Tab 2.0 Volume Control worksheets to use this Oversized Detention Worksheet.

Step 1:	Oversized Detention Co	omputation		Units	Notes
	Co	aximum Release Rate for Rate ontrol (automatically entered from II G73 on Rate Control worksheet)	0.38	cfs	
	Ra	ate Control Storage	12,664	cubic feet	
	Vo	lume Control Storage	1,977	cubic feet	
		tal Storage Required for oversized tention	14,641	cubic feet	
	tot	y reducing release rate here until al required storage in oversized	0.22	afa	

Oversized Detention Calculation

STORM EVENT (5,10,25,50 or 100) =

	cfs	0.22			100			
Storage Volume Rate	Storage Rate (Qi-Qo)	Release Rate Qo	Total Storm Vol	Infow Rate	Drainage Area A	Rainfall Intensity	Runoff Coefficient	Storm Duration
(Qi-Qo)*t*60 ((Cfs)	(cfs)	(cf)	Q=CIA	(acres)	(in/hr)	C	(minute)
3,069	10.23	0.22	3,135	10.45	1.41	10.920	0.68	5
5,620	9.37	0.22	5,752	9.59	1.41	10.020	0.68	10
6,863	7.63	0.22	7,061	7.85	1.41	8.200	0.68	15
9,249	5.14	0.22	9,645	5.36	1.41	5.600	0.68	30
11,470	3.19	0.22	12,262	3.41	1.41	3.560	0.68	60
13,813	1.92	0.22	15,397	2.14	1.41	2.235	0.68	120
14,330	1.33	0.22	16,706	1.55	1.41	1.617	0.68	180
14,813	0.69	0.22	19,565	0.91	1.41	0.947	0.68	360
13,195	0.31	0.22	22,699	0.53	1.41	0.549	0.68	720
9,752	0.15	0.22	24,008	0.37	1.41	0.387	0.68	1080
7,101	0.08	0.22	26,109	0.30	1.41	0.316	0.68	1440
-9,909	-0.06	0.22	28,107	0.16	1.41	0.170	0.68	2880
-26,781	-0.10	0.22	30,243	0.12	1.41	0.122	0.68	4320
-60,733	-0.14	0.22	34,307	0.08	1.41	0.083	0.68	7200
-151,708	-0.18	0.22	38,372	0.04	1.41	0.046	0.68	14400
0	-0.22	0.22	0	0.00	1.41	0.000	0.68	
0	-0.22	0.22	0	0.00	1.41	0.000	0.68	
0	-0.22	0.22	0	0.00	1.41	0.000	0.68	
0	-0.22	0.22	0	0.00	1.41	0.000	0.68	
0	-0.22	0.22	0	0.00	1.41	0.000	0.68	
0	-0.22	0.22	0	0.00	1.41	0.000	0.68	
0	-0.22	0.22	0	0.00	1.41	0.000	0.68	
0	-0.22	0.22	0	0.00	1.41	0.000	0.68	
0	-0.22	0.22	0	0.00	1.41	0.000	0.68	
0	-0.22	0.22	0	0.00	1.41	0.000	0.68	

Note: 1) the calculation assumes that the rising and recessing limb of inflow and outflow hydrograph are vertical

Name of Project: Carnotite Radiation Site
Address: 434 East 26th Street Jim Wescott A/E of Record:

(FOR COMPUTATIONS AND REFERENCE)

City of Chicago Intensity-Duration-Frequency (IDF) Curve

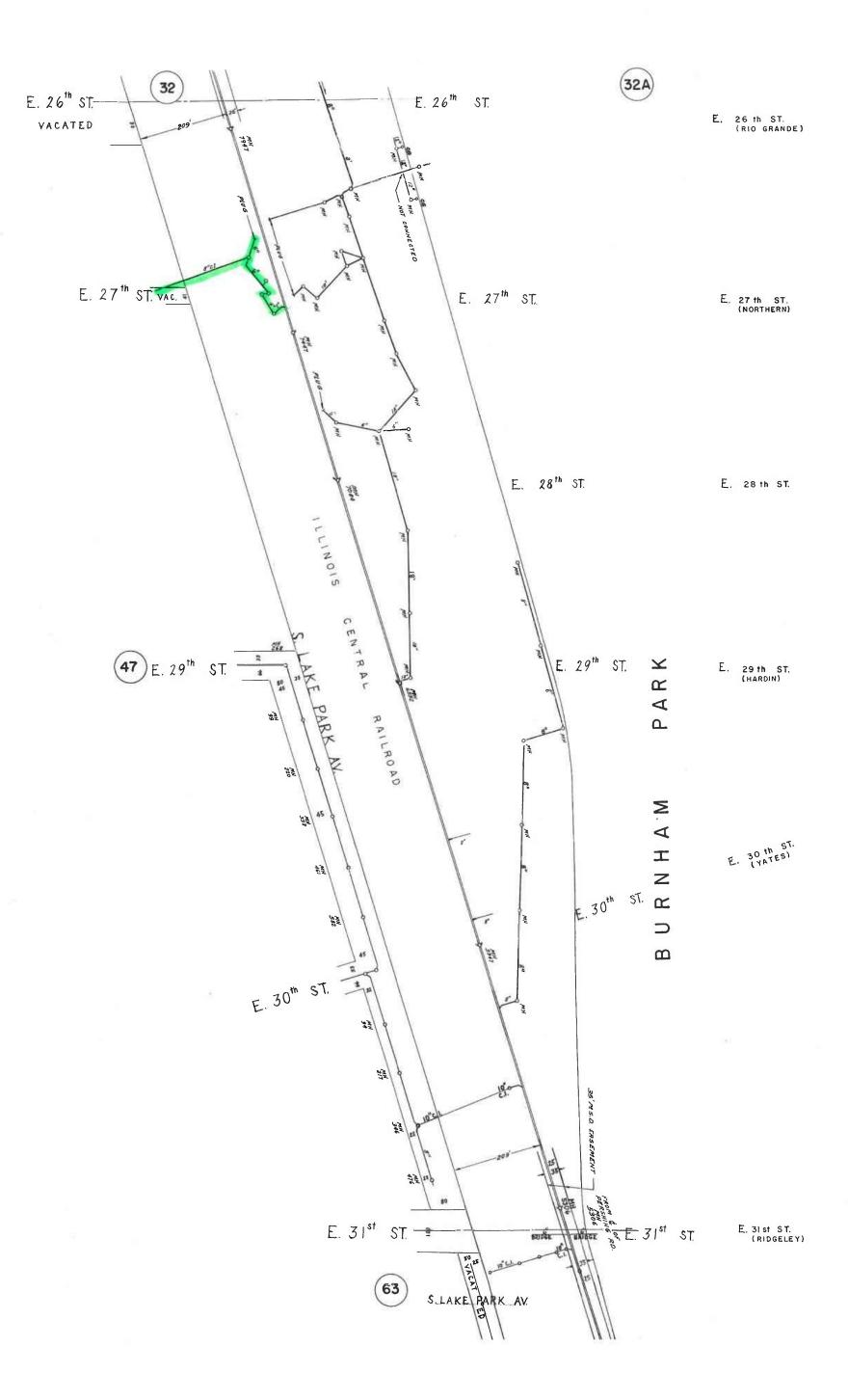
(Based on Bulletin 70 Rainfall Data)

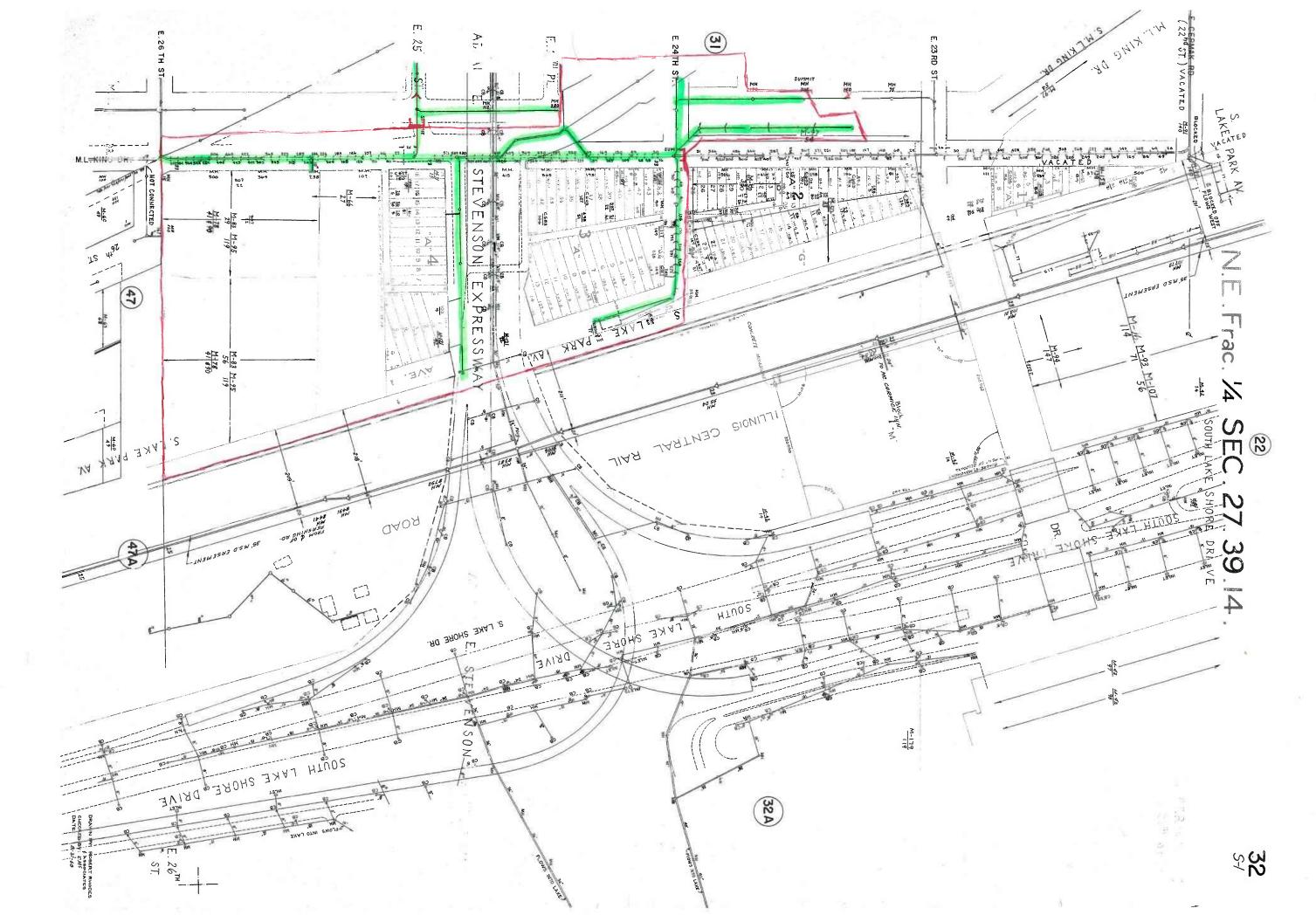
					Storm Eve	ent in Years	;			
	5-Y	'ear	10-	Year	25-`	Year	50-	Y ear	100-	Year
Storm		Average		Average		Average		Average		Average
Duration	Rainfall	Intensity	Rainfall	Intensity	Rainfall	Intensity	Rainfall	Intensity	Rainfall	Intensity
(min)	(in)	(in/hr)	(in)	(in/hr)	(in)	(in/hr)	(in)	(in/hr)	(in)	(in/hr)
5	0.46	5.520	0.54	6.480	0.66	7.920	0.78	9.360	0.91	10.920
10	0.84	5.040	0.98	5.880	1.21	7.260	1.42	8.520	1.67	10.020
15	1.03	4.120	1.21	4.840	1.49	5.960	1.75	7.000	2.05	8.200
30	1.41	2.820	1.65	3.300	2.04	4.080	2.39	4.780	2.80	5.600
60	1.79	1.790	2.10	2.100	2.59	2.590	3.04	3.040	3.56	3.560
120	2.24	1.120	2.64	1.320	3.25	1.625	3.82	1.910	4.47	2.235
180	2.43	0.810	2.86	0.953	3.53	1.177	4.14	1.380	4.85	1.617
360	2.85	0.475	3.35	0.558	4.13	0.688	4.85	0.808	5.68	0.947
720	3.31	0.276	3.89	0.324	4.79	0.399	5.62	0.468	6.59	0.549
1080	3.50	0.194	4.11	0.228	5.06	0.281	5.95	0.331	6.97	0.387
1440	3.80	0.158	4.47	0.186	5.51	0.230	6.46	0.269	7.58	0.316
2880	4.09	0.085	4.81	0.100	5.88	0.123	6.84	0.143	8.16	0.170
4320	4.44	0.062	5.18	0.072	6.32	0.088	7.41	0.103	8.78	0.122
7200	4.91	0.041	5.70	0.048	6.93	0.058	8.04	0.067	9.96	0.083
14400	6.04	0.025	6.89	0.029	8.18	0.034	9.38	0.039	11.14	0.046

$\underline{\textbf{ATTACHMENT 6}} - \underline{\textbf{WATERSHED LIMITS}}$









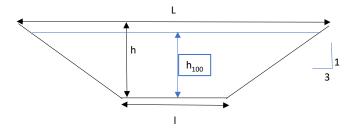


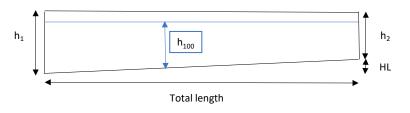
 $\underline{\textbf{ATTACHMENT 7} - \textbf{DETENTION BASIN VOLUME CALCULATION}}$



	TOTAL BASIN VOLUME		
	DETENTION BASIN SECTION L	25 ft	
	1	13 ft	
	h_1	2 ft	
		(- T)	
Time to empty basin	SECTION AREA	$A = \frac{(L+l)}{l}$)*h
0.22 cfs		2	
3472.425 sec	A_1	38 ft ²	2
0.04019 day			
	TOTAL VOLUME		
	Total length	450 ft	
	V = A*Total length	17100 ft ³	3
,	VOLUME LOSS DUE TO BOTTOM SLOPE		
	Bottom slope	0.1 %	
	HL = Bottom slope * Total length	0.45 ft	
	Volume loss = (HL*I*Total length)/2	1316.25 ft ³	3
	h ₂	1.55 ft	
	A_2	29.45 ft ²	2
TOTAL BASIN	VOLUME = Total Volume - Volume Loss	15784 ft ³	3
	TOTAL VOLUME NEEDED	14813 ft ³	3

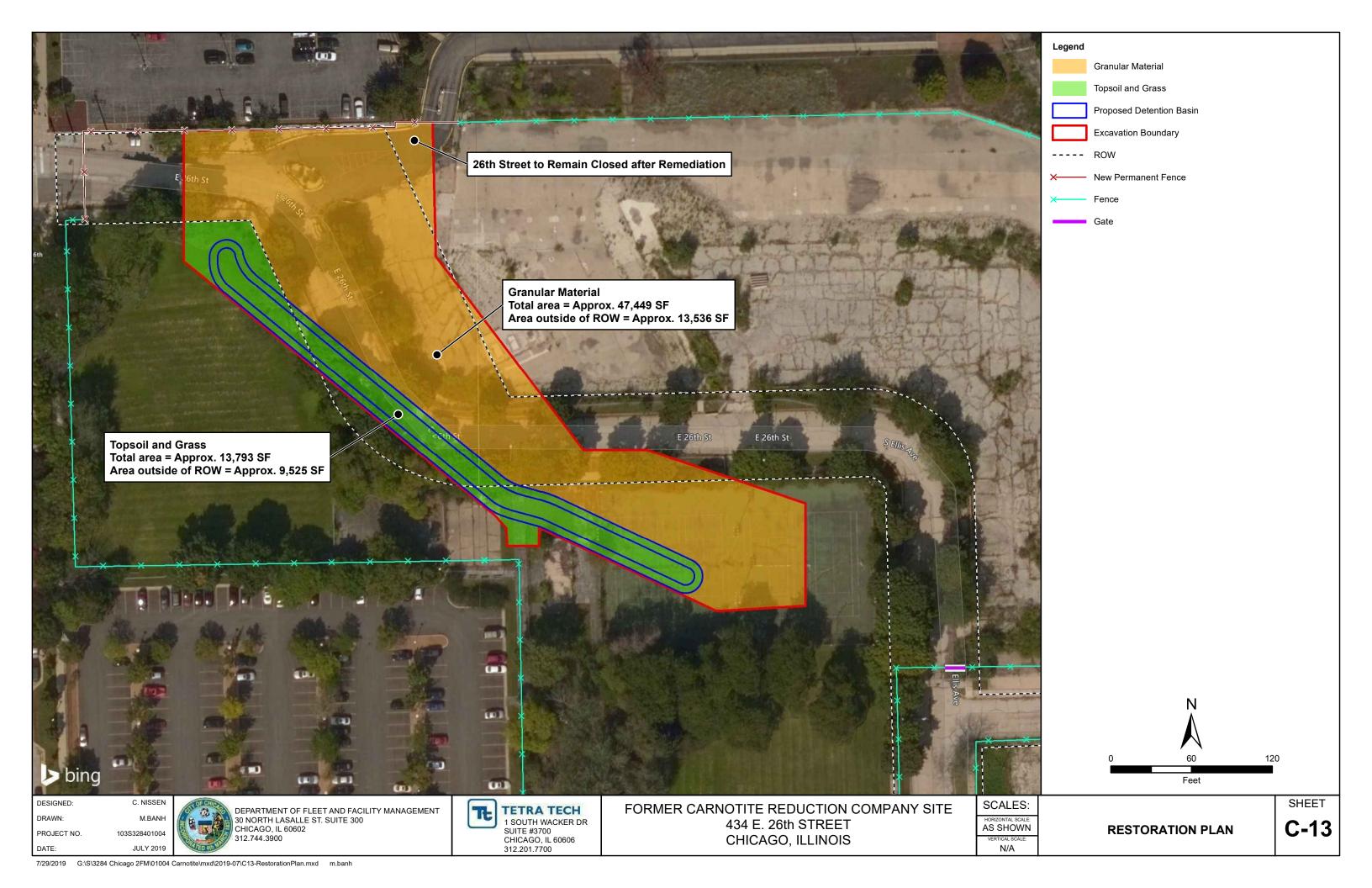
	RATE CONTROL VOLUME (100YR)
23.32 ft	DETENTION BASIN SECTION L
13 ft	1
1.72 ft	h ₁₀₀
$A = \frac{(L+l)*h}{2}$	SECTION AREA
31.24 ft ²	А
	TOTAL VOLUME
450 ft	Total length
14055.84 ft ³	V = A*Total length
	VOLUME LOSS DUE TO BOTTOM SLOPE
0.1 %	Bottom slope
0.45 ft	HL = Bottom slope * Total length
1316.25 ft ³	Volume loss = (HL*I*Total length)/2
12740 ft ³	TOTAL BASIN VOLUME
12664 ft ³	VOLUME NEEDED (RATE CONTROL)





<u>ATTACEMENT 8 – PERVIOUS AND IMPERVIOUS PROPOSED AREAS</u> (PLAN SHEET C-13)





<u>ATTACHMENT 9 – GEOTECHNICAL REPORT</u>



Geotechnical Investigation Report

Sheet Pile Wall Design 434 East 26th Street Chicago, Illinois

Prepared for:

Department of Fleet and Facility Management (2FM) 30 N. LaSalle Street, Suite 300 Chicago, Illinois 60602

Project Design Engineer:
Tetra Tech
1 South Wacker Drive, Suite 3700
Chicago, IL 60606

Prepared by:



623 Cooper Court • Schaumburg, IL 60173 Tel: 630.994.2600 • Fax: 312.733.5612 www.gsg-consultants.com

January 25, 2019

January 25, 2019

Ms. Carol Nissen, PE, PG **Environmental Engineer** Tetra Tech 1 South Wacker Drive, Suite 3700 Chicago, IL 60606

Proposed Temporary Sheet Piling Improvements 434 East 26th Street Chicago, Illinois

Dear Ms. Davis:

Attached is a copy of the Geotechnical Report for the above referenced project. The report provides a brief description of the site investigation, site conditions and construction recommendations. The site investigation included advancing three (3) soil borings to depths of 61.0 and 70.0 feet.

Should you have any questions or require additional information, please call us at 312-733-6262.

Sincerely,

Alex Barlan, P.E. **Project Engineer** Ala E Sassila, Ph.D., P.E.

Blusarna

Principal



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Appendix C Lab Results

Geotechnical Report Proposed Temporary Sheet Piling Improvements 434 East 26th Street Chicago, Illinois

1.0 INTRODUCTION

GSG Consultants, Inc. (GSG) completed a geotechnical investigation for the installation of temporary sheet piling for excavation activities at the property located 434 East 26th Street in Chicago, Illinois. The purpose of the investigation was to explore the subsurface conditions, to determine engineering properties of the subsurface soil, and develop design and construction recommendations for installation of the temporary sheet piling for the project.

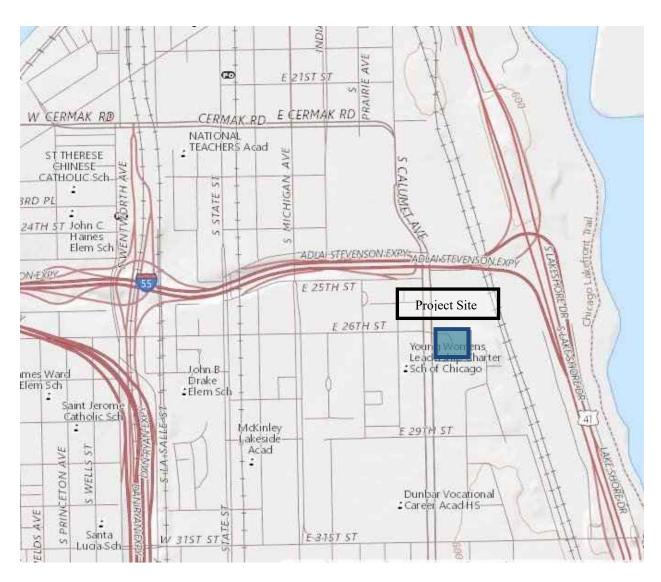


Figure 1: Project Location Map – USGS National Map



1.1 Site Conditions

The project site is currently a vacant parcel of land with a paved roadway at the north, abandoned parking and demolished building lot to the east end and a grass field to the southwest. The ground surface at the property of the proposed project is relatively flat at about an elevation of 597 to 599 feet .

1.2 Proposed Project Information

Based on the preliminary design information provided by Tetra Tech (client), temporary sheet pile retaining walls will be required for excavation of the soils from the site. It is anticipated that these walls would have exposed heights of approximately 15 feet to help facilitate the removal of unsuitable soils. Plans have not been provided for the approximate dimensions of the sheet piling, but the following Table 1 summarizes the assumed/approximate sheet pile wall information.

Table 1 – Wall information Summary

Wall Location	Approximate Length (ft)	Maximum Exposed Wall Height (ft)	Back Slope / Front Slope
North side	165	15	Level
South Side	165	15	Level
West Side	100	15	Level
East Side	100	15	Level



2.0 SITE SUBSURFACE EXPLORATION PROGRAM

This section describes the subsurface exploration program and laboratory testing program completed as part of this project.

2.1 Subsurface Exploration Program

The proposed locations and depths of the soil borings were proposed by the Tetra Tech and were completed based on field conditions and accessibility. The subsurface exploration was conducted between December 3rd thru 7th, 2018 and included advancing three (3) standard penetration test (SPT) borings near the locations of the temporary proposed walls. The borings were drilled to depths of 61.0 and 70.0 feet below existing grade. The locations of the soil borings are shown on the **Appendix A** - **Boring Location Map**. **Table 2** below presents a list of the borings completed for the new retaining walls.

Table 2 – Summary of Subsurface Exploration Borings

Location	Soil Boring	Depth (ft)	Existing Ground Elevation in feet (in CCD)
Northwest area on 26 th Street	B-1	61.0	597 (17.1 CCD)
Southwest area in field	B-2	61.5	598 (18.1 CCD)
East area in parking lot	B-3	70.0	599 (19.1 CCD)

The soil borings were drilled using a Mobile B-57 truck mounted drill rig using 3½-inch I.D. hollow stem augers for B-1 and B-3 and mud-rotary techniques for B-2 below a depth of 40 feet. Soil sampling was performed according to AASHTO T 206, "Penetration Test and Split Barrel Sampling of Soils." Soil samples were obtained at 2.5-foot intervals to a depth of 35 feet, and 5-foot intervals beyond that. Water level measurements were made in each boring when evidence of free groundwater was detected on the drill rods or in the samples. The boreholes were also checked for free water immediately after auger removal, and before filling the open boreholes with soil cuttings.



GSG's field representative inspected, visually classified and logged the soil samples during the subsurface exploration activities and performed unconfined compressive strength tests on cohesive soil samples using a calibrated Rimac compression tester and a calibrated hand penetrometer in accordance with IDOT procedures and requirements. Representative soil samples were collected from each sample interval and were placed in jars and returned to the laboratory for further testing and evaluation.

2.2 Laboratory Testing Program

All samples were inspected in the laboratory to verify the field classifications. A laboratory testing program was undertaken to characterize and determine engineering properties of the subsurface soils encountered in the area of the proposed retaining walls.

The following laboratory tests were performed on representative soil samples:

- Moisture content ASTM D2216
- Atterberg Limits ASTM D 4318
- Dry Unit Weight ASTM D7263

Based on the laboratory test results, the soils encountered were classified according to the Unified Soil Classification System (USCS). The results of the laboratory testing program are shown along with the field test results in **Appendix B, Soil Boring Logs and in Appendix C – Lab Results.**

2.3 Subsurface Conditions

This section provides a brief description of the soils encountered in the borings performed. Variations in the general subsurface soil profile were noted during the drilling activities. Detailed descriptions of the subsurface soils are provided in the soil boring logs.

The soil boring logs provide specific conditions encountered at each boring location. The soil boring logs include soil descriptions, stratifications, penetration resistance, elevations, location of the samples, and laboratory test data. Unless otherwise noted, soil descriptions indicated on boring logs are visual identifications. The stratifications shown on the boring logs represent the conditions only at the actual boring locations and represent the approximate boundary between subsurface materials; however, the actual transition may be gradual.



2.3.1 Soil Conditions

Boring B-1 noted 4 inches of asphalt and 10 inches of base course gravel at the surface (approximately 597 feet MSL) underlain by sand with gravel fill soils to a depth of 6 feet below grade. Beneath the fill, loose to medium dense sand was noted to a depth of 31 feet below grade. Following this depth, very soft to stiff fat clay was encountered to a depth of 61 feet. The fill granular soils had an SPT blow counts between 4 and 10 and the native granular soils had SPT blow counts between 4 and 15 blows. The native high plasticity fat clay soils had unconfined compressive strengths ranging from 0.2 tsf to 1.3 tsf.

Boring B-2 noted 6 inches of topsoil at the surface (approximately 598 feet MSL) underlain by sand with gravel fill soils to a depth of 6.5 feet below grade. Beneath the fill, loose to medium dense sand was noted to a depth of 29.5 feet followed by a very loose to loose silty sand to a depth of 34 feet below grade. Following this depth, a medium stiff clay was encountered to a depth of 48.5 feet followed by a stiff to very stiff silty clay to a depth of 61.5 feet. The fill granular soils had an SPT blow counts between 4 and 24 and the native granular soils had SPT blow counts between 5 and 14 blows. The native clay soils had unconfined compressive strengths ranging from 0.5 tsf to 3.75 tsf.

Boring B-3 noted sand with gravel fill soils to a depth of 3.5 feet below grade (approximately 599 feet MSL or 9.10 CCD). Beneath the fill, loose to medium dense sand was noted to a depth of 26 feet followed by a loose silty sand to a depth of 31 feet below grade. Following this depth, a soft to medium stiff clay was encountered to a depth of 39 feet followed by a soft to stiff silty clay to a depth of 63.5 feet. The boring then noted a dense to very dense silt, trace clay and limestone fragments to a depth of 70 feet. The fill granular soils had an SPT blow counts of 13 and the native granular soils had SPT blow counts between 4 and 14 blows. The native clay soils had unconfined compressive strengths ranging from 0.5 tsf to 1.0 tsf.

Boring B-1 and B2 were terminated at 61 and 61.5 feet upon encountering auger refusal on very hard silty clay soils, while boring B-3 was terminated at a deeper depth of 71 feet.

2.3.2 Groundwater Conditions

Water levels were checked in each boring to determine the general groundwater conditions present at the site and were measured while drilling and after each boring was completed.



Groundwater was encountered in boring B-1, B-2, and B-3 while drilling at about 18.5 feet, 8.5 feet and 13.5 feet below grade (approximate elevations of 578.5, 589.5, and 585.5 feet MSL respectively).

Based on the color change from brown to gray and the water surface elevation in the adjacent Lake Michigan, it is anticipated that the long-term groundwater level is near elevation 579 to 580 feet MSL. However, water levels at the time of drilling varied greatly from 578.5 to 589.5 feet. Water level readings were made in the boreholes at times and under conditions shown on the boring logs and stated in the text of this report. However, it should be noted that fluctuations in groundwater level may occur due to variations in rainfall, other climatic conditions, Lake Michigan water levels or other factors not evident at the time measurements were made and reported herein.



3.0 GEOTECHNICAL ANALYSES

This section provides GSG's geotechnical analysis and recommendations for the design of the proposed retaining walls and ramp based on the results of the initial field exploration, laboratory testing, and geotechnical analysis. Subsurface conditions in unexplored locations may vary from those encountered at the boring locations.

3.1 Derivation of Soil Parameters for Design

GSG determined the geotechnical parameters to be used for the project design based on the results of field and laboratory test data on individual boring logs as well as our experience. Unit weights, friction angles and shear strength parameters were estimated using standard penetration test (SPT) results for the fill and cohesion less soils and in-situ and laboratory test results for cohesive soils. The SPT values were corrected for hammer efficiency and overburden weight. The hammer efficiency correction factor considers the use of a safety hammer/rope/cat-head system, generally estimated to be 60% efficient. Thus, correlations should be based upon what is currently termed as N₆₀ data. GSG used a truck mounted Mobile B-57 drill rig for completing the field subsurface exploration at this site. The efficiency of the automatic hammers for the drill rig was measured to be approximately 98%, based on GSG's most recent calibrations records. The correction for hammer efficiency is a direct ratio of relative efficiencies. The following equations should be used in calculating the corrected blow counts for the purposes of design and analysis:

$$N_{60} = N_{Field} * (98/60)$$
 for Mobile B-57 drill rig

Where the N Field value is the field recorded blow counts during drilling activities.

Table 1 presents generalized soil parameters to be used for design based on the laboratory and in-situ testing data:



Table 3 – Summary of On-site Soil Parameters

	Table 3 – Summary of On-site Soil Parameters										
Depth	Co.ii	In situ Unit	Undr	ained	Drained						
(Elevation, feet)	Soil Description	Weight γ (pcf)	Cohesion c (psf)	Friction Angle ф (Degrees)	Cohesion c (psf)	Friction Angle φ (Degrees)					
0.0 – 6.0 (598.0 – 592.0)	FILL: Black and Brown Sands and Gravels	125	0	30	0	30					
6.0 – 19.0 (592.0 – 579.0)	Brown and Gray Loose to Medium Dense Sand	122	0	34	0	34					
19.0 – 33.0 (579.0 – 565.0)	Gray Very Loose to Medium Dense Silty Sand	119	0	32	0	32					
33.0 – 48.0 (565.0 – 550.0)	Gray Very Soft to Medium Stiff Fat Clay	80	460	0	0	21					
48.0 – 61.0 (550.0 – 537.0)	Gray Medium Stiff to Very Stiff Fat Clay	80	1,530	0	75	24					
61.0 – 70.0 (537.0 – 528.0)	Gray Dense to Extremely Dense Silt	140	0	40	0	40					



4.0 GEOTECHNICAL RECOMMENDATIONS

This section provides recommendation regarding design parameters for the proposed sheet pile design. The recommendations were developed based on the project information provided by the Tetra Tech and the results of the site investigation. If there are any significant changes to the project characteristics or if significantly different subsurface conditions are encountered during construction, GSG should be consulted so that the recommendations of this report can be reviewed. GSG understands that the proposed temporary earth retention System will be sheet pile type. Below is a general discussion of the wall design requirements and required design parameters.

4.1 Sheet Pile Wall Design

Sheet pile walls are typically used in cut areas when continuous support must be provided to maintain existing structures or other adjacent facilities. To provide lateral resistance against the retained soil, the walls can be designed to act as a cantilever or can use tie backs behind the wall. The installation of sheet pile walls requires the use of specialty equipment to drive the piles into the ground. The walls maintain the existing site conditions with minimal disturbance to existing structures and can be installed relatively quickly. However, due to the presence of very stiff clays and extremely dense silts near an elevation of 530.0 feet (-49.9 feet CCD), we recommend using a heavier pile section with a minimum thickness of 0.4 inch to alleviate any damage to the pile section during driving if the sheet pile design is to extend past a depth of 55 feet below the existing surface. Grade 50 steel should be used for the sheet pile. The interlocks could be partially clogged during driving and after installation due to fine soil particle migration. The steel sheet piles may be subject to potentially corrosion. Corrosion rates are typically a function of temperature, soil pH, access to oxygen, and chemistry of the environment surrounding the pile. The walls are intended to be temporary, but if the wall is to remain in place as a long-term wall, corrosion deterioration should be considered on the sheet pile wall design.

Based on the overall wall stability, it is recommended that the cantilevered sheet pile system be installed to a minimum elevation of 550 feet (-29.9 feet CCD) with the pile section being at least 50 feet in length. GSG does not anticipate any constructability issues while driving these sheet piles, however, if an alternate system is considered then an anchored wall system may be considered. Different anchor systems such as grouted tiebacks, deadman anchor, or waler beams may be considered. The anchor system will transmit all loads from the soil through the retaining walls to the anchor and will align and brace the walls in position.



4.2 Lateral Earth Pressures and Loading

The wall shall be designed to withstand earth and live lateral earth pressures. The lateral earth pressures on retaining walls depend on the type of wall (i.e. restrained or unrestrained), the type of backfill and the method of placement against the wall, and the magnitude of surcharge weight on the ground surface adjacent to the wall. Sheet pile walls are considered flexible and such the earth loads may be calculated using active earth pressure for load above the design grade, and both active and passive earth pressures below the design grade. The active earth pressure coefficient (K_a), and the passive earth pressure coefficient (K_p) were determined in accordance with AASHTO Section 3.11.5.3 and 3.11.5.4, respectively.

The design should include a structural evaluation of the sheet pile section to meet applied shear and moment, and an evaluation of overturning to determine embedment depth and other design requirements. The simplified earth pressure distributions shown in the AASHTO Standard Specifications for Highway Bridges could be used for the wall design. **Table 4** also provides recommended lateral soil modulus and soil strain parameters that can be used for laterally loaded pile analysis via the p-y curve method based on the encountered subsurface conditions. The passive resistance in front of the sheet pile wall should be ignored for the upper 3.5 feet due to excavation activities and frost-heave condition.



Table 4 - Geotechnical Lateral Design Parameters

Depth Elevation (feet)	Soil Description	Active Earth Pressure Coefficient (K _a)	Passive Earth Pressure Coefficient (K _P)	At Rest Earth Pressure Coefficient (K _o)	Lateral Modulus of Subgrade Reaction (pci)	Soil Strain (E ₅₀)	Adhesion (C _a) psf	Friction Angle between Steel and Soils
		(Na)	(Np)	(10)	(pci)			(Tan δ)
0.0 – 6.0 (598.0 – 592.0)	FILL: Black and Brown Sands and Gravels	0.33	3.00	0.50	90	NA	NA	17 (0.30)
6.0 – 19.0 (592.0 – 579.0)	Brown and Gray Loose to Medium Dense Sand	0.28	3.53	0.44	25	NA	NA	17 (0.30)
19.0 – 33.0 (579.0 – 565.0)	Gray Very Loose to Medium Dense Silty Sand	0.31	3.25	0.47	20	NA	NA	14 (0.25)
33.0 – 48.0 (565.0 – 550.0)	Gray Very Soft to Medium Stiff Fat Clay	0.47	2.12	0.64	30	0.020	460	NA
48.0 – 61.0 (550.0 – 537.0)	Gray Medium Stiff to Very Stiff Fat Clay	0.42	2.37	0.59	760	0.007	850	NA
61.0 – 70.0 (537.0 – 528.0)	Gray Dense to Extremely Dense Silt	0.22	4.59	0.36	125	NA	NA	11 (0.20)

Traffic and other surcharge loads should be included in the retaining wall design. A live load surcharge of 250 psf (or the equivalent weight of 2 feet soil overburden) should be applied where vehicular load is expected to act on the surface of the backfill. Heavy equipment should not be allowed closer than five (5) feet to the retaining wall to prevent inducing high lateral



earth pressures and causing wall yielding and/or other damage.

4.3 Excavation Base Stability

In open-cuts, it is necessary to consider the possibility of the base of the excavation failure by heaving, due to the removal of the weight of excavated soil. Heaving typically occurs in very soft or soft fat clays, as encountered in the borings, when the excavation depth is sufficiently deep enough to cause the surrounding soil to displace vertically due to a failure of the soil beneath the excavation bottom, with a corresponding upward movement of the soils in the bottom of the excavation. In fat and lean clays, heave normally does not occur unless the ratio of Critical Height to Depth of Cut approaches one. The sheet pile wall designer should check to make sure the sheet pile is sufficiently embedded in the stiffer clay soils to avoid heaving.



5.0 CONSTRUCTION CONSIDERATIONS

This section provides general construction consideration during construction activities at the site. Site specific information should be utilized based on site survey condition and construction phasing of the project.

5.1 Existing Utilities

Before proceeding with construction, any existing underground utility lines that will interfere with construction should be completely rerouted or removed from beneath the proposed construction areas. Existing utility lines that are to be abandoned in place should be removed and/or plugged with a minimum of 2 feet of cement grout. All excavations resulting from underground utilities removal activities should be cleaned of loose and disturbed materials, including all previously-placed backfill, and backfilled with suitable fill materials in accordance with the requirements of this section. During the clearing and stripping operations, positive surface drainage should be maintained to prevent the accumulation of water.

5.2 General Excavations

The contractor will be responsible to provide a safe excavation during the construction activities of the project. All excavations should be conducted in accordance with applicable federal, state, and local safety regulations, including, but not limited to the Occupational Safety and Health administration (OSHA) excavation safety standards. Excavation stability and soil pressures on temporary shoring are dependent on soil conditions, depth of excavations, installation procedures, and the magnitude of any surcharge loads on the ground surface adjacent to the excavation. Excavation near existing structures and underground utilities should be performed with extreme care to avoid undermining existing structures. Excavations should not extend below the level of adjacent existing foundations or utilities unless underpinning or other support is installed. It is the responsibility of the contractor for field determinations of applicable conditions and providing adequate shoring for all excavation activities.

5.3 Groundwater Management

Based on the soil boring logs, the long-term water table is about 19 feet below the existing ground surface; however, groundwater was encountered as shallow as 8 feet during the drilling operations. It is anticipated that groundwater issues may be present at the site as the excavation is anticipated to be at a depth of 15 feet. GSG recommends that the sheet piles incorporate interlocking edges and extend thru the sandy soils into the clay soils, to act as a cutoff wall to prevent ground water from entering the site. Some water may still seep through



the interlocks of the steel sheeting, but this could be removed by normal sump pump operations. Even then well points may be required to dewater the excavation area and the contractor should provide a dewatering plan detailing how groundwater will be controlled and prevent water infiltration into the excavation/construction site.



6.0 LIMITATIONS

This report has been prepared for the exclusive use of 2FM and its structural consultant Tetra Tech. The recommendations provided in the report are specific to the project described herein and are based on the information obtained at the soil boring locations within the proposed retaining wall area. The analyses performed and the recommendations provided in this report are based on subsurface conditions determined at the location of the borings. This report may not reflect all variations that may occur between boring locations or at some other time, the nature and extent of which may not become evident until during the time of construction. If variations in subsurface conditions become evident after submission of this report, it will be necessary to evaluate their nature and review the recommendations presented herein.



APPENDIX A BORING LOCATION MAP



LEGEND : SOIL BORING LOCATION



623 Cooper Court • Schaumburg, IL 60173
Tel: 630.994.2600 • Fax: 312.733.5612

www.gsg-consultants.com integrity | Quality | Reliability SCALE: DRAWN BY:

NTS AB

CHECKED BY:

AES

DATE:

01/24/19

APPENDIX A : BORING LOCATION MAP

PROPOSED TEMPORARY SHEET PILING IMPROVEMENT

434 EAST 26th STREET CHICAGO, ILLINOIS

etra Jech Carnotite (septech Ceptech Carnotite) Appendix A - Carnotite- Roung Jocation Man dwg 9/12/2019 8:53:1/ AM iscott, ANS till bleed A (8.50 x 11.00 inche

APPENDIX B

BORING LOGS

GSG Consultants, Inc. 623 Cooper Court Schaumburg, Illinois 60173 Telephone: 630-694-2600

BORING NUMBER B-1

PAGE 1 OF 3

1	I)HCSTO	Fax: 312-733-5012				.				
1										
1			PROJECT LOCATION 434 East 26th Street GROUND ELEVATION 597.00 ft HOLE SIZE 3 1/4"							
1				-		п	OLE SIZE _3 1/4			
1		AETHOD HOA								
1		METHOD HSA								
1		Y _JC CHECKED BY				I/A				
NOTE	:5		AFTER DF	KILLING	N/A	1				
			出	>		₽S	▲ SPT N VALUE 20 40 60 80			
<u>F</u> (l β β		⊤ä	/ER	NA NTS	FINIT SSSI	20 40 60 80			
DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	1PLE	RECOVERY (%)	BLOW COUNTS (N VALUE)	NON (st)	Moisture Content10 20 30 40			
,	Ö		SAMPLE TYPE NUMBER	H	OS	UNCONFINED COMPRESSION (tsf)	☐ Unconfined Compression (tsf)			
0		4 inches of Asphalt					2 4 6 8			
	0 0	10 inches of Base Course Gravel								
2		FILL: SAND, with wood fragments - Black - Moist	√ ss	56	3-5-5					
-			1		(10)					
<u>-</u>							-			
<u> </u> 			√ ss	†	1-2-2					
5		FILL: SAND, fine grained - Brown - Moist		56	(4)		$ \uparrow\uparrow$			
		, 3								
-		SAND, fine grained (SP) - Brown and Gray - Loose to Med	dium /			1				
-		Dense - Moist to Wet	\	67	4-4-4 (8)					
) 			<u> </u>	+	,	-				
-										
				78	5-5-6					
10_			4		(11)					
-			√ ss	00	5-6-6					
 			5	89	(12)		7			
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20		SILTY SAND (SM) - Gray - Loose to Medium Dense - Wet		100	(8)					
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<u>`</u>			SS 9	100	(8)		.			
<u>-</u>			<u> </u>	+		1				
			\ //	+		-				
<u>-</u>			SS 10	89	3-5-6 (11)					
25			/\ 10		(11)		: :/ : :			

GSG Consultants, Inc. 623 Cooper Court Schaumburg, Illinois 60173 Telephone: 630-994-2600

BORING NUMBER B-1

PAGE 2 OF 3

Fax: 312-733-5612 CLIENT Tetra Tech PROJECT NAME 2FM Carnotite Site PROJECT NUMBER 18-2084 PROJECT LOCATION 434 East 26th Street UNCONFINED COMPRESSION (tsf) ▲ SPT N VALUE SAMPLE TYPE NUMBER RECOVERY (%) BLOW COUNTS (N VALUE) 20 40 60 Moisture Content0 20 30 40 MATERIAL DESCRIPTION 10

☐ Unconfined Compression (tsf) SILTY SAND (SM) - Gray - Loose to Medium Dense - Wet (continued) SS 8-7-6 89 11 (13)GEOTECH BH PLOTS - GINT STD US.GDT - 1/24/19 14:18 - \(\)GSGFS02\PROJECTS - ENGINEERING\TETRA TECHICARNOTITE GEOTECH\GEOTECH\GEOTECH\GEOTECH\GEOTECH\GEOTECHNICAL\FIELD TESTING\18:2084.GPJ SS 2-2-2 100 12 (4) 30 SS 1-1-2 89 13 (3) FAT CLAY (CH) - Gray - Very Soft to Medium Stiff - Moist to Very SS 0-0-2 0.25 67 14 (2) SS 0-1-1 78 0.20 15 (2) SS 0-1-2 100 0.50 16 (3) FAT CLAY, trace gravel (CH) - Gray - Medium Stiff to Stiff - Moist to Very Moist SS 2-2-2 100 1.30 70.1 Ф (4)

BORING NUMBER B-1

PAGE 3 OF 3

CLIENT Tetra Tech PROJECT NAME 2FM Carnotite Site

PROJECT NUMBER 18-2084 PROJECT LOCATION 434 East 26th Street

	U		YPE R	RY	V TS JE)	INED	2	▲ SPT	N VALU	JE 80
DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY (%)	BLOW COUNTS (N VALUE	UNCONFII COMPRES (tsf)	1	● Moistu 0 20	ure Con 30	tent 40
	9		SAN	RE	οz	COM	□ Und	confined (Compre 6	ssion (tsf) 8
55		FAT CLAY, trace gravel (CH) - Gray - Medium Stiff to Stiff - Moist to Very Moist <i>(continued)</i> Sand seam at 54 feet	SS 18	100	2-2-2 (4)	0.75				
			V V							<u> </u>
									<u>i</u>	
-1		Spoon refusal at 58.5 feet	SS 19		50				<u>i</u>	
60		Auger refusal at 61.0 feet	V V						:	<u>:</u> : :

Bottom of borehole at 61.0 feet.

GEOTECH BH PLOTS - GINT STD US.GDT - 1/24/19 14:18 - \(\)GSGFS02\PROJECTS - ENGINEERING\TETRA TECH\CARNOTITE GEOTECH\GEOTECH\IGIT\IGEOTECH\IGEOTECH\IGEOTECH\IGEOT\IGEOT\IGEOTECH\IGEOTECH\IGEOT

GSG Consultants, Inc. 623 Cooper Court Schaumburg, Illinois 60173 Telephone: 630-994-2600

BORING NUMBER B-2

PAGE 1 OF 3

Changa .	DECLES TO	Fax: 312-733-5612						
CLIE	NT _Te	etra Tech P	ROJEC	T NAME	2FM	Carnotite S	Site	
PROJ	ECT I	NUMBER 18-2084 P	ROJEC	T LOCA	TION_	434 East 2	6th Stre	et
DATE	STAF	RTED 12/7/18 COMPLETED 12/7/18 G	ROUNE	ELEVA	TION_	598.00 ft	н	OLE SIZE 3 1/4"
DRIL	LING	CONTRACTOR GSG Drilling G	ROUNE	WATE	R LEVE	ELS:		
1		METHOD Mud Rotary	$ar{oxtsymbol{oxed}}$ at	TIME O	F DRIL	LING 8.50) ft / Elev	v 589.50 ft
1		Y_JJR CHECKED BY				 L ING N		
1						N/A		
								A OPT NIVALUE
	0			'PE	≿	w iii		▲ SPT N VALUE 20 40 60 80
Ӗ	 	MATERIAL DESCRIPTION		E T	 VEF	NTS	ESS (F	● Mointure Content
DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION		JPL JUM	RECOVERY (%)	BLOW COUNTS (N VALUE)		● Moisture Content 10 20 30 40
,	0			SAMPLE TYPE NUMBER	2	٥٤	UNCONFINED COMPRESSION (tsf)	☐ Unconfined Compression (tsf)
0	1. <u>1. 1. 1.</u>	6 inches of Topsoil						2 4 6 8
27		FILL: SAND, with gravel and brick fragments - Black and Bro	own -					
		Moist		√ ss	50	3-9-15		
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	\bowtie			1			1	
į				\ /			1	
-				SS 2	67	4-2-2 (4)		
5	\bowtie			/\\		(4)	-	
				√ ss		2-3-3	1	
밁 -		SAND, fine grained (SP) - Brown and Gray - Loose to Mediu Dense - Wet	ım	3	78	(6)		
				<u> </u>			1	
		$\bar{\Sigma}$. ,			-	
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10				<u> 4</u>		(7)		
<u>{</u>								
				√ ss		1-2-3	1	
<u>-</u>				∑ 5	83	(5)		
				/ \			1	
<u>-</u> -				SS 6	100	2-5-9		\
15				/\ 6	100	(14)		Ti iT i i
2002								
<u></u> -				\		0.7.	1	
- -				SS 7	100	3-5-8 (13)		<u> </u>
<u><u> </u></u>				/ \			-	
<u>-</u>								
<u> </u>		SAND, fine grained (SP) - Gray - Loose to Medium Dense -	Mot	√ ss	100	4-5-8		
20		SAND, line grained (SP) - Gray - Loose to Medium Dense -	vvet	8	100	(13)		†
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				\ /			1	
-				SS 9	100	6-5-6 (11)		
á				/ \		(- · /	-	
<u>-</u>								
<u>-</u>				√ ss	100	4-5-7		
25				SS 10	100	(12)		

GSG Consultants, Inc. 623 Cooper Court Schaumburg, Illinois 60173 Telephone: 630-994-2600

BORING NUMBER B-2

PAGE 2 OF 3

Fax: 312-733-5612 CLIENT Tetra Tech PROJECT NAME 2FM Carnotite Site PROJECT NUMBER 18-2084 PROJECT LOCATION 434 East 26th Street UNCONFINED COMPRESSION (tsf) ▲ SPT N VALUE SAMPLE TYPE NUMBER RECOVERY (%) BLOW COUNTS (N VALUE) 20 40 60 GRAPHIC LOG DEPTH (ft) Moisture Content0 20 30 40 MATERIAL DESCRIPTION 10 ☐ Unconfined Compression (tsf) SAND, fine grained (SP) - Gray - Loose to Medium Dense - Wet (continued) SS 5-5-5 100 (10) 11 GEOTECH BH PLOTS - GINT STD US.GDT - 1/24/19 14:18 - NGSGFS02/PROJECTS - ENGINEERING\TETRA TECHICARNOTITE GEOTECHIGEOTECHNICAL\FIELD TESTING\18-2084.GPJ SS 3-4-5 100 12 (9)SILTY SAND, fine grained (SM) - Gray - Very Loose to Loose -30 Wet SS 2-2-2 100 13 (4) SS 1-1-2 CLAY (CL/CH) - Gray - Medium Stiff - Very Moist 100 0.75 14 (3) 35 SS 1-1-1 100 0.5 **S**0.7 15 (2) 40 SS 1-1-1 100 0.75 16 (2) 45 SILTY CLAY, trace gravel (CL) - Gray - Stiff to Very Stiff - Moist SS 4-6-8 2.5 (14)50

BORING NUMBER B-2

PAGE 3 OF 3

 CLIENT
 Tetra Tech
 PROJECT NAME
 2FM Carnotite Site

PROJECT NUMBER 18-2084 PROJECT LOCATION 434 East 26th Street

DEPTH (ft) GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY (%)	BLOW COUNTS (N VALUE)	UNCONFINED COMPRESSION (tsf)	▲ SPT N VALUE 20 40 60 80 Moisture Content 10 20 30 40 Unconfined Compression (tsf) 2 4 6 8
HNICAL/FIELD TESTING(18-2084/GPJ) 9	SILTY CLAY, trace gravel (CL) - Gray - Stiff to Very Stiff - Moist (continued) Auger refusal at 61.5 feet	SS 18	100	6-10-13 (23) 2-4-5 (9)	1.75	

Bottom of borehole at 61.5 feet.

GEOTECH BH PLOTS - GINT STD US.GDT - 1/24/19 14:18 - \\GSGFSQ2\PROJECTS - ENGINEERING\TETRA TECH\CARNOTITE GEOTECH\GEOTECHNIG

GSG Consultants, Inc. 623 Cooper Court Schaumburg, Illinois 60173 Telephone: 630-994-2600

BORING NUMBER B-3

PAGE 1 OF 3

1	UNISTRA	Fax: 312-733-5012	PROJECT NAME OF NO. 171 OF								
1			PROJECT NAME 2FM Carnotite Site PROJECT LOCATION 434 East 26th Street								
1											
		METHOD_HSA					50 ft / Ele	ev 585.50 ft			
1		Y JC CHECKED BY				LING N					
1						N/A					
							- 7	▲ SPT N VALUE			
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DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION		LE T	OVE	-OW UNT ALU	NFI RES (st)	Moisture Content			
🛎	GR/ L			SAMPLE TYPE NUMBER	RECOVERY (%)	BLOW COUNTS (N VALUE)	UNCONFINED COMPRESSION (tsf)	10 20 30 40 ☐ Unconfined Compression (tsf)			
0				8/			58	2 4 6 8			
		FILL: SAND, with gravel and concrete fragments - Brown a Gray - Wet	ınd								
-		oldy Wol				6-10-3	1				
: 				SS 1	33	(13)		 			
				<u> </u>			1				
	\bowtie	SAND, fine grained (SP) - Brown and Gray - Loose to Med	ium	\ /			-				
-		Dense - Moist to Wet	iuiii	SS 2	56	2-3-4 (7)					
5				/ \ _		(.,	-				
				V ss	67	3-4-4					
<u> </u>				3		(8)					
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20		SAND, fine grained (SP) - Gray - Loose to Medium Dense	- Wet	8	89	(7)		 			
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S				SS 9	89	3-4-6 (10)					
				<u> </u>		. ,	1				
_							-				
-				SS 10	100	3-6-6					
25				/\ 10		(12)					

GSG Consultants, Inc. 623 Cooper Court Schaumburg, Illinois 60173 Telephone: 630-994-2600 Fax: 312-733-5612

BORING NUMBER B-3

PAGE 2 OF 3

CLIENT Tetra Tech

PROJECT NAME 2FM Carnotite Site

PROJECT NUMBER 18-2084 PROJECT LOCATION 434 East 26th Street

		PROJECT LOCATION 434 East 26th Street						
MATERIAL DESCRIPTION 25	SAMPLE TYPE NUMBER	RECOVERY (%)	BLOW COUNTS (N VALUE)	UNCONFINED COMPRESSION (tsf)	▲ SPT N VALUE 20			
SAND, fine grained (SP) - Gray - Loose to Medi (continued) SILTY SAND (SM) - Gray - Loose - Wet	ium Dense - Wet	100	2-2-2 (4)					
FAT CLAY (CH) - Gray - Soft to Medium Stiff - V	SS 12	100	1-2-2 (4)					
FAT CLAY (CH) - Gray - Solt to Medium Still - V	ss 13	100	2-2-2 (4)	0.25				
35 35	SS 14	100	2-2-2 (4)	0.5	7.5			
SILTY SAND (SM) - Gray - Loose - Wet	SS 15	100	2-2-2 (4)					
SILTY CLAY (CL) - Gray - Soft to Stiff - Moist to	SS 16	100	3-4-4 (8)	1.25				
SILTY CLAY (CL) - Gray - Soft to Medium Stiff - V SILTY SAND (SM) - Gray - Loose - Wet SILTY CLAY (CL) - Gray - Soft to Stiff - Moist to	SS 17	89	3-3-3 (6)	1.25				

BORING NUMBER B-3

PAGE 3 OF 3

CLIENT Tetra Tech

PROJECT NAME 2FM Carnotite Site

PROJECT NUMBER 18-2084 PROJECT LOCATION 434 East 26th Street

ļ								
	DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY (%)	BLOW COUNTS (N VALUE)	UNCONFINED COMPRESSION (tsf)	▲ SPT N VALUE 20 40 60 80 Moisture Content 10 20 30 40 Unconfined Compression (tsf) 2 4 6 8
PJ.	 55 		SILTY CLAY (CL) - Gray - Soft to Stiff - Moist to Very Moist (continued)	SS 18	100	2-2-2 (4)	1.0	A
L\FIELD TESTING\18-2084.G	60			SS 19	33	2-3-3 (6)	0.25	•
NGINEERING\TETRA TECH\CARNOTITE GEOTECH\GEOTECHNICAL\FIELD TESTING\18-2084.GPJ	 		SILT, trace clay and limestone fragments (ML) - Gray -Dense to Very Dense - Moist	SS 20	56	16-22-26 (48)		
IG\TETRA TECH\CARNOTIT				<u> </u>				
NGINEERIN	70		Spoon refusal at 70.0 feet	SS 21		50		

Bottom of borehole at 70.0 feet.

GEOTECH BH PLOTS - GINT STD US.GDT - 1/24/19 14:18 - \(GSGFS02\\PROJECTS - EN



Unified Soil Classification

Soil Classification is based on the Unified Soil Classification System and ASTM Designations D-2487 and D-2488. Coarse Grained Soils have more than 50% of their dry weight retained on a #200 sieve; they are described as: clays, if they are plastic, and silts if they are slightly Plastic or non-plastic. Major constituents may be added as modifiers and minor constituents may be added according to the basis of their relative in-place density and fine grained soils on the basis of their consistency. Example: Lean clay with sand, trace gravel, stiff (CL); silty sand, trace gravel, medium dense (SM).

Drilling & Sampling Symbols

SS: Split Spoon Water Level (ft)
ST: Thin-Walled Tube

While Drilling

HS: Hand Sample

Standard "N" Penetration: Blows per foot of a 140 pound hammer falling 30 inches on a 2 inch OD split spoon, except where poted

				hammer falling 30 inches on a 2 inch OD split spoon, except where noted.				
Major Divisions				Group Symbols	Typical Names	Consistency of Cohesive Soil		
	(More than Half of material is larger than No. 200 sieve size)	Gravels (More than hall of coarse fraction is larger than No. 4 sieve size)	Clean Gravels (Little or no fines)	GW	Well graded gravels, gravel- sand mixtures, little or no fines	Unconfined Compressive N- strength, Qu, tsf Blows/ft. Consiste	ency	
			Clean (Little o	GP	Poorly graded gravels, gravel-sand mixtures, little or no fines	 < 0.25 Below 2 < Very Soft 0.25 - 0.50 2-4 - Soft 		
oils			s with es ciable	GM 1	- clay mixtures	0.50 - 1.0 4-8 - Medium Sti 1.0 - 2.0 8-15 - Stiff	iff	
Coarse Grained Soils			Gravels with fines (Appreciable amount of fines)	GC	Clayey gravels, gravel-sand- clay mixtures	2.0 - 4.0 15-30 - Very Stiff 4.0 - 8.0 30-50 - Hard		
Coarse G	material	Sands (More than hall of coarse fraction is smaller than No. 4 sieve size)	Sands no fines)	SW	Well graded sands, gravelly sands, little or no fines	> - 8.0 > 50 - Very Hard		
	an Half of		Clean Sands (Little or no fines)	SP	Poorly graded sands, gravelly sands, little or no fines	Relative Density of Coarse-Grained Soils		
	More th		Sands with fines (Appreciable amount of fines)	SM C	mixtures	N-Blows/ft. Relative Density 0-3 Very Loose		
				SC	Clayey sands, sand-clay mixtures	4-10 Loose 11-29 Medium Dense		
	sieve size)	Silts and Clays (liquid limit less than 50)		ML	Inorganic silts and very fine sands, rock flour, silty or claye fine sands or clayey silts with slight plasticity	30-49 Dense 50-80 Very Dense		
	smaller than No. 200 sieve size)			CL	Inorganic clay of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays	>80 Extremely Dense Description Term(s) of Components Present in Sa	mnle	
ed Soils	ller tha			OL	Organic silts and organic silty clays of low plasticity	Trace < 10% Little 10-19% Some 20-34% And 35-50%		
	More than half of material is sma	Silts and Clays (liquid limit greater than 50)		МН	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts	PLASTICITY CHART		
Fine				СН	Inorganic clays of high plasticity, fat clays	€ 50 CH		
				ОН	Organic clays of medium to high plasticity, organic silts	30 PI = 0, 73(LL-20) CL MH&OH		
		High Organic		Pt	Peat and other highly organic soils	2 20	0	

APPENDIX C

LAB RESULTS

623 Cooper Court Schaumburg, IL 60173 Tel: 630.994.2600

ATTERBERG LIMITS - GINT STD US LAB.GDT - 1/24/19 10:41 - \\GSGFS02\PROJECTS - ENGINEERING\TETRA TECHICARNOTITE GEOTECH\GEOTECH\GEOTECHNICAL\FIELD TESTING\18-2084. GPJ

ATTERBERG LIMITS' RESULTS

PROJECT NAME 2FM Carnotite Site CLIENT Tetra Tech PROJECT NUMBER 18-2084 PROJECT LOCATION 434 East 26th Street (CL) (CH) 50 L A S T I 40 C T Y 30 ١ \blacksquare N D E X 20 * 10 CL-ML (ML) (MH) 0 20 40 0 60 80 100 LIQUID LIMIT **BOREHOLE DEPTH** LL PL PI Fines Classification ● B-1 29 38.5 55 26 **▼** B-2 33.5 45 22 23 53.5 20 lack**B-2** 40 20 ★ B-3 43.5 33 17 16

<u>ATTACHMENT 10 – APPENDIX II-C "AFFIDAVIT IN SUPPORT OF SOIL EROSION AND SEDIMENT CONTROL – MEASURES DURING CONSTRUCTION</u>



APPENDIX II-C City of Chicago - DWM Bureau of Engineering Services - Sewer Design Section

Project Name:

Design /Construction Affidavit in Support of Soil Erosion and Sediment Control Measures during Construction

Former Carnotite Reduction Company Site

Property Address(es)(Property): 434 East 26th Street, Chicago, IL														
The Developer/Owner Affidavit and have pe			•	,										

The Developer/Owner and General Contractor (hereafter the Affiants) have authority to sign this Affidavit and have personal facts of the matters contained herein. The Affiants acknowledge that the plans, dated _______, and approved by the DWM/DOB (Plans) form the basis for this Affidavit. The Affiants further acknowledge that Regulated Developments, under the City's Stormwater Ordinance, that discharge to a combined sewer system, must provide functional and effective construction soil erosion and sediment control (SESC) at the Property as identified above. Further, the Affiants assume full responsibility for the design, construction and maintenance of SESC measures to prevent the discharge of sediment, dust, and other pollutants in stormwater runoff from the Property.

Construction SESC measures will be installed at the above address(es) prior to land disturbing activities and be maintained in functional order until the property has been "permanently stabilized" (i.e. when all land disturbing activities have been completed, all construction SESC measures have been removed, and an uniform perennial vegetative cover with a density of 70 percent for unpaved areas and areas not covered by permanent structures has been established or equivalent permanent stabilization measures have been completed).

Any breach of the conditions contained in this Affidavit, as determined solely by the City of Chicago, that are not cured by the Developer/Owner within 7 (seven) days of official notice, the City of Chicago may utilize any and all legal and equitable remedies available to the City."

As the General Contractor of the subject Property, I certify adherence to this Affidavit and to the following:

Design /Construction Affidavit in Support of Soil Erosion and Sediment Control Measures during Construction Page two

The SESC measures will be designed, constructed and maintained in accordance with standards and specifications set forth in the most recent version of the Illinois Urban Manual published by the Illinois Environmental Protection Agency (IEPA) and the Natural Resources Conservation Service (NRCS). As a minimum, all temporary SESC measures such as vegetative cover, silt fences, inlet protection, check dams, etc., shall be designed to accommodate anticipated 1-year storm flows.

Any applicable Storm Water Pollution Prevention Plan (SWPPP) will be followed along with the minimum SESC measures specified herein. The SWPPP will be kept onsite during construction for inspection.

Temporary soil stabilization will be applied to topsoil stockpiles and disturbed areas where construction activity will not occur for a period of more than 21 calendar days.

Permanent soil stabilization shall be done within 14 calendar days after completion of final grading of the soil

Inspection of SESC measures will be completed at least once every 7 calendar days and within 24 hours of a storm 0.5 inches or greater. SESC measures will be maintained to perform their intended function until the site is permanently stabilized.

All temporary roadways, access drives and parking areas will be stabilized and be of sufficient width and length to prevent sediment from being tracked onto public or private roadways. Any sediment reaching a public or private road shall be removed by street cleaning (not by water flushing) as necessary, or before the end of each workday.

Tires and wheel wells of vehicles and construction equipment shall be free of dirt and/or sediment before leaving a construction area to prevent tracking onto a public or private paved road, or sidewalk.

Trucks loaded with waste material that may be carried off by wind or rain shall be covered prior to leaving the construction site.

All onsite drainage structures within the construction area and down slope within the public right-of-way shall be protected with sediment control measures.

The discharge of sediment into the sewer system, as part of site dewatering, must be controlled and minimized to prevent clogging of the City's sewer system.

The use, storage and disposal of chemicals, cement and other compounds and building materials used on the construction site shall be managed during the construction period, to prevent their entrance into the City's sewer system.

All temporary SESC measures will be removed within 30 days after final site stabilization is achieved or after temporary measures are no longer needed.

Unless adequate sediment and erosion control measures are implemented for all onsite infiltration BMP systems, the installation of such systems will be scheduled after all of the major construction activity is completed.

Design /Construction Affidavit in Support of Soil Erosion and Sediment Control Measures during Construction Page three

Signed by General Contractor	
Name/Company:	
Address:	
Phone Number:	
	Data
Signature:	, Date
Contractor License Number:	
As the Developer/Owner of the sub	ject Property, I certify adherence to this Affidavit.
The Developer of the sub	Jeet Property, I certify auncience to this minute.
Signed by Developer/Owner	
Name/Company:	
Address:	
Phone Number:	
Signature:	Date

ATTACHMENT 11 – OPERATION AND MAINTENANCE PLAN INCLUDING OWNER'S CERTIFICATION STATEMENT





FORMER CARNOTITE REDUCTION COMPANY SITE

SITE REMEDIATION PROJECT 434 E. 26th Street Chicago Illinois

OPERATION AND MAINTENANCE PLAN STORMWATER BEST MANAGEMENT PRACTICES

MARCH 2020

TABLE OF CONTENTS

- 1.0 DRY DETENTION BASIN DETAILS 1
- 2.0 DRY DETENTION BASIN -INSPECTION ACTIVITIES 1
- 3.0 DRY DETENTION BASIN OPERATION AND MAINTENANCE ACTIVITIES 1

Attachment 1 - Vortex Fabrication Drawing Sheet and Rating Curve

REVISIONS

*revisions to the plan procedures and practices must be done annually

REVISION NUMBER	DATE
00	November 2019
01	March 2020

1.0 DRY DETENTION BASIN DETAILS

Owner: City of Chicago, Department of Assets, Information & Services (AIS)

Ongoing Maintenance: AIS, Bureau of Facility Operations (Julie Bedore, 312-744-7594)

Side slopes: 1V / 3H

Admissible release rate: 0.27 cubic feet per second/acre (cfs/ac)

Basin depth: 2 ft Slope of basin bottom: 0.1 %

Basin total volume capacity: 15,784 cubic feet (ft)

Sedimentation catch basin capacity: 9 cubic ft High Water Level (100 yr): 1.72ft

HWL (100yr) volume: 12,664 cubic ft

Vortex Model FA1214 with 4" discharge orifice from Contech Engineered Solution (or equivalent).

Vortex regulator release rate 0.22 cfs

2.0 DRY DETENTION BASIN -INSPECTION ACTIVITIES

Inspection of the basin after rain events exceeding 1.5 inches over a 24-hour period will be conducted to ensure that all components of the stormwater management system for the site are working and to ensure that the detention basin and regulator are functioning as designed and comply City of Chicago regulations. The list below provides a summary of the operation and maintenance activities planned after each a rain event exceeding 1.5 inches.

- General inspection of the installations, including dry basin, sediment trap, outlet, catch basin, manhole
 and vortex regulator to ensure minimal sediment accumulation and no debris blocking pipe flow to the
 manhole.
- Confirm basin is dry after 72 hours of a 1.5 inch or greater rain event over a 24-hour period.

3.0 DRY DETENTION BASIN - OPERATION AND MAINTENANCE ACTIVITIES

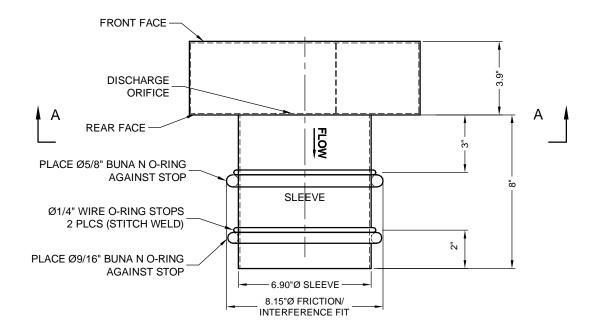
Below is a list of operation and maintenance activities to be completed after construction is complete. To document operation and maintenance activities, each site inspection, visit, and other site activities will be recorded in the log book. The operation and maintenance schedule for each activity is noted below. In the event of a 1.5 inch or greater rain event, each activity will be conducted and documented. Specific operation and maintenance activities associated with the dry detention basin and associated stormwater management include:

- Visual inspection of detention basin for evidence of erosion, tree growth (root damage), basin damage (settling or cracking of the slopes and bottom), sediment accumulation, blockage of pipes (at least twice a year). The outlet and vortex will also be inspected for blockage, vegetative growth that may impair operation, and functionality (at least twice a year).
- Inspect upstream conditions that could affect basin operation.
- Inspection and cleaning of catch basin and manhole lid (at least twice a year). Remove sediment from bottom of catch basin every two years.
- Remove sediment from sediment trap in the basin as needed.
- Remove sediment from detention basin as needed to keep designed levels (at least every 5 to 10 years).
- Clean debris (including trash and litter) and invasive vegetation from basin, outlet pipe and rip-rap (at least every year).

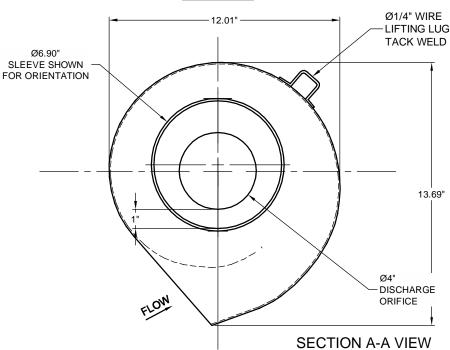
ATTACHMENT 1 – VORTEX FABRICATION DRAWING SHEET AND RATING CURVE

TETRA TECH

- 1. ALL WELDS CONTINUOUS, UNLESS OTHERWISE NOTED
- 2. MATERIALS:
- 1/8" ALUMINUM 5052
- (1) 5/8" AND (1) 9/16" BUNA N, 50 DUROMETER O-RINGS



TOP VIEW



This CADD file is for the purpose of specifying stormwater flow control equipment to be furnished by Contech Engineered Solutions LLC and may only be transferred to other documents exactly as provided by Contech Engineered Solutions LLC. Title block information, excluding the Contech Engineered Solutions LLC logo and the Fluidic-Amp or Fluidic-Cone designation and patent number may be deleted if necessary. Revisions to any part of this CADD file without prior coordination with Contech Engineered Solutions LLC shall be considered unauthorized use of proprietary information.



FABRICATION DRAWING FOR FLUIDIC-AMP VORTEX VALVE MODEL FA1214 WITH SLEEVE ATTACHMENT SIZED FOR 8" PIPE (8.0" ID) PROJECT NAME LOCATION

200 Enterprise Drive, Scarborough, ME 04074

77-907-8676 207-885-9830 207-885-9825 FAX

FAX DAT

DATE:01/09/2020 SCALE: 1:5

FILE NAME

DRAWN: NDC

CHECKED: NDC

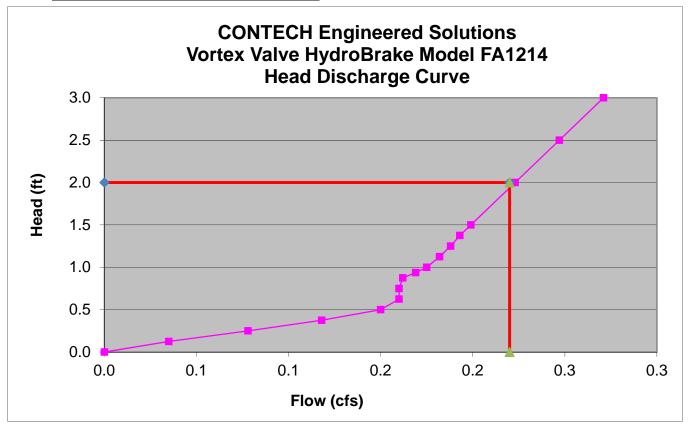
PPS/52 VORTEX VALVE/44 FABRICATION/CHICAGO ALUM 5052/DO NOT USE - STANDARD PIPE 8-10 INCH. DWG 1/9/2020 1:40 PM



Vortex Valve FA1214 with 4" Opening

(ft) (cfs) 0 0 0.125 0.035 0.25 0.078 0.375 0.118 0.50 0.150 0.625 0.160 0.875 0.162 0.938 0.169 1.00 0.175 1.125 0.182 1.375 0.193 1.50 0.199 2.00 0.223 2.50 0.247 3.00 0.271
0.125 0.035 0.25 0.078 0.375 0.118 0.50 0.150 0.625 0.160 0.875 0.162 0.938 0.169 1.00 0.175 1.125 0.182 1.25 0.188 1.375 0.193 1.50 0.199 2.00 0.223 2.50 0.247
0.25 0.078 0.375 0.118 0.50 0.150 0.625 0.160 0.75 0.162 0.938 0.169 1.00 0.175 1.125 0.182 1.25 0.188 1.375 0.193 1.50 0.199 2.00 0.223 2.50 0.247
0.375 0.118 0.50 0.150 0.625 0.160 0.75 0.160 0.875 0.162 0.938 0.169 1.00 0.175 1.125 0.182 1.25 0.188 1.375 0.193 1.50 0.199 2.00 0.223 2.50 0.247
0.50 0.150 0.625 0.160 0.75 0.160 0.875 0.162 0.938 0.169 1.00 0.175 1.125 0.182 1.25 0.188 1.375 0.193 1.50 0.199 2.00 0.223 2.50 0.247
0.625 0.160 0.75 0.160 0.875 0.162 0.938 0.169 1.00 0.175 1.125 0.182 1.25 0.188 1.375 0.193 1.50 0.199 2.00 0.223 2.50 0.247
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0.938 0.169 1.00 0.175 1.125 0.182 1.25 0.188 1.375 0.193 1.50 0.199 2.00 0.223 2.50 0.247
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1.125 0.182 1.25 0.188 1.375 0.193 1.50 0.199 2.00 0.223 2.50 0.247
1.25 0.188 1.375 0.193 1.50 0.199 2.00 0.223 2.50 0.247
1.375 0.193 1.50 0.199 2.00 0.223 2.50 0.247
1.50 0.199 2.00 0.223 2.50 0.247
2.00 0.223 2.50 0.247
2.50 0.247
3.00 0.271
0.00 0.271
3.50 0.298
4.00 0.325
4.50 0.351
5.00 0.377
5.50 0.406
6.00 0.435
6.50 0.464
7.00 0.494
7.50 0.526
8.00 0.558
8.50 0.592
9.00 0.626
9.50 0.663
10.00 0.700

Target Head:	2 ft
Target Flow:	0.22 cfs
Achieved Flow:	0.22 cfs



Operation and Maintenance Plan Owner's Certification Statement

Property Name: Former Carnotite Reduction Company Site

Property Address: 434 East 26th Street, Chicago, IL

As the owner(s) of the subject property, by signing this document, I/we acknowledge that I/we have received and reviewed the Operation and Maintenance Plan, dated March 2020 and understand its contents. (as required by the Stormwater Management Ordinance, Section 11-18-030).

In the event that I/we were to sell this property, I/we agree to give a copy of the Plan to the new owner(s) and this Owner's Certification Statement for signature. This signed Certification Statement must be submitted to the City's Department of Buildings upon transfer of ownership.

I/we further agree to adhere to the maintenance schedule of best management practices stipulated in the Plan. I/we also acknowledge that if I/we don't maintain the measures as shown in the Plan, upon City inspection, I/we could be liable for a violation of the City's Municipal Code (according to Stormwater Management Ordinance Section 11-18-130).

Initial Owner(s) Printed Name Lewer Land Lungton Initial Owner(s) Signature	3/13/2e Date	CARMEN J VEGA Official Seal Hotary Public - State of Illinois ov Commission Expires Feb. 15, 2023 Notary Public
2nd Owner(s) Printed Name	A N A O WAR	
2nd Owner(s) Signature	Date	Notary Public
3rd Owner(s) Printed Name		
3rd Owner(s) Signature	Date	Notary Public

<u>ATTACHMENT 12 – TETRA TECH MEMO "SUMMARY OF SEWER CLEANOUT AND INVESTIGATION</u>







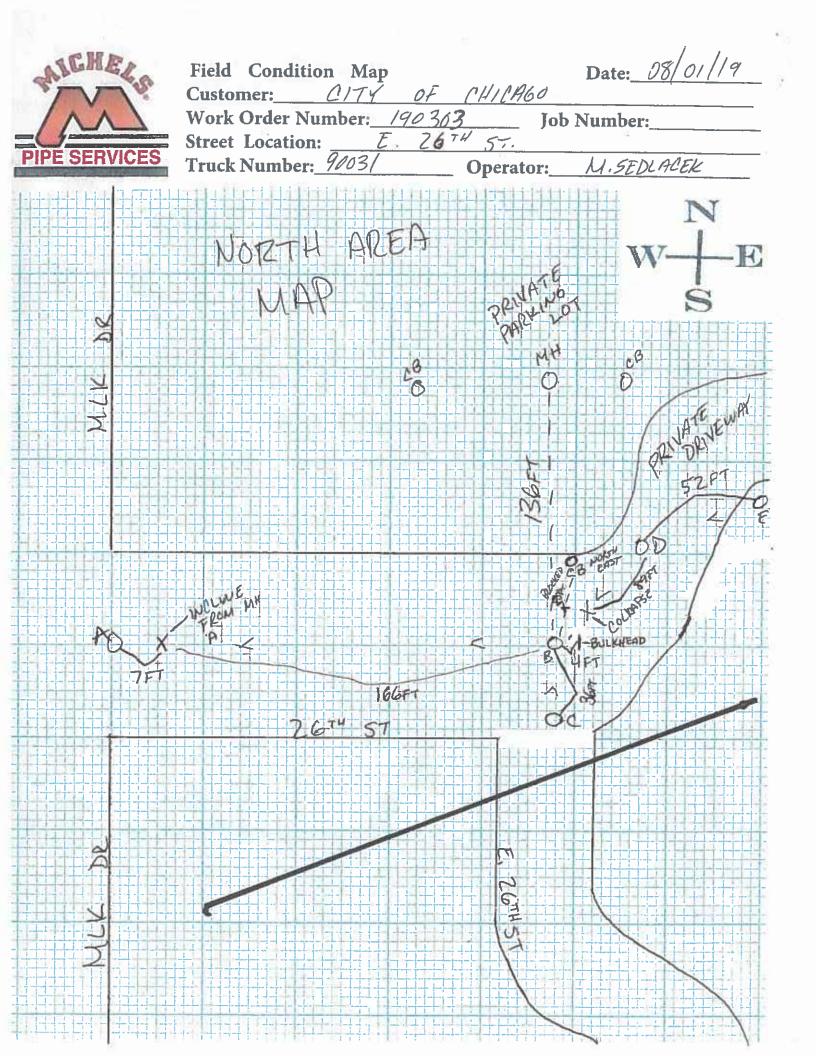
То:	Abby Mazza, City of Chicago Department of Fleet and Facility Management (2FM)
Cc:	Carol Nissen, Tetra Tech, Inc. (Tetra Tech)
From:	Kris Schnoes, Tetra Tech
Date:	October 7, 2019
Subject:	Summary of Sewer Cleanout and Investigation Former Carnotite Reduction Company Site (Carnotite) 434 E. 26 th Street Chicago, IL

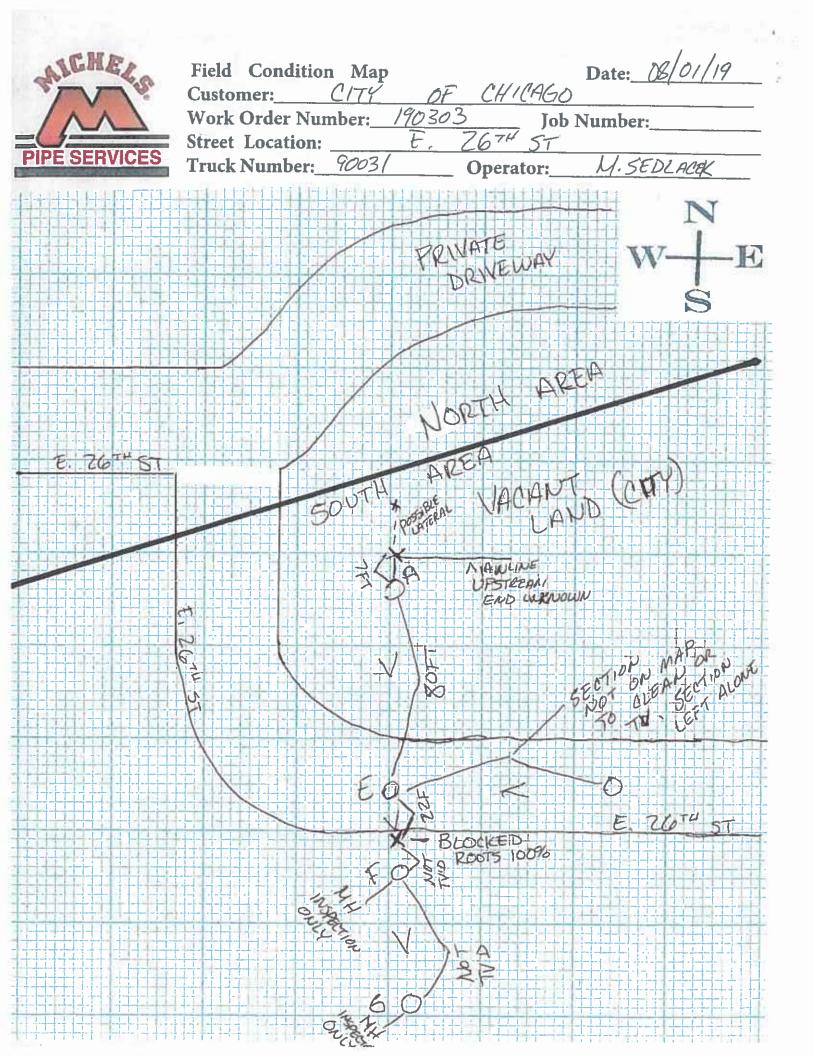
On July 31 and August 1, 2019, 2FM conducted sewer cleanout and investigation activities at the Carnotite site as required by the City of Chicago Department of Sewers prior to removal of sewers during remediation activities. 2FM conducted the sewer investigation in planned excavation areas to determine if any unknown connections to the combined sewers exist. Prior to televising the sewer, SET Environmental, Inc. (SET) cleaned the sewers by hydro-jetting and collected the sewer water and sludge in one 25-cubic yard vacuum box and five 300-gallon totes. The vacuum box and totes remained onsite for waste characterization prior to disposal. Once sewer cleanout was complete, Michel's Pipe Service conducted sewer televising activities. Tetra Tech was present during portions of the sewer cleanout and investigation. Also, Stan A. Huber Consultants, Inc. conducted radiation screening of the work areas and equipment during cleanout and investigation activities and screened all equipment in contact with potentially contaminated sewer water and sludge for free release after decontamination of the equipment was conducted, as necessary.

The sewer cleanout and investigation began in the south portion of the future remediation area, along 26th Street as it bends from a north-south orientation to an east-west orientation near the tennis courts. The investigation then continued to the north portion of the future remediation area, along 26th Street east of Martin Luther King Drive. The following information was documented during sewer televising activities:

- In the south area, the 12-inch vitrified clay pipe (VCP) sewer south of 26th Street was completely blocked with roots and debris.
- In the south area, the 12-inch VCP sewer north of 26th Street was inspected.
- In the north area, the 26th Street sewer was observed to be 12-inch diameter VCP east of the vegetated circular center median in 26th Street and 18-inch diameter VCP west of the vegetated median. A portion of the sewer pipe is corrugated metal pipe approximately 7 feet east of Martin Luther King Drive.
- In the north area, two previously unknown sewer connections to the north 26th Street sewer from the adjacent property to the north were discovered. One 10-inch VCP sewer drains the southwest parking lot located on the property north of the site. One 8-inch VCP sewer appears to connect to a combined sewer from the building located on the north property as well as drain storm water from the grass area located east of the existing driveway entering the north property from 26th Street.
- The 12-inch VCP sewer along north 26th Street was observed to be in poor condition in some locations, including a collapsed section in the eastern portion of 26th Street located about 90 feet west-southwest of the driveway entering the north property from 26th Street.

Hand drawn field maps prepared by Michel's are attached. Michel's report also included video and photos of the inspected sewer pipes.



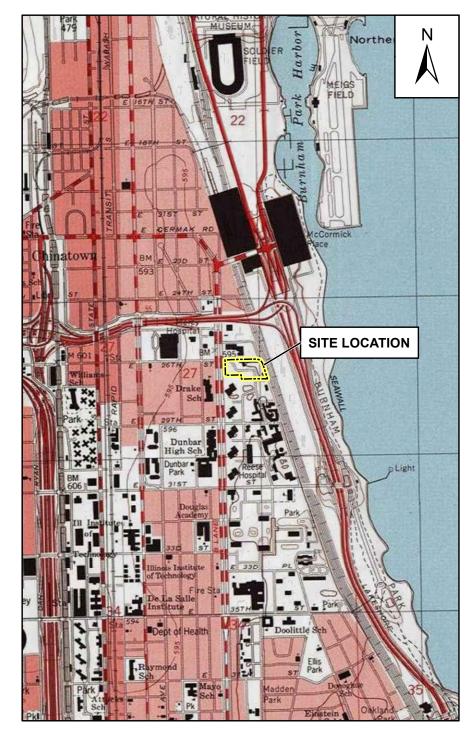


<u>ATTACHMENT 13 – PLAN SHEETS</u>



FORMER CARNOTITE REDUCTION COMPANY SITE EXCAVATION PLAN CHICAGO, ILLINOIS

SHEET	DESCRIPTION
C-1	COVER SHEET
C-2	EXISTING CONDITIONS
C-3	SOIL ANALYTICAL RESULTS
C-4	EXTEND OF SUBSURFACE
C-5	GEOLOGICAL CROSS SECTIONS
C-6	DECOMMISSIONING PLAN
C-7	REMEDIATION EXCAVATION PLAN
C-8	PROPOSED REMEDIATION SITE LAYOUT
C-9	PROPOSED EROSION CONTROL PLAN
C-10	EROSION CONTROL DETAIL
C-11	GRADING PLAN AND PROPOSED STORM AND SANITARY SEWER
C-12	STORM AND SANITARY SEWER DETAILS
C-13	RESTORATION PLAN
C-14	OPERATION AND MAINTENANCE PLAN
T-1	SIGN LOCATION AND MAINTENANCE OF TRAFFIC PLAN
T-2	SIGN TYPE 1
T-3	SIGN TYPE R11 AND OM4
T-4	SIGN MOUNTING - BAND MOUNT AND FENCE MOUNT
T-5	SIGN MOUNTING - NEW SIGN POSTS
T-6	SIGN MOUNTING AT CONCRETE BARRIER
S-1	SITE PLAN
S-2	EXCAVATION PLAN
S-3	EXCAVATION CROSS SECTION
S-4	TEMPORARY RETAINING WALL
S-5	TEMPORARY RETAINING WALL DETAILS
S-6	SEWER PLAN AND PROFILE
S-7	CITY OF CHICAGO STANDARD DETAILS





NOTES:

1. NO CITY BRNCHMARK COULD BE USED FOR THIS LOCATION.

2. LEGEND FOR ABBREVIATIONS AND SYMBOLS ARE SPECIFIC FOR EACH SHEET.

 DESIGNED:
 C. NISSEN

 DRAWN:
 M.BANH

 PROJECT NO.
 103S328401004

 DATE:
 JULY 2019



DEPARTMENT OF FLEET AND FACILITY MANAGEMENT 30 NORTH LASALLE ST. SUITE 300 CHICAGO, IL 60602 TETRA TECH
1 SOUTH WACKER DR
SUITE #3700
CHICAGO, IL 60606
312.201.7700

FORMER CARNOTITE REDUCTION COMPANY SITE 434 E. 26th STREET CHICAGO, ILLINOIS

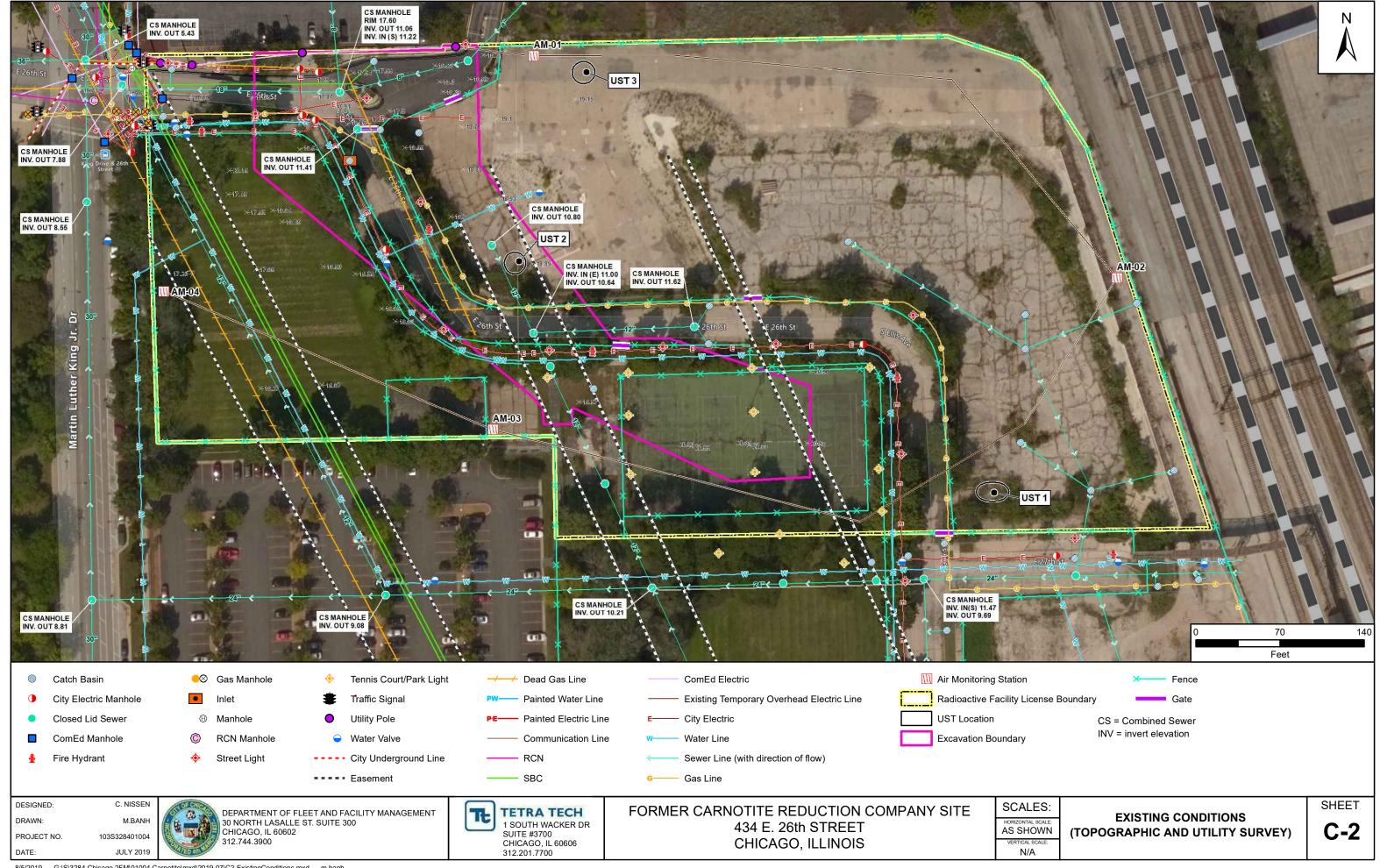
SCALES:

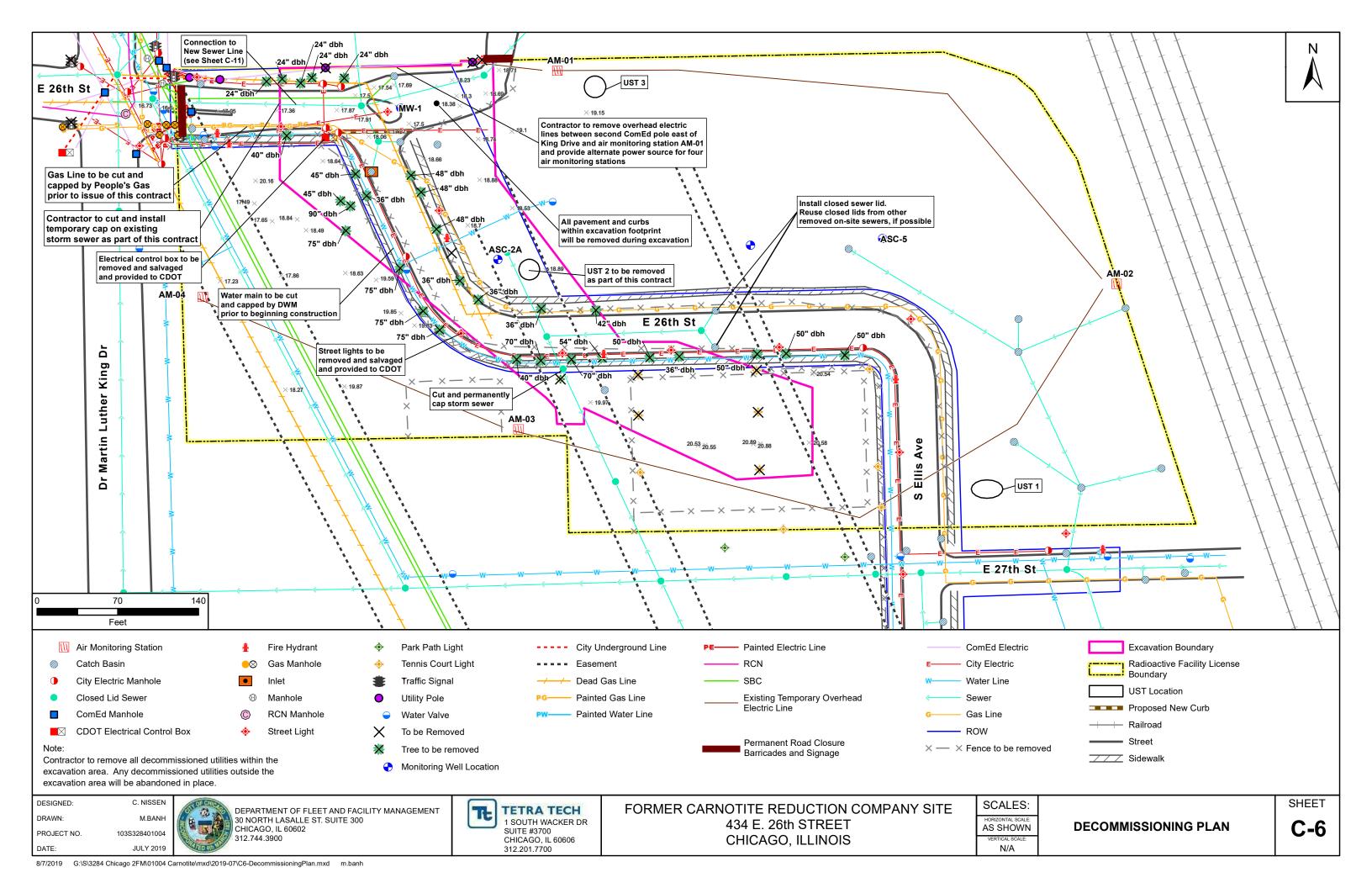
HORIZONTAL SCALE:
AS SHOWN

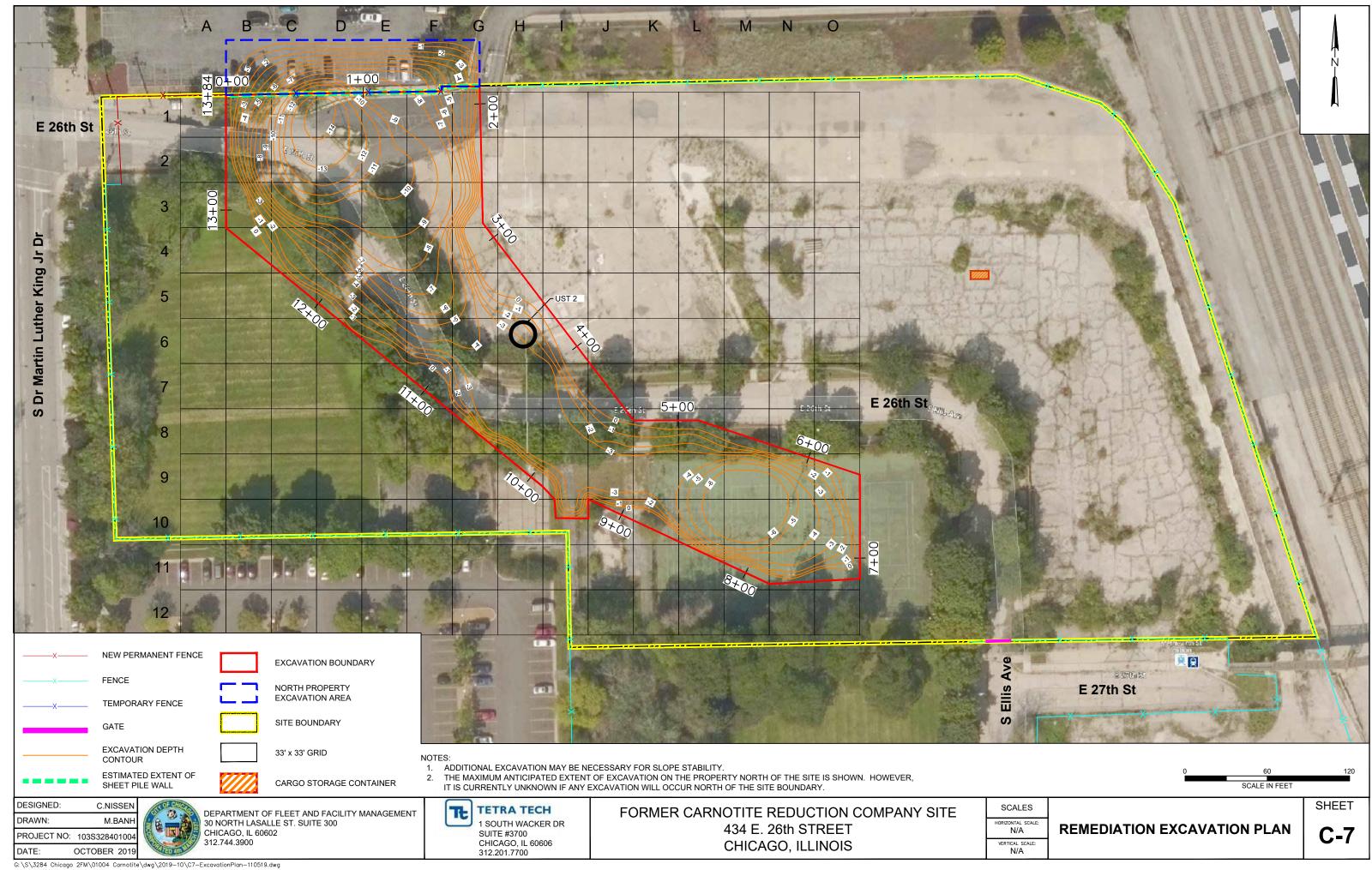
VERTICAL SCALE:
N/A

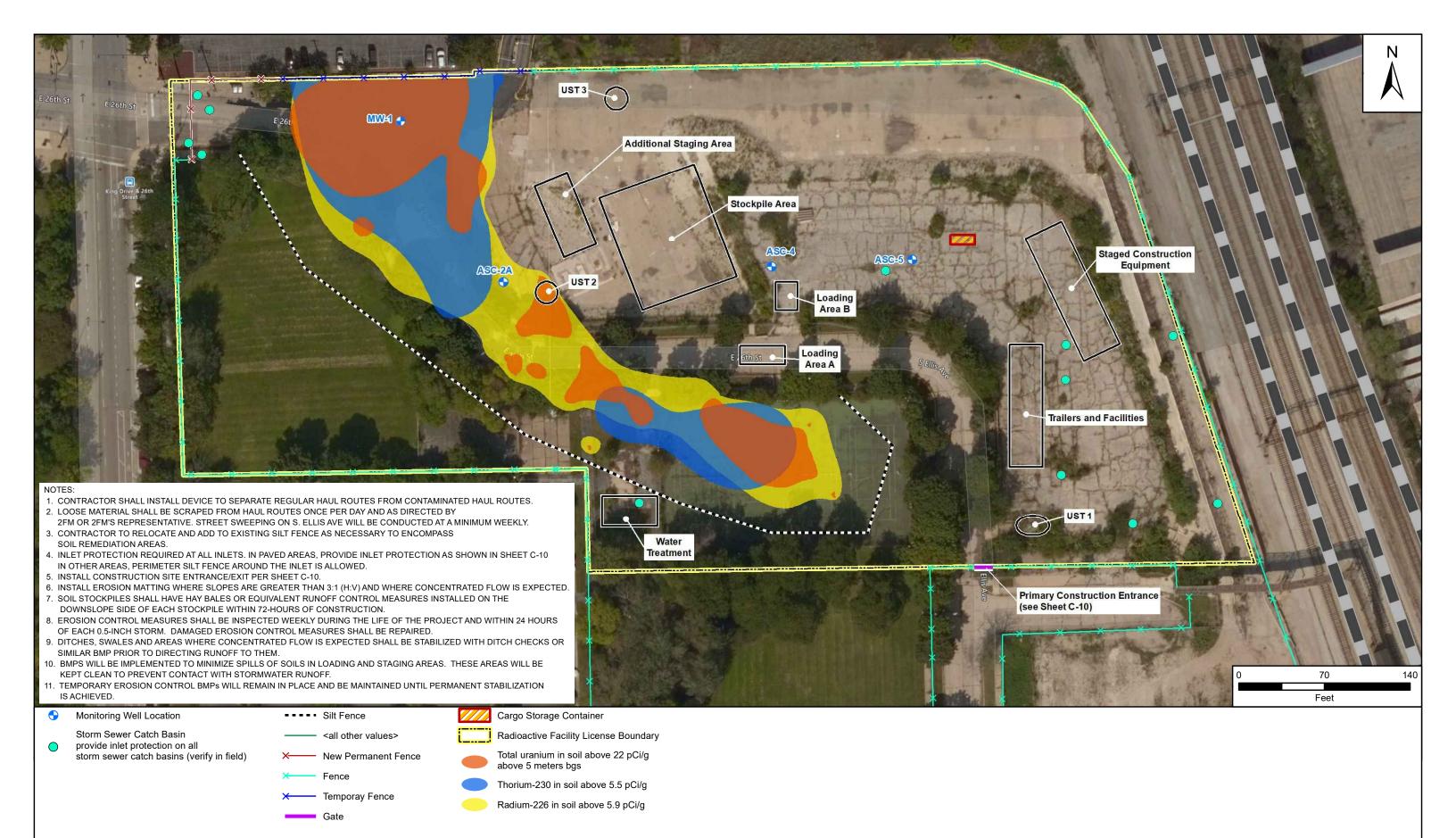
COVER SHEET

SHEET C-1









DESIGNED: C. NISSEN
DRAWN: M.BANH
PROJECT NO. 103S328401004

DATE:



DEPARTMENT OF FLEET AND FACILITY MANAGEMENT 30 NORTH LASALLE ST. SUITE 300 CHICAGO, IL 60602



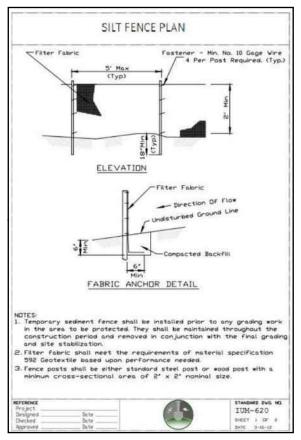
FORMER CARNOTITE REDUCTION COMPANY SITE 434 E. 26th STREET CHICAGO, ILLINOIS

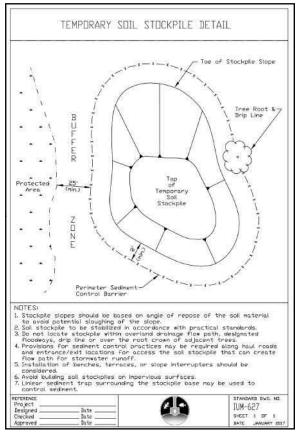
SCALES:
HORIZONTAL SCALE:
AS SHOWN
VERTICAL SCALE:
N/A

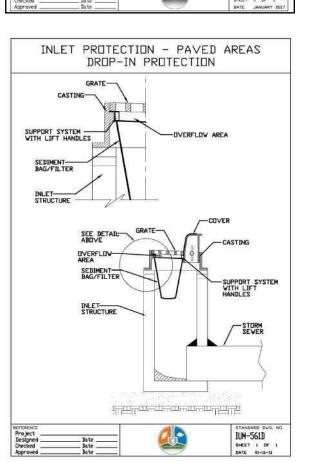
PROPOSED EROSION CONTROL PLAN

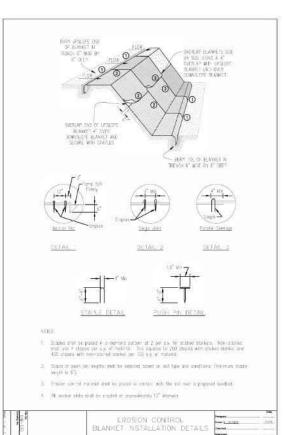
C-9

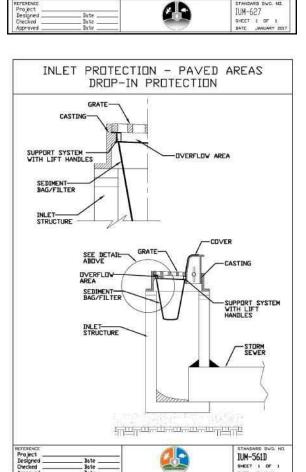
JULY 2019













DESIGNED:

PROJECT NO.

DRAWN.

DATE:



DEPARTMENT OF FLEET AND FACILITY MANAGEMENT 30 NORTH LASALLE ST. SUITE 300 CHICAGO, IL 60602



FORMER CARNOTITE REDUCTION COMPANY SITE 434 E. 26th STREET CHICAGO, ILLINOIS

STABILIZED CONSTRUCTION ENTRANCE PLAN

Positive Drainage To Sedinent Trapping Device.

PLAN VIEW

SIDE ELEVATION

NOTES

1. Filter fabric shall neet the requirements of naterial specification 598 GEDTEXTILE. Table I or 2. Class I. II or IV and shall be placed over the cleared area prior to the placing of rock.

2. Rock or reclained concrete shall neet one of the following IDDT coarse aggregate gradation, CA-I., CA-2, CA-3 or CA-4 and be placed according to construction specification 25 RDCXTILL using placement Method I and Class III compaction.

3. Any deniange facilities required because of washing shall be constructed according to nanufacturers specifications.

4. If wash rocks are used they shall be installed according to the nanufacturer's specifications.

IL-630 SHEET 1 OF 2 DATE 8-19-94

Must Extend Full Width Of Ingress And Egress Operation.

SCALES: AS SHOWN VERTICAL SCALE: N/A

EROSION CONTROL DETAIL

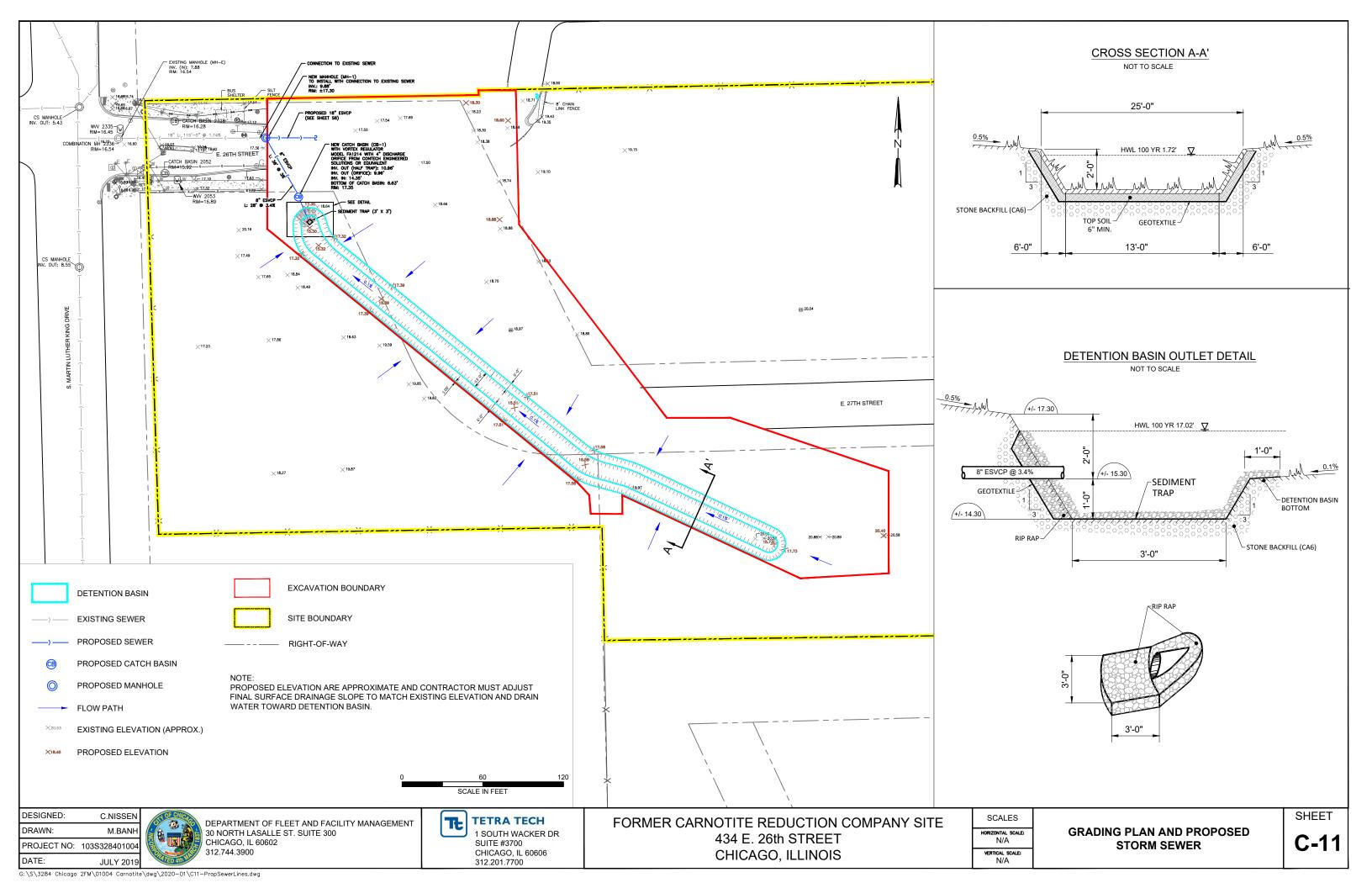
STABILIZED CONSTRUCTION ENTRANCE PLAN

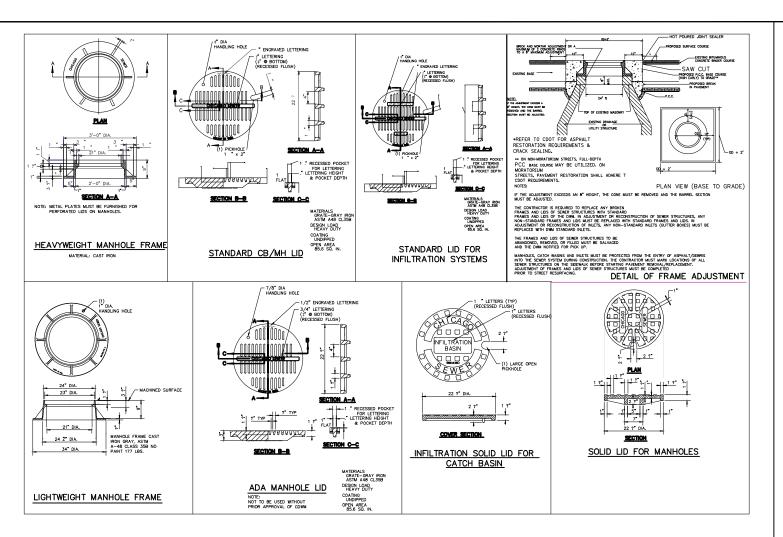
SECTION A-A

SECTION B-B

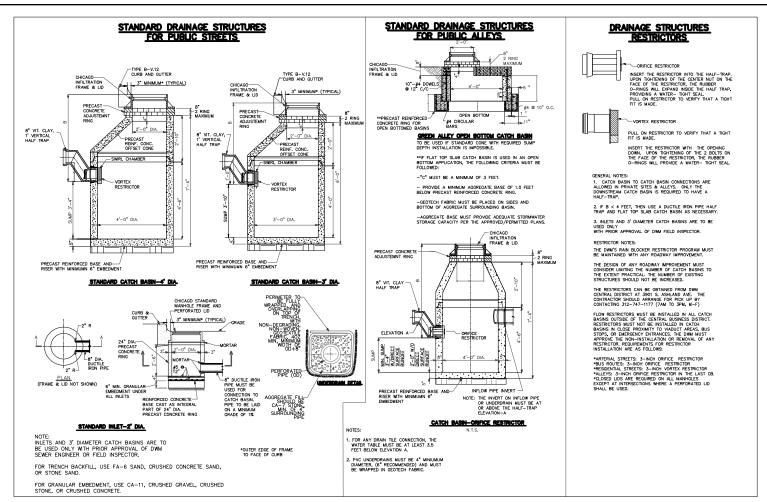
SHEET

7/19/2019 G:\S\3284 Chicago 2FM\01004 Carnotite\mxd\2019-07\C10-ErosionControlDetail.mxd m.banh

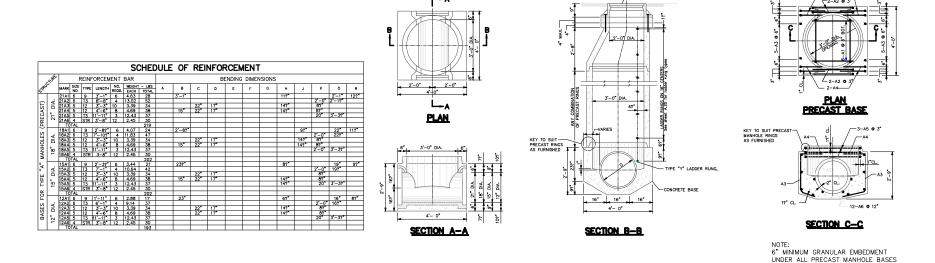








DRAINAGE STRUCTURE DETAILS



TYPE A PRECAST MANHOLE FOR SEWERS 21" DIA. AND SMALLER NOT TO SCALE

STORM AND SANITARY DETAILS ARE DOWNLOADED FROM THE CITY OF CHICAGO DEPARTMENT OF WATER MANAGEMENT BUREAU OF ENGINEERING SERVICES

C.NISSEN DRAWN: M.BANH PROJECT NO: 103S328401004 JULY 2019

DATE:



DEPARTMENT OF FLEET AND FACILITY MANAGEMENT 30 NORTH LASALLE ST. SUITE 300 CHICAGO, IL 60602 312 744 3900

TETRA TECH 1 SOUTH WACKER DR SUITE #3700 CHICAGO, IL 60606 312.201.7700

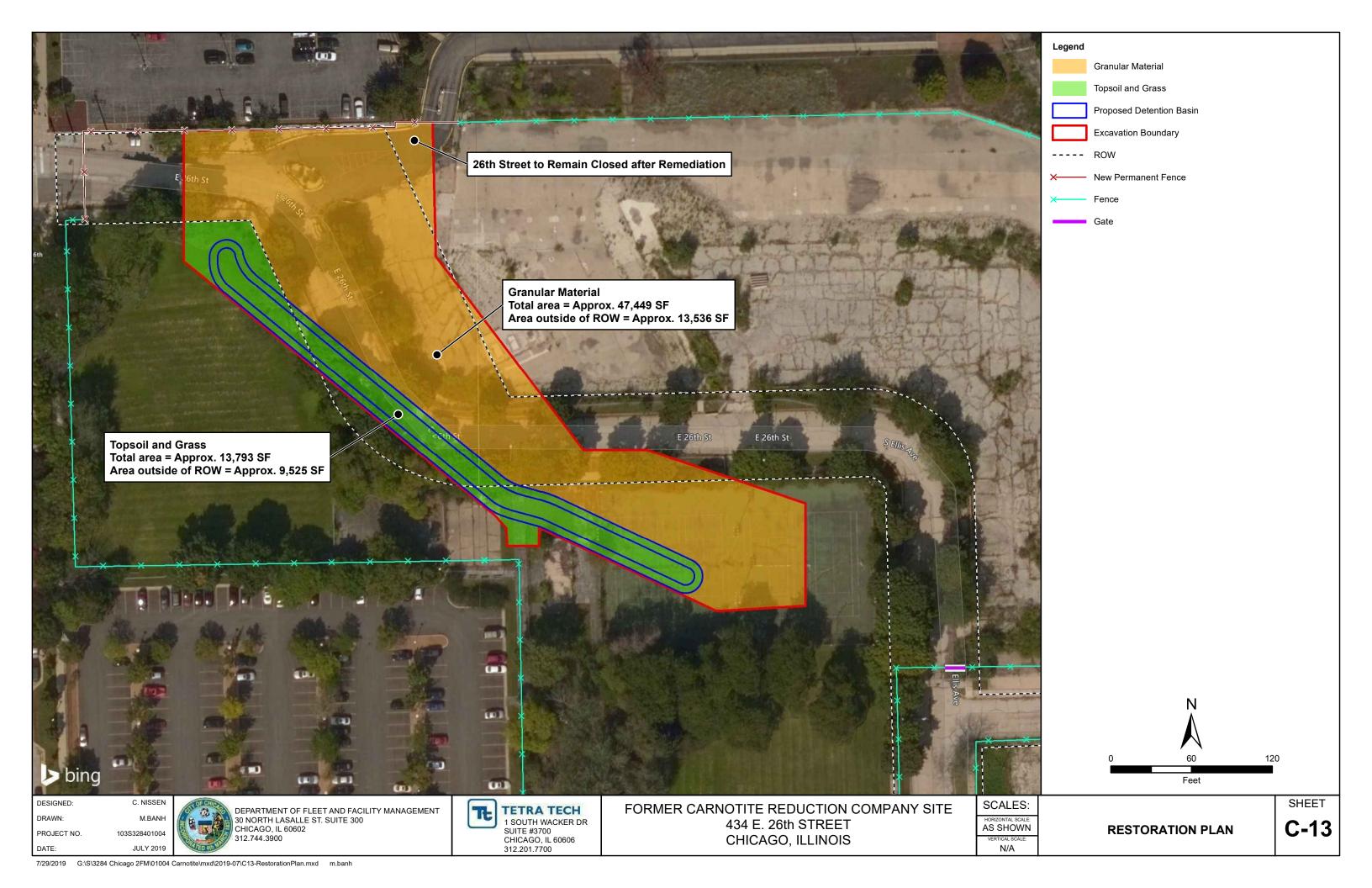
FORMER CARNOTITE REDUCTION COMPANY SITE 434 E. 26th STREET CHICAGO, ILLINOIS

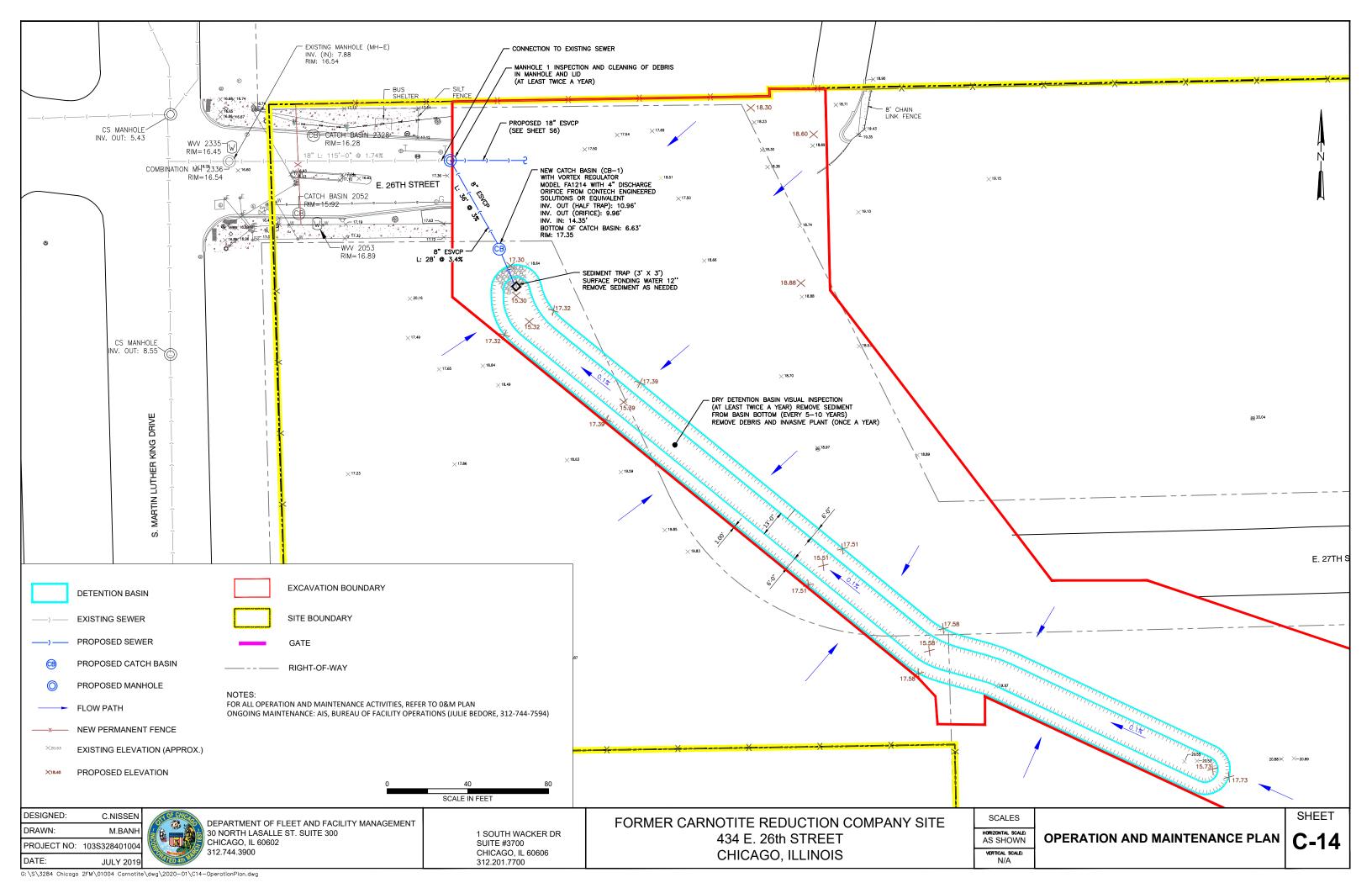
SCALES N/A VERTICAL SCALE: N/A

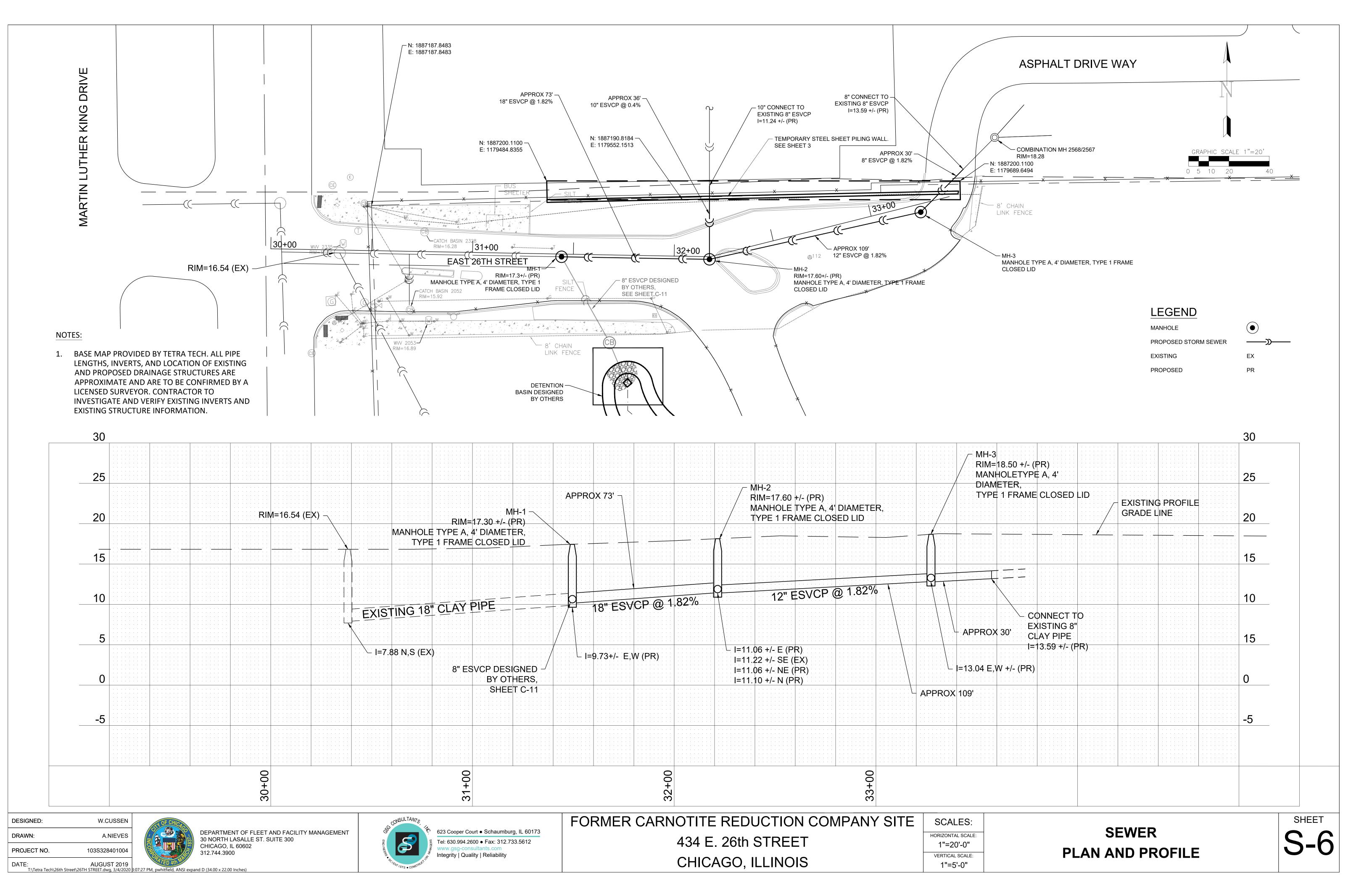
STORM SEWER DETAILS

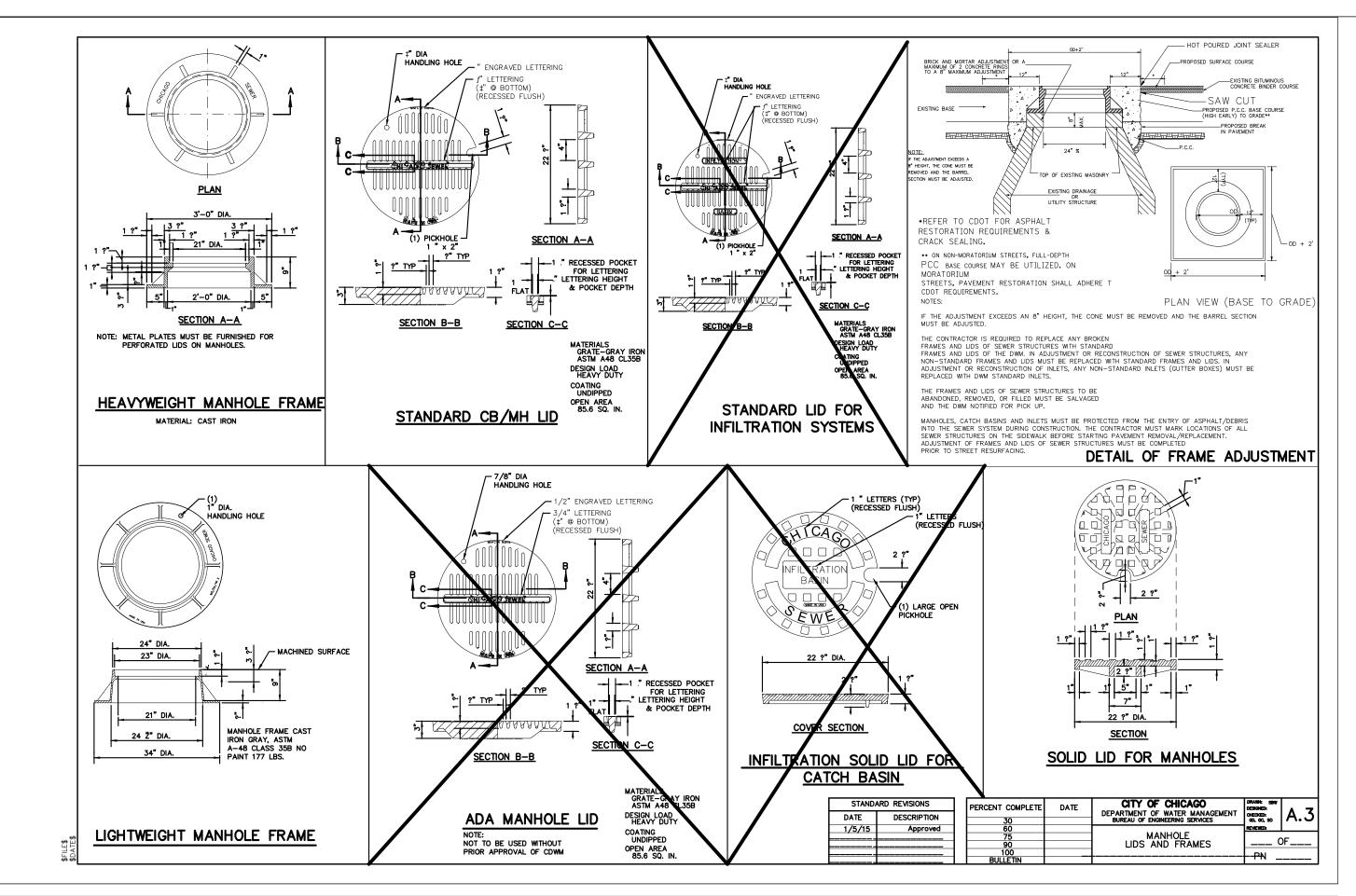
SHEET

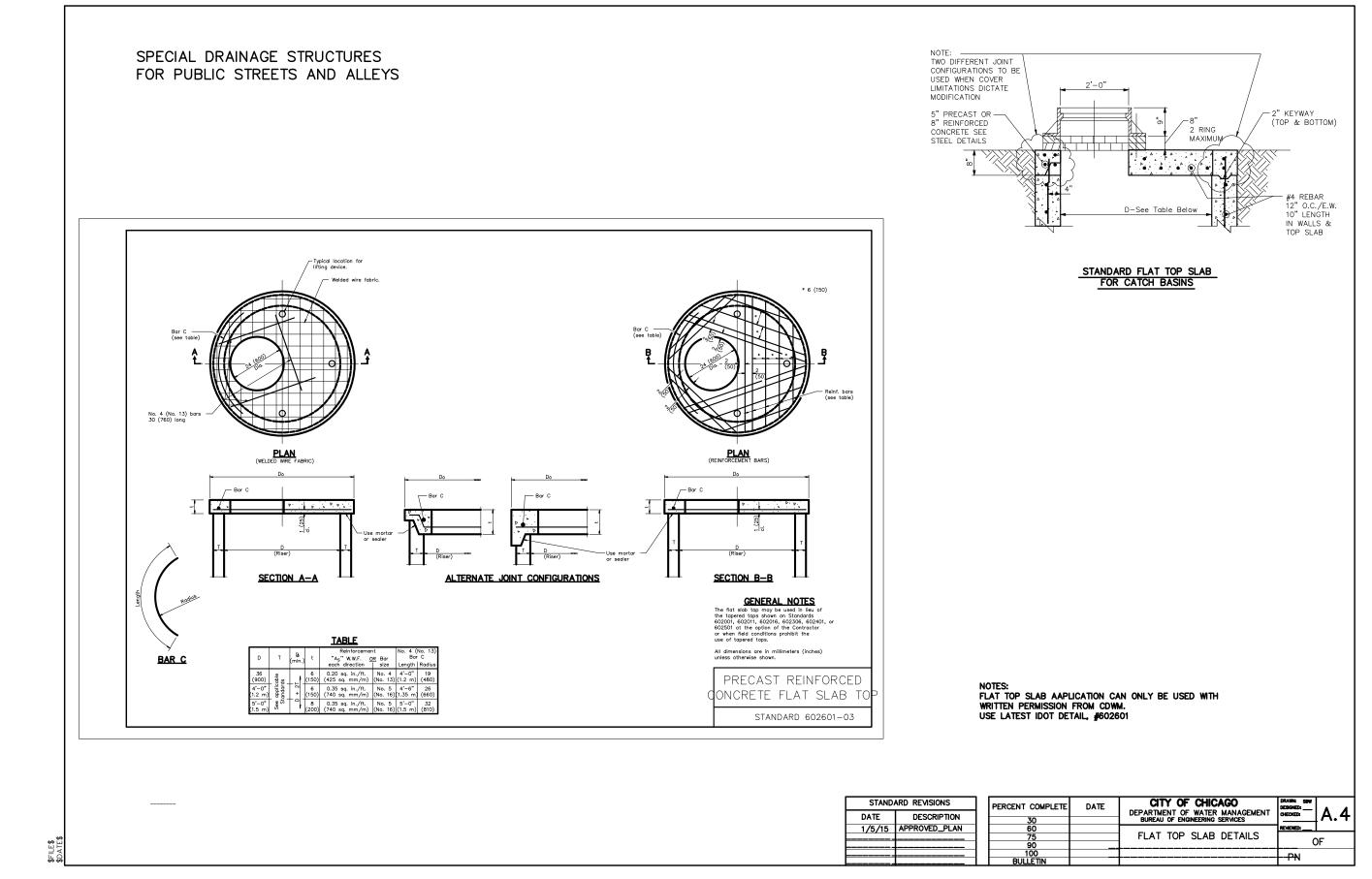
C-12

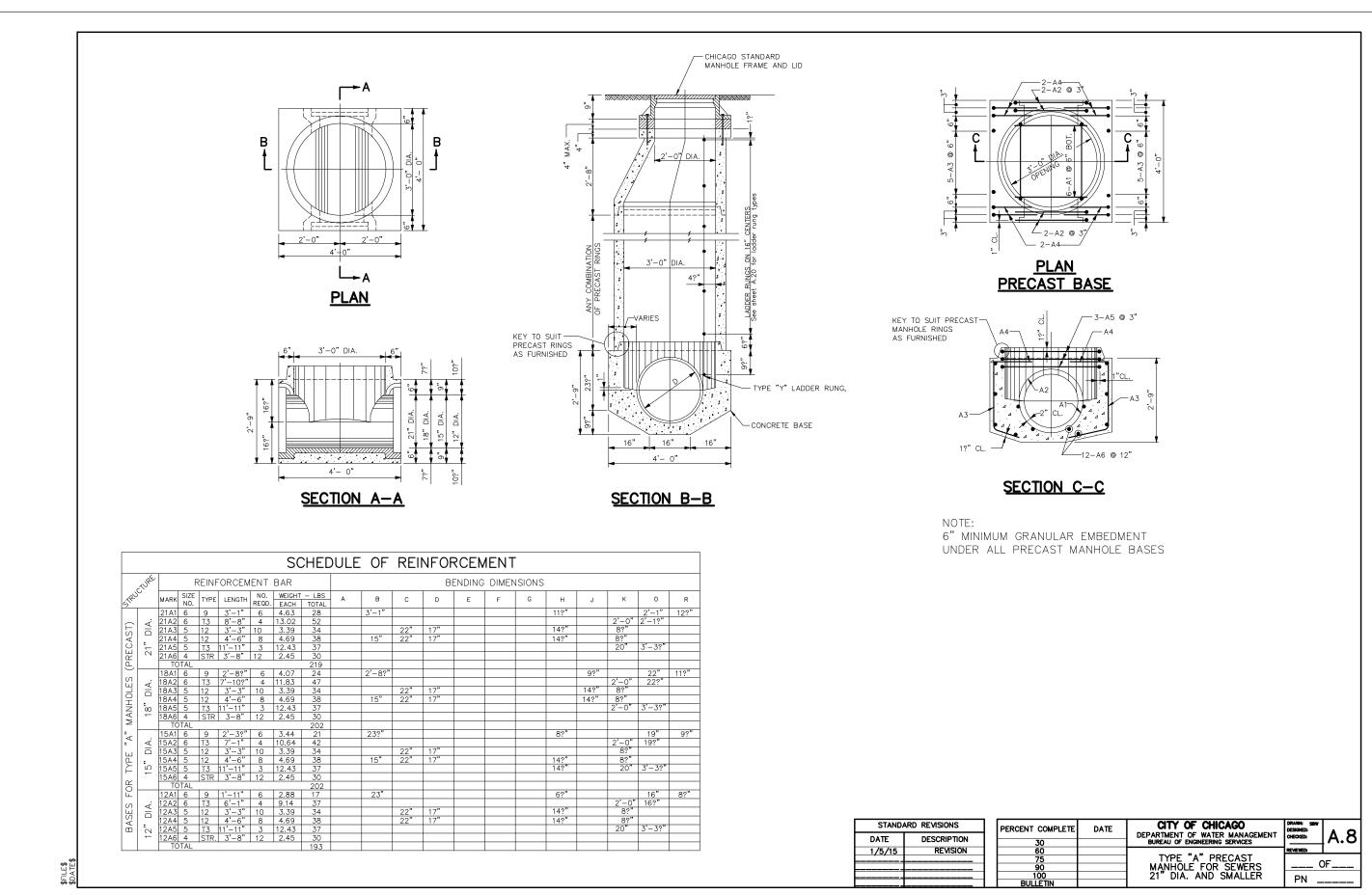


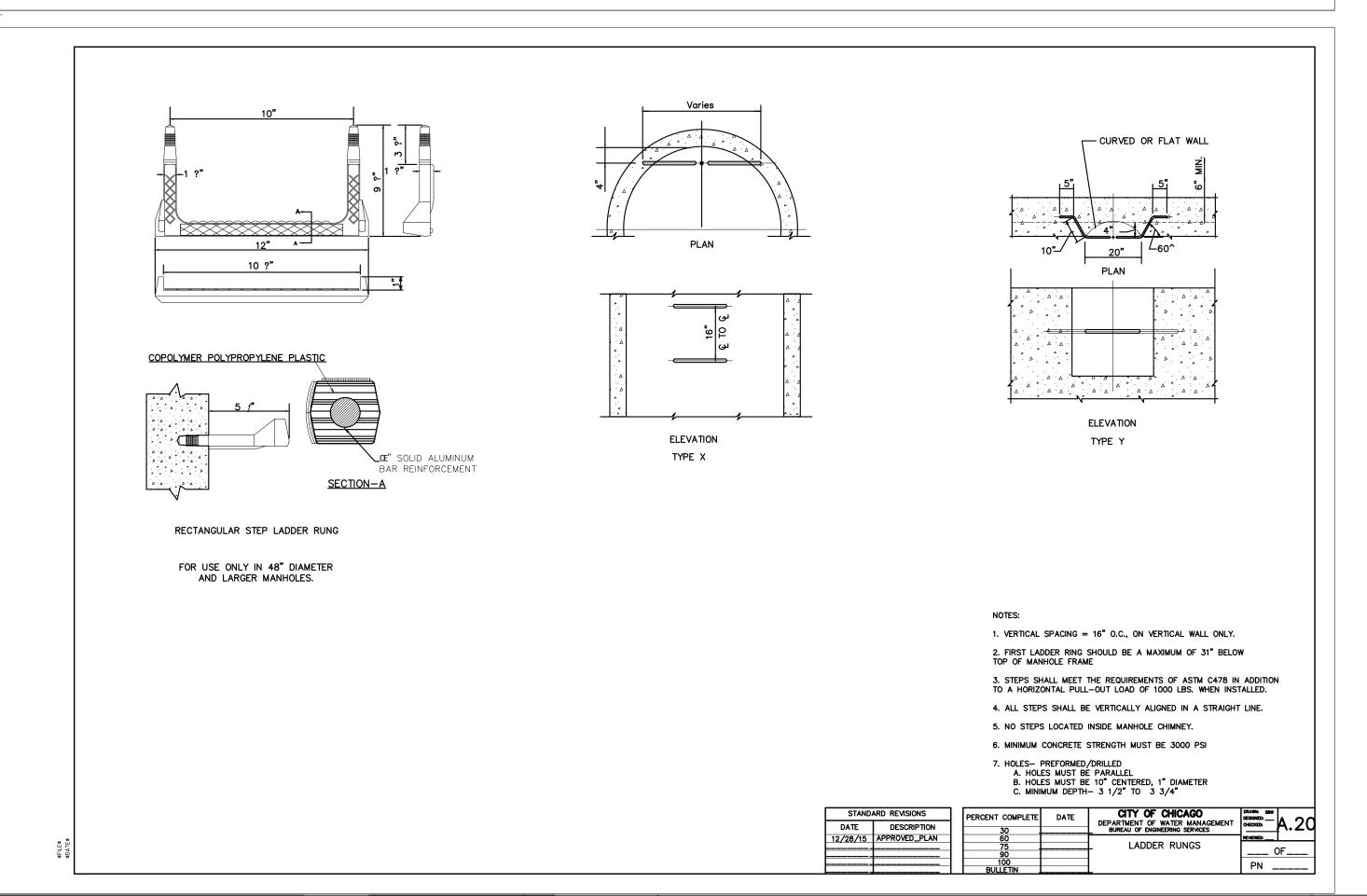


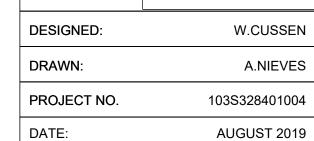












T.\Tetra Tech\26th Street\CITY OF CHICAGO DETAILS.dwg, 11/5/2019 2:11:18 PM, iscott, ANSI expand D (34.00 x 22.00 Inches)





FORMER CARNOTITE REDUCTION COMPANY SITE

434 E. 26th STREET CHICAGO, ILLINOIS

SCALES:	
HORIZONTAL SCALE:	
NTS	
VERTICAL SCALE:	
NTS	

CITY OF CHICAGO STANDARD DETAILS

$\frac{\textbf{ATTACHMENT 14} - \textbf{MEMORANDUM FROM THE CITY OF CHICAGO DEPARTMENT OF}}{\textbf{BUILDINGS}}$





DEPARTMENT OF BUILDINGSCITY OF CHICAGO

MEMORANDUM

DATE: 12/03/2019

TO: Kristine Schnoes, Tetra Tech

FROM: Andrew Billing, PE, CFM, Lead Stormwater Reviewer

Mackie Consultants, LLC

SUBJECT: Review of Design Plans, dated: **July 2019**

Project Name: Former Carnotite Reduction Company Site Remediation

Project Address: 434 E 26th St

Tracking Number: N/A

Designer/Developer: Tetra Tech

In response to your submittal of engineering plans and calculations for the above referenced project, we have the following review comments:

- 1. Attachment 7 Detention Basin Volume Calculations: Since the volume control component of the Stormwater Ordinance is proposed to be met using oversized detention, the detention basin must be sized to provide 14,641 cu ft (12,664 + 1,977), the rate control plus the volume control volumes.
- 2. Attachment 5 Stormwater Calculations: Add a cut sheet and rating curve for a custom vortex restrictor that will discharge 0.22 cfs under the proposed head conditions.
- 3. C-11 Grading and Storm Sewer Plan: Specify the manufacturer, model and size of the custom vortex restrictor.
- 4. The removal of public sewer and installation of public sewer and public MHs must be approved directly by DWM, Sewer Design Section. Please submit this design to Sid Osakada or provide evidence that you are already in the review process with them.
- 5. C-11 Grading and Storm Sewer Plan: Move CB-1 a few feet south outside the right-of-way.
- 6. C-14 O&M Plan: Add the owner's certification statement (DWM Standard Detail A.108) with owner's signature and notary.

- 7. C-14 O&M Plan: List the name and contact information for the individual responsible for ongoing maintenance following construction.
- 8. C-14 O&M Plan: Changes to the vortex restrictor and CB-1 location must be reflected on this plan. The custom vortex restrictor manufacturer, model and size must be clearly called out.
- 9. Provide the SESC Affidavit and Infiltration Affidavit when submitting final documents.
- 10. Revise and resubmit a hard copy of the entire plan set and calculations.

If you have any questions regarding these comments, please call me at 847-774-3821 (cell).

<u>ATTACHMENT 15 – EMAIL ADDRESSING COMMENT NO 1 BETWEEN TETRA TECH AND THE CITY OF CHICAGO DEPARTMENT OF BUILDINGS</u>



Couture, Marika

From: Andrew Billing <abilling@mackieconsult.com>

Sent: Friday, December 6, 2019 3:13 PM

To: Schnoes, Kris

Cc: Couture, Marika; Abigail Mazza **Subject:** Re: 434 E 26th St-Review

⚠ CAUTION: This email originated from an external sender. Verify the source before opening links or attachments. 🛕

Kris,

Thank you for clarifying the intent of your calculations in Attachment 7. I understand. Your detention basin is sized adequately to meet the Rate Control + Volume Control volumes together in the one basin. Please disregard my comment 1.

Andrew Billing, PE, CFM, LEED Green Associate Lead Stormwater Reviewer Mackie Consultants, LLC, consultant to: City of Chicago, Department of Buildings 847-774-3821 (cell)

From: Schnoes, Kris <Kris.Schnoes@tetratech.com> **Sent:** Thursday, December 5, 2019 11:19 AM **To:** Andrew Billing <abilling@mackieconsult.com>

Cc: Couture, Marika < Marika. Couture@tetratech.com>; Abigail Mazza < abby.mazza@cityofchicago.org>

Subject: RE: 434 E 26th St-Review

Andrew,

We have a question regarding your Comment #1: "Attachment 7 Detention Basin Volume Calculations: Since the volume control component of the Stormwater Ordinance is proposed to be met using oversized detention, the detention basin must be sized to provide 14,641 cu ft (12,664 + 1,977), the rate control plus the volume control volumes."

In Attachment 7, the total volume of the basin in section "TOTAL BASIN VOLUME" is 15,784 cubic feet. This includes the volume and the rate control volumes. This volume occurs at a height of 2 feet, where the basin is 25 feet long. The volume calculated in the section "RATE CONTROL VOLUME (100YR)" is to demonstrate that at an elevation of 23.32 feet (HWL line), we provide the rate control volume of 12,664 cubic feet. Therefore, we believe our basin volume meets requirements. If not, could you please clarify your comment? Alternately, we could participate in a call or meeting to discuss.

Thank you again for your assistance.

Kris

Kristine K Schnoes | Environmental Scientist

Direct (312) 201-7480 | Business (312) 201-7700 | Mobile (773) 759-8058 | Kris.Schnoes@tetratech.com

Tetra Tech | Complex World, Clear Solutions™

1 S. Wacker Dr., Suite 3700, Chicago, IL 60606 | tetratech.com







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From: Andrew Billing <abilling@mackieconsult.com>

Sent: Tuesday, December 03, 2019 10:14 AM **To:** Schnoes, Kris < Kris. Schnoes@tetratech.com>

Cc: Couture, Marika < Marika. Couture@tetratech.com>

Subject: 434 E 26th St-Review

⚠ CAUTION: This email originated from an external sender. Verify the source before opening links or attachments. ⚠

Kris/Marika,

Here are my comments on your project at 434 E 26th St.

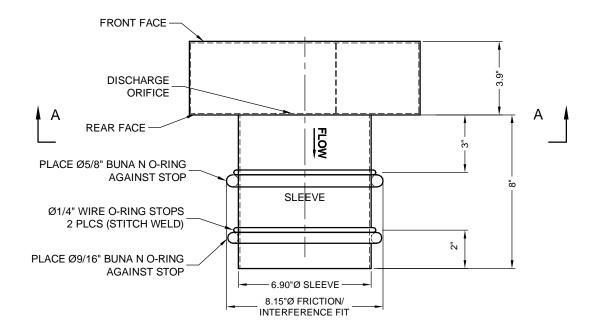
Please let me know if you have any questions.

Andrew Billing, PE, CFM, LEED Green Associate Lead Stormwater Reviewer Mackie Consultants, LLC, consultant to: City of Chicago, Department of Buildings 847-774-3821 (cell)

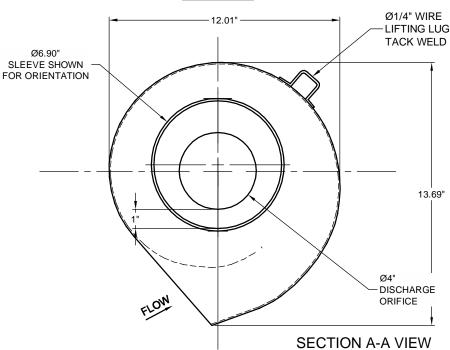
ATTACHMENT 16	VORTEY RECLUAT	OP DETAILS PATIN	IG CURVE AND CUT SHEET
ATTACHMENT 10-	VORTEA REGULAT	OR DETAILS, RATE	G CORVE AND COT SHEET

TETRA TECH

- 1. ALL WELDS CONTINUOUS, UNLESS OTHERWISE NOTED
- 2. MATERIALS:
- 1/8" ALUMINUM 5052
- (1) 5/8" AND (1) 9/16" BUNA N, 50 DUROMETER O-RINGS



TOP VIEW



This CADD file is for the purpose of specifying stormwater flow control equipment to be furnished by Contech Engineered Solutions LLC and may only be transferred to other documents exactly as provided by Contech Engineered Solutions LLC. Title block information, excluding the Contech Engineered Solutions LLC logo and the Fluidic-Amp or Fluidic-Cone designation and patent number may be deleted if necessary. Revisions to any part of this CADD file without prior coordination with Contech Engineered Solutions LLC shall be considered unauthorized use of proprietary information.



FABRICATION DRAWING FOR FLUIDIC-AMP VORTEX VALVE MODEL FA1214 WITH SLEEVE ATTACHMENT SIZED FOR 8" PIPE (8.0" ID) PROJECT NAME LOCATION

200 Enterprise Drive, Scarborough, ME 04074

77-907-8676 207-885-9830 207-885-9825 FAX

FAX DAT

DATE:01/09/2020 SCALE: 1:5

FILE NAME

DRAWN: NDC

CHECKED: NDC

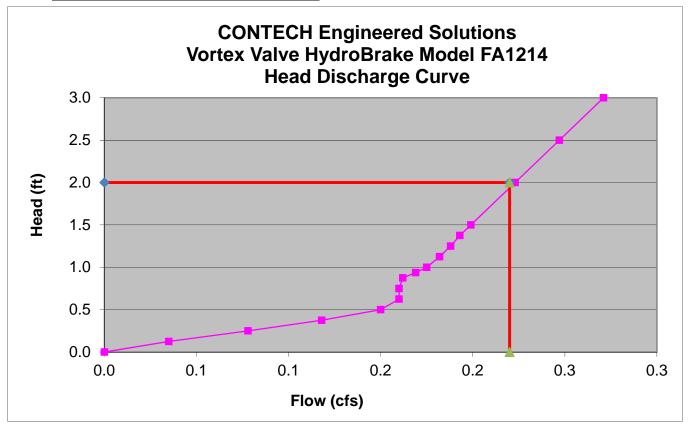
PPS/52 VORTEX VALVE/44 FABRICATION/CHICAGO ALUM 5052/DO NOT USE - STANDARD PIPE 8-10 INCH. DWG 1/9/2020 1:40 PM



Vortex Valve FA1214 with 4" Opening

(ft) (cfs) 0 0 0.125 0.035 0.25 0.078 0.375 0.118 0.50 0.150 0.625 0.160 0.875 0.162 0.938 0.169 1.00 0.175 1.125 0.182 1.375 0.193 1.50 0.199 2.00 0.223 2.50 0.247 3.00 0.271
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0.938 0.169 1.00 0.175 1.125 0.182 1.25 0.188 1.375 0.193 1.50 0.199 2.00 0.223 2.50 0.247
1.00 0.175 1.125 0.182 1.25 0.188 1.375 0.193 1.50 0.199 2.00 0.223 2.50 0.247
1.125 0.182 1.25 0.188 1.375 0.193 1.50 0.199 2.00 0.223 2.50 0.247
1.25 0.188 1.375 0.193 1.50 0.199 2.00 0.223 2.50 0.247
1.375 0.193 1.50 0.199 2.00 0.223 2.50 0.247
1.50 0.199 2.00 0.223 2.50 0.247
2.00 0.223 2.50 0.247
2.50 0.247
3.00 0.271
0.00 0.271
3.50 0.298
4.00 0.325
4.50 0.351
5.00 0.377
5.50 0.406
6.00 0.435
6.50 0.464
7.00 0.494
7.50 0.526
8.00 0.558
8.50 0.592
9.00 0.626
9.50 0.663
10.00 0.700

Target Head:	2 ft
Target Flow:	0.22 cfs
Achieved Flow:	0.22 cfs



 $\underline{\textbf{ATTACHMENT 17} - \textbf{EMAIL CONFIRMING THE SUBMITTAL TO DWM FOR REVIEW}}$

TETRA TECH

Schnoes, Kris

From: Abigail Mazza < Abby. Mazza@cityofchicago.org >

Sent: Tuesday, March 10, 2020 2:45 PM

To: Brendan Schreiber; Sid Osakada; Girley Abraham

Cc: Anupam Verma; Schnoes, Kris; Nissen, Carol; Kimberly Worthington

Subject: 434 E. 26th Street/Carnotite Sewer Drawing Review Request

Attachments: Carnotite Drawings for DWM 2020-03-10.pdf

CAUTION: This email originated from an external sender. Verify the source before opening links or attachments.

Brendan, Sid and Girley,

Attached are sewer drawings for the Carnotite project for your review and feedback ahead of the OUC submittal and as requested by Andrew Billings as part of our stormwater permit application. The drawings include replacement of the sewer within 26th Street in the "north area" impacted by our remedial excavation as well as the sewer lines coming in from the north property from both the north and northeast. These replacements are based on your review of the sewer inspection results summarized below and our own discussions with the north property owner.

Please let me know if you have any comments or concerns or would like to see any of the other sheets. We are finalizing the bid documents for advertisement so would like to get any requested revisions incorporated as soon as possible.

Thanks,

Abby Mazza, P.E. | Environmental Engineer III City of Chicago | Department of Assets, Information and Services (AIS) Bureau of Environmental, Health & Safety Management (EHS) 30 N. LaSalle St., Suite 300 | Chicago, Illinois 60602-2575

Tel: 312.744.3161 | Fax: 312.744.6451

From: Brendan Schreiber

Sent: Wednesday, August 07, 2019 4:19 PM

To: Abigail Mazza < Abby. Mazza@cityofchicago.org >

Cc: Nissen, Carol < Carol. Nissen@tetratech.com >; Sid Osakada < Sid. Osakada@cityofchicago.org >; Girley Abraham

<Girley.Abraham@cityofchicago.org>; Schnoes, Kris Kruk <Kris.Schnoes@tetratech.com>; Anupam Verma

<a href="mailto:<a href="mailto: Kimberly Worthington@cityofchicago.orgKimberly.Worthington@cityofchicago.orgKimberly.Worthington@cityofchicago.orgKimberly.Worthington@cityofchicago.org

Subject: RE: 434 E. 26th Street/Carnotite Sewer Specification and As-Built Drawing Request

Abby,

Attached are the microfilms we were able to track down. As indicated by the drawings, this appears to have been Michael Reese property at one point. Once you compare these to current parcel/property lines you may be able to shed more light on what is and is not connected to the sewer in 26th St and whether the property to the north would be impacted.

Based on the video inspection reports, the sewer designated by Michels as the "south area" can be abandoned and or removed up to MH G as designated by Michels. The "north area" sewer replacement limit will depend on the need of the sewer that has been identified as running east/northeast from MH B. in the Michels report. At a minimum, this sewer will need to be replaced up to the drain connection identified in the Michels report (MH B).