

CITY OF CHICAGO

LORI LIGHTFOOT, MAYOR

DEPARTMENT OF TRANSPORTATION
GIA BIAGI, COMMISSIONER

DIVISION OF ENGINEERING DANIEL BURKE, P.E., S.E., CHIEF ENGINEER

CONTRACT PLANS FOR

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH OF THE CHICAGO RIVER

FROM ASHLAND AVENUE TO DOMINICK STREET

PROJECT MAP

TRAFFIC DATA

ADT (CURRENT) = 7,650 ADT (2040) = 8,800 DESIGN YEAR = 2050

WSP (ISA Inc. 30 M. IASALE STREET SUITE 4200 C. 1500 C

END IMPROVEMENT WEBSTER AVENUE STA. 9+01.62

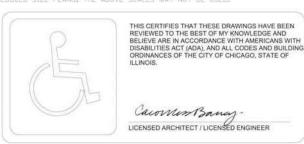
BEGIN IMPROVEMENT WEBSTER AVENUE STA. 15+24.78

C.D.O.T. PROJECT NO. E-1-525 SPECIFICATION NO. 1188838

SEE SHEET G-1 FOR INDEX OF SHEETS

0 100' 200' 300' 1"- 100'
0 50' 50' 1"- 50'
0 50' 100' 1'- 50'
0 50' 100' 1'- 50'
0 50' 100' 1'- 50'
0 50' 100' 1'- 50'

FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASURMENTS ON BEDUCED LISE BLANS THE ABOVE SCALES HAVE NOT BE USED.



NET LENGTH OF IMPROVEMENT: 623.16 FEET (0.118 MILES)

ISSUED BY: DEPARTMENT OF PROCUREMENT SERVICES SHANNON E. ANDREWS, CHIEF PROCUREMENT OFFICER

JAMAL GRANAWS

JAMAL



MOUSSA A. ISSA, S.E., P.E.
HBM ENGINEERING GROUP, LLC
LICENSE NO.: 081–005738
EXPIRES: November 30, 2020
DATE: April 01, 2020
SHEETS S-8 THRU S-35,
& S-94 THRU S-107



CAROL ROSS BARNEY, AIA
ROSS BARNEY ARCHITECTS
LICENSE NO.: 001-008045
EXPIRES: November 30, 2020
DATE: April 01, 2020
SHEETS A-0.0 THRU A-6.8



WSP USA INC.
LICENSE NO.: 062-068573
EXPIRES: November 30, 2021
DATE: 03/31/20
SHEETS G-1 THRU G-7,
C-1 THRU C-12 &
PMK-1 THRU PMK-3



SRIJAN ADHIKARI, P.E.
EJM ENGINEERING, INC.
LICENSE NO.: 062-069784
EXPIRES: November 30, 2021
DATE: 4 1 2020
SHEETS MOT-1 THRU MOT-4

JUDE OSEI BONSU. P.E.

LICENSE NO.: 062–069421 EXPIRES: November 30, 2021 DATE: 4 1 2020

SHEETS M-1 THRU M-8

WSP USA INC.

DEPARTMENT OF TRANSPORTATION

DATE: December 17

APPROVED: PROJECT MANAGER

APPROVED: CHIEF BIRDGE ENGINEER

APPROVED: CHIEF BIRDGE ENGINEER

DIVISION DE INNERBING

APPROVED: COMMUNICATION DE INNERBING

APPROVED: COMMUNICATION DE INNERBING

APPROVED: COMMUNICATION DE INNERBING

OF ESSION MCHAMED R. RASHED DG2-053645

Mohammed K Rashed

EJM ENGINEERING, INC.

DATE: 04/02/2020

SHEETS E-1 THRU E-13

LICENSE NO.: 062-053645

EXPIRES: November 30, 2021

MOHAMMED K. RASHED, P.E.

INDEX OF SHEETS

COVER SHEET

INDEX OF SHEETS

GENERAL NOTES - SHEET 1 OF 2

GENERAL NOTES - SHEET 2 OF 2

SUMMARY OF QUANTITIES - SHEET 1 OF 4

SUMMARY OF QUANTITIES - SHEET 2 OF 4

SUMMARY OF QUANTITIES - SHEET 3 OF 4

SUMMARY OF QUANTITIES - SHEET 4 OF 4

ALIGNMENT, TIES, AND BENCHMARKS

TYPICAL SECTIONS - SHEET 1 OF 2

TYPICAL SECTIONS - SHEET 2 OF 2

PLAN AND PROFILE - SHEET 1 OF 2

PLAN AND PROFILE - SHEET 2 OF 2

SEDIMENT CONTROL SILT CURTAIN

ADA RAMPS DETAILS - SHEET 1 OF 2

ADA RAMPS DETAILS - SHEET 2 OF 2

EROSION AND SEDIMENT CONTROL PLAN

ROADWAY REMOVAL PLAN

UTILITY PLAN

C-12 ROADWAY DETAIL

MAINTENANCE OF TRAFFIC

MOT-2 DETOUR PLAN

PAVEMENT MARKINGS

THRU THRU

STRUCTURAL

S-1

S-3

S-4

S-5

S-7

S**-**8

S-10 S-11

S-12

S-14 S-15

S-17 S-18

S-19

S-21

S-22

S-23

S-24 S-25

S-26

S-27

S-28

MOT-1 DETOUR GENERAL NOTES

HIGHWAY STANDARD DETAILS

GENERAL PLAN

MOT-3 TRAFFIC SIGNAL TIMING SCHEDULES

SD-1 STANDARD DETAILS - SHEET 1 OF 16

SD-16 STANDARD DETAILS - SHEET 16 OF 16

PMK-1 PAVEMENT MARKING AND SIGNING - SHEET 1 OF 2

PMK-2 PAVEMENT MARKING AND SIGNING - SHEET 2 OF 2

EXISTING PLANS AND REMOVAL - FIXED SPANS

EXISTING PLANS AND REMOVAL - BASCULE SPAN

TOP OF SLAB ELEVATIONS: WEST FIXED SPANS I

TOP OF SLAB ELEVATIONS: WEST FIXED SPANS II

TOP OF SLAB ELEVATIONS: EAST FIXED SPANS I

TOP OF SLAB ELEVATIONS: EAST FIXED SPANS II

TOP OF SLAB ELEVATIONS: EAST APPROACH SLAB

SUPERSTRUCTURE DETAILS - WEST FIXED SPANS

SUPERSTRUCTURE PLAN - EAST FIXED SPANS

SUPERSTRUCTURE PLAN GEOMETRICS - WEST FIXED SPANS

PARAPET ELEVATIONS AND DETAILS - WEST FIXED SPANS

SUPERSTRUCTURE PLAN GEOMETRICS - EAST FIXED SPANS

PARAPET ELEVATIONS AND DETAILS - EAST FIXED SPANS

SUPERSTRUCTURE PLAN - WEST FIXED SPANS

CROSS SECTIONS - WEST FIXED SPANS

CROSS SECTIONS - EAST FIXED SPANS

DIAPHRAGM DETAILS - EAST FIXED SPANS

DIAPHRAGM DETAILS - WEST FIXED SPANS

GENERAL DATA I (GENERAL NOTES & INDEX OF SHEETS)

GENERAL DATA II (SCOPE OF WORK & TOTAL BILL OF MATERIAL)

PMK-3 PAVEMENT MARKING AND SIGNING SCHEDULES

RETAINING WALL PLANS & DETAILS

MOT-4 MAINTENANCE OF TRAFFIC DETAILS

GENERAL

G-0

G-1

G-3

G-4

G-5

G-6

G-7

C-1

C-2

C-3

C-4

C-5

C-6

C-7

C-8

C**-**9

C-10

C-11

	S-29 S-30 S-31	FIXED S FIXED S FIXED S
151	SUITE 4: CHICAGO TEL: (31	ASALLE STREET

		.,				
SUPERS	TRUCTURE DETAILS - EAST FIXE	D SPANS			S-95	V
EAST AF	PPROACH SLAB				S-96	E.
EAST AF	PPROACH SLAB DETAILS				S-97	E.
FRAMING	G PLAN - WEST FIXED SPANS				S-98	W
FRAMING	G PLAN - EAST FIXED SPANS				S-99	E
FIXED S	PANS - STRINGER MOMENT ANI	O REACTION TABLE			S-100	ΡI
FIXED S	PANS STEEL DETAILS I				S-101	W
FIXED S	PANS STEEL DETAILS II				S-102	E
USA Inc.	USER NAME = PJLAUX	DESIGNED - PJL	REVISED	-		
LASALLE STREET 4200		CHECKED - IJL	REVISED	-		
GO, IL 60602 (312) 782-8150	PLOT SCALE =	DRAWN - PJL	REVISED	-		
(312) 782-1684	PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED	-		

STRUCTURAL (CONTINUED)

BEARING DETAILS

S-33

S-35

S-36

S-37

S-38

S-39

5-40

S-42

S-43

S-44

S-45

S-46

S-47

5-49

S-50

S-51

S-52

S-53

S-54

S-55

S-56

S-57

S-58

S-59 S-60

S-61

S-62 S-63

S-64 S-65

S-66

S-67

S-68

S-69 S-70

S-71

5-72

S-73

S-74

S-75

S-76

S-77

S-78

S-79

S-80

S-81

S-82

S-83 S-84

S-85

S-86

S-87

S-88

S-89

S-90

S-91

S-92

S-93

S-94

FIXED SPANS STEEL DETAILS III

BASCULE SPAN: DECK PLAN BASCULE SPAN: DECK DETAILS

BASCULE SPAN: CURB DETAILS

BASCULE SPAN: SIDEWALK PLAN BASCULE SPAN: SIDEWALK DETAILS

BASCULE SPAN: FRAMING PLAN

BASCULE SPAN: FLOORBEAM 0-0

BASCULE SPAN: FLOORBEAM DETAILS I

BASCULE SPAN: FLOORBEAM DETAILS II BASCULE SPAN: FLOORBEAM DETAILS III

BASCULE SPAN: FLOORBEAM 10-10

BASCULE SPAN: SW TRUSS REPAIRS

BASCULE SPAN: CENTER BREAK DETAILS

BASCULE SPAN: LOWER LATERAL BRACING I

BASCULE SPAN: LOWER LATERAL BRACING II

BASCULE SPAN: FLOORBEAM 10-10 DETAILS

BASCULE SPAN: FLOORBEAM 2-2, 4-4, 6-6 & 8-8

BASCULE SPAN: REAR BREAK DETAILS

FIXED SPANS SLOT RAILING DETAILS

FIXED SPANS ANCHOR COLUMN FLOORBEAM DETAILS

BASCULE SPAN: GUSSETS FOR LOWER LATERAL BRACING I BASCULE SPAN: GUSSETS FOR LOWER LATERAL BRACING II

STRUCT	TURAL (CONTINUED)
S-103	NORTHEAST RETAINING WALL PLAN AND ELEVATION I
S-104	NORTHEAST RETAINING WALL PLAN AND ELEVATION II
S-105	SOUTHEAST RETAINING WALL PLAN AND ELEVATION I
S-106	SOUTHEAST RETAINING WALL PLAN AND ELEVATION II
S-107	SOUTHEAST RETAINING WALL PLAN AND ELEVATION III
S-108	EAST RETAINING WALLS RAILING DETAILS
S-109	STEEL RAILING DETAILS
S-110	DOLPHINS AND PIER PROTECTION I
S-111	DOLPHINS AND PIER PROTECTION II
S-112	BORING LOGS I
S-113	BORING LOGS II
ARCHIT	ECTURAL
A-0.0	ARCHITECTURAL GENERAL NOTES
A-1.1	OVERALL SITE PLAN
A-1.2	BRIDGE HOUSE PLAN
A-1.3	BRIDGE HOUSE PLAN
A-1.4	BRIDGE HOUSE STAIR PLAN, SECTION, DETAIL
A-2.1	SOUTHEAST BRIDGE HOUSE ELEVATION
A-2.2	SOUTHEAST BRIDGE HOUSE ELEVATION
A-2.3	NORTHWEST BRIDGE HOUSE ELEVATION
A-2.4	SOUTHWEST & NORTHEAST BRIDGE WALL ELEVATION
A-3.1	BRIDGE HOUSE SECTION
A-3.2	BRIDGE HOUSE WALL SECTION
A-3.3	BRIDGE HOUSE DETAIL
A-3.4	BRIDGE HOUSE DETAIL
A-3.5	BRIDGE HOUSE DETAIL
A-3.6	BRIDGE HOUSE DETAIL
A-3.7	LOWER WALL SECTION
A-3.8	BRIDGE HOUSE DOOR SCHEDULE AND DETAIL
A-3.9	BRIDGE HOUSE ROOF FRAMING AXONOMETRIC VIEW
A-4.1	NORTHEAST RAILING
A-4.2	SOUTHEAST RAILING
A-4.3	SOUTHWEST RAILING
A-4.4	NORTHWEST RAILING
A-4.5	PRECAST CONCRETE RAILING DETAIL
A-4.6	PRECAST CONCRETE RAILING DETAIL
A-4.7	PRECAST CONCRETE RAILING AXONOMETRIC VIEW
A-6.1	BRIDGE HOUSE AXONOMETRIC VIEW
A-6.2	BRIDGE HOUSE PRECAST CONCRETE PANEL
A-6.3	BRIDGE HOUSE PRECAST CONCRETE PANEL
A-6.4	BRIDGE HOUSE PRECAST CONCRETE PANEL
A-6.5	BRIDGE HOUSE PRECAST CONCRETE PANEL
A-6.6	BRIDGE HOUSE PRECAST CONCRETE PANEL
A-6.7 A-6.8	PRECAST CONCRETE RAILING PYLON AT BRIDGE HOUSE PRECAST CONCRETE RAILING PYLON AT BRIDGE HOUSE
ELECTR E-1	STANDARD CODE FOR TRAFFIC SIGNALS & STREET LIGHTIN
E-2	ROADWAY FOUNDATION & UNDERGROUND CONDUIT PLAN
E-3	ROADWAY FOUNDATION & UNDERGROUND CONDUIT PLAN
E-4	ROADWAY LIGHTING INSTALATION PLAN
E-5	ROADWAY LIGHTING REMOVAL PLAN
E-6	NAVIGATIONAL LIGHTING INSTALLATION PLAN
E-7	NAVIGATIONAL LIGHTING REMOVAL PLAN
E-8	ELECTRICAL DETAILS
E-9	ELECTRICAL DETAILS
E-10	ELECTRICAL DETAILS
E-11	ELECTRICAL DETAILS
E-12	ELECTRICAL DETAILS
E-13	ELECTRICAL DETAILS
МЕСНА	NICAL
M-1	GENERAL MACHINERY NOTES
M-2	GENERAL PLAN AND ELEVATION
M-3	EXISTING CENTER LOCK MACHINERY ASSEMBLY
M-4	NEW CENTER LOCK ASSEMBLY
M - 5	NEW CENTER LOCK DETAILS 1
B 4 C	NEW CENTED LOCK DETAILS 2

EBSTER AVENUE BRIDGE OVER	
NORTH BRANCH CHICAGO RIVER	

M-6

NEW CENTER LOCK DETAILS 2 NEW CENTER LOCK DETAILS 3 NEW SUMP PUMP ASSEMBLY

GENERAL NOTES

- 1 DUE TO THE POSSIBILITY OF LINDOCLIMENTED MODIFICATIONS AND CHANGES TO THE STRUCTURE, IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THE CORRECTNESS OF ALL REFERENCE DRAWINGS, PLAN DIMENSIONS AND DETAILS RELATIVE TO EXISTING STRUCTURES HAVE BEEN TAKEN FROM EXISTING PLANS AND ARE SUBJECT TO NOMINAL CONSTRUCTION VARIATIONS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY SUCH DIMENSIONS AND DETAILS IN THE FIELD AND MAKE NECESSARY APPROVED ADJUSTMENTS PRIOR TO CONSTRUCTION OR FABRICATION AND ORDERING OF MATERIALS. SUCH VARIATIONS ARE NOT CAUSE FOR ADDITIONAL COMPENSATION OR A CHANGE IN THE SCOPE OF WORK. HOWEVER, THE CONTRACTOR WILL BE PAID FOR THE QUANTITY ACTUALLY FURNISHED AT THE UNIT PRICE BID FOR THE WORK. CONTRACTOR SHALL NOT BE RELIEVED OF PERFORMING DETAILED SURVEY AS PART OF THIS WORK AND MAY REQUEST FOR DOCUMENTATION, IF ANY, OF PREVIOUS REPAIRS PERFORMED SINCE PREPARATION OF THESE DOCUMENTS.
- ALL CONSTRUCTION MATERIALS AND CONSTRUCTION ACTIVITIES USED ON THIS CONTRACT MUST CONFORM TO THE STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION ADOPTED APRIL 1, 2016 BY THE ILLINOIS DEPARTMENT OF TRANSPORTATION, SPECIFIED SUPPLEMENTAL SPECIFICATIONS AND RECURRING SPECIAL PROVISIONS, DATED JANUARY 1, 2019 AND PROJECT DETAIL SPECIFICATIONS EXCEPT AS MODIFIED HEREIN.
- 3. THE CONTRACTOR SHOULD EXPECT THAT DUE TO THE NATURE OF THIS REHABILITATION PROJECT, THE EXACT EXTENT OF REPAIR WORK CANNOT ALWAYS BE ACCURATELY DETERMINED PRIOR TO COMMENCEMENT OF WORK. PRE-BID VISITS AND ACCESS TO THE SITE SHALL BE REQUIRED AND CAN BE ARRANGED THROUGH THE CHICAGO DEPARTMENT OF TRANSPORTATION, DIVISION OF ENGINEERING.
- 4. ALL AVAILABLE EXISTING PLANS WILL BE PROVIDED TO THE CONTRACTOR IN ELECTRONIC FORMAT AND WILL BE AVAILABLE AT CITY HALL ON THE DAY OF BID ADVERTISEMENT. THE CONTRACT PLANS REFERENCE THE APPLICABLE EXISTING PLAN FILE THAT IS USUALLY LOCATED IN THE BOTTOM RIGHT HAND CORNER OF THESE CONTRACT DOCUMENTS.
- 5. THE CONTRACTOR MUST PROTECT AND CAREFULLY PRESERVE ALL PERMANENT SURVEY MONUMENTS, BENCHMARKS OR PROPERTY MARKERS WHICH ARE ENCOUNTERED DURING THE COURSE OF HIS/HER WORK. IF IT IS NECESSARY TO REMOVE ANY SUCH MONUMENT OR MARKER, THE COMMISSIONER MUST BE NOTIFIED PRIOR TO REMOVAL. THE CONTRACTOR MUST PROTECT AND CAREFULLY PRESERVE ALL PROPERTY MARKERS AND MONUMENTS UNTIL THE COMMISSIONER, AN AUTHORIZED SURVEYOR, OR AGENT HAS WITNESSED OR OTHERWISE REFERENCED THEIR LOCATION. ANY SUCH MONUMENT OR MARKER WHICH HAS BEEN DISTURBED BY THE CONTRACTOR MUST BE RESET BY A REGISTERED PROFESSIONAL LAND SURVEYOR TO THE SATISFACTION OF THE COMMISSIONER AND AT NO ADDITIONAL EXPENSE TO THE CITY.
- 6. THE CONTRACTOR MUST TAKE ALL NECESSARY SAFETY PRECAUTIONS TO PROTECT ABUTTING PROPERTY, PEDESTRIANS, AND RIVER AND VEHICULAR TRAFFIC. THE CONTRACTOR MUST TAKE PRECAUTIONS TO PROTECT THE PUBLIC FROM FALLING DEBRIS. THE CONTRACTOR MUST SUBMIT PLANS FOR PEDESTRIAN, RIVER, AND VEHICULAR PROTECTION TO THE COMMISSIONER FOR APPROVAL PRIOR TO BEGINNING THE WORK. IF ANY DEBRIS FALLS INTO THE WATERWAY, THE CONTRACTOR SHALL REMOVE IT FROM THE RIVER TO THE COMMISSIONER'S SATISFACTION. AND AT NO ADDITIONAL COST TO THE CITY.
- 7. ALL TEMPORARY ITEMS INSTALLED WITHIN THE WATERWAY MUST BE COMPLETELY REMOVED. WORK BARGE OR ANY FLOATING WORK PLATFORM SHALL NOT REMAIN MOORED BENEATH DRAW OVERNIGHT OR WHILE WORK IS SUSPENDED.
- THE CONTRACTOR MUST CALL DIGGER (CHICAGO UTILITY ALERT NETWORK) AT 312-744-7000 TO HAVE THE LOCATION OF EXISTING UNDERGROUND UTILITIES STAKED, 48 HOURS BEFORE STARTING FXCAVATION WORK
- 9. THE CONTRACTOR IS RESPONSIBLE FOR THE PROTECTION OF ALL UNDERGROUND AND OVERHEAD UTILITIES WITHIN THE PROJECT LIMITS WHETHER OR NOT THE UTILITIES ARE SHOWN ON THE PLANS. THE CONTRACTOR MUST INVESTIGATE THE WORK ZONE TO IDENTIFY LOCATIONS OF SIGNS, POLES, OR OTHER STRUCTURES WHICH WOULD IMPACT THE PROPOSED WORK LOCATIONS. COST ASSOCIATED WITH PROTECTION OF EXISTING UTILITIES IS INCLUDED IN THE COST OF "MOBILIZATION". ANY UTILITY THAT IS DAMAGED DURING CONSTRUCTION MUST BE REPAIRED OR REPLACED BY THE CONTRACTOR AT HIS/HER OWN EXPENSE TO THE SATISFACTION OF THE COMMISSIONER.

GENERAL NOTES (CONTINUED)

- 10. ANY ADJUSTMENT REQUIRED TO EXISTING UTILITIES WILL BE CARRIED OUT BY THE PRIVATE UTILITY COMPANIES OR THEIR CONTRACTORS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING WITH EACH UTILITY COMPANY. COST ASSOCIATED WITH COORDINATING WITH UTILITY COMPANIES IS INCLUDED IN THE COST OF "MOBILIZATION".
- 11. WHEN ARTIFICIAL LIGHTING IS USED IN NIGHT OPERATIONS THE CONTRACTOR SHALL EXERCISE UTMOST PRECAUTIONS IN PREVENTING ADVERSE VISIBILITY TO THE MOTORING PUBLIC, MARINE TRAFFIC ON THE RIVER, AND ADJOINING COMMERCIAL AND RESIDENTIAL AREAS IN ACCORDANCE WITH THE LATEST EDITION OF THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS (MUTCD) AND THE ILLINOIS SUPPLEMENT TO THE NATIONAL MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES.
- 12. ALL ELEVATIONS SHOWN ON THE PLANS ARE REFERENCED TO THE CHICAGO CITY DATUM: 579.48 FEET ABOVE MEAN SEA LEVEL, 1929
- 13. DO NOT SCALE PLANS.
- 14. FOR ADDITIONAL GENERAL NOTES, SEE SHEETS MOT-1, S-3, A-1, E-1, LT-1, AND M-1.
- 15. ALL DIMENSIONS SHOWN ON THE ROADWAY PLANS ARE TO THE FACE OF THE CURB, UNLESS OTHERWISE SPECIFIED.
- 16. THE ENTIRE AREA WHICH IS TO RECEIVE 'BITUMINOUS MATERIAL PRIME COAT' SHALL BE SWEPT CLEAN BEFORE THE MATERIAL APPLICATION. SWEEPING SHALL NOT BE DEPOSITED IN THE GUTTER OR ON THE CURB, PARKWAY, OR SIDEWALK, BUT SHALL BE PICKED UP AND DISPOSED OF PROPERLY BEYOND THE LIMITS OF THE PROJECT ON THE SAME DAY THAT SWEEPING IS DONE. THIS WORK SHALL BE INCLUDED IN THE COST OF BITUMINOUS MATERIAL (PRIME COAT) AND NO ADDITIONAL COMPENSATION SHALL BE ALLOWED.
- 17. SAW CUT (FULL DEPTH) SHALL BE REQUIRED AT THE JOINT BETWEEN PAVEMENT, SIDEWALK, CURB AND GUTTER TO BE REMOVED AND THE LEFT IN PLACE OR AS DIRECTED BY THE COMMISSIONER. THIS WORK WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE COST OF THE REMOVAL ITEMS.
- 18. THE CONTRACTOR SHALL MAKE HIS OWN INVESTIGATION TO DETERMINE THE EXISTENCE, NATURE AND EXACT LOCATION OF ALL UTILITY LINES AND APPURTENANCES WITHIN THE LIMITS OF IMPROVEMENT. THE COST OF THIS WORK WILL BE CONSIDERED INCIDENTAL TO THE CONTRACT.
- 19. ONE-HALF INCH (1/2") THICK EXPANSION JOINTS SHALL BE PLACED BETWEEN THE SIDEWALK, AND ALL STRUCTURES SUCH AS LIGHT STANDARDS, TRAFFIC LIGHT STANDARDS AND MANHOLES WHICH EXTEND THROUGH THE SIDEWALK IN ACCORDANCE WITH SECTION 424.07 OF THE STANDARD SPECIFICATIONS
- 20. CONSTRUCTION OF THE ADA RAMPS MUST MEET ALL CRITERIA SET FORTH IN THE STANDARDS. ALL SIDEWALK RAMPS CONTAINING TILES AND RAMP FLARES SHALL BE 8" THICK UNLESS OTHERWISE NOTED. ALL LANDING AREAS (KEYSTONE), RAMPS WITHOUT TILES, AND TRANSITION PANEL SHALL BE 8" THICK UNLESS OTHERWISE NOTED.
- 21. THERMOPLASTIC PAVEMENT MARKING LINE AND/OR LETTERS AND SYMBOLS SHALL BE DONE AT EACH LOCATION WHERE PAVEMENT MARKINGS ARE REQUIRED WITHIN 3 WORKING DAYS AFTER FINAL BITUMINOUS SURFACE IS IN PLACE.
- 22. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE STORAGE OF EXISTING SIGN PANELS AND POLE ASSEMBLIES WHICH ARE TO BE SALVAGED AND REINSTALLED UNDER PAY ITEM REMOVE, STORE AND RE-ERECT SIGN PANEL. THE CONTRACTOR SHALL BE RESPONSIBLE TO REPLACE, AT NO ADDITIONAL COST TO THE CONTRACT, ANY SIGNS AND/OR POLE ASSEMBLIES DAMAGED DURING STORAGE AND/OR DURING THE TRANSPORTATION TO AND FROM THE STORAGE LOCATIONS.
- 23. THE LATEST REVISION NUMBER OF THE IDOT HIGHWAY STANDARD AT THE TIME OF LETTING SHALL APPLY TO THIS CONTRACT.
- 24. A PERMIT IS REQUIRED FROM THE DEPARTMENT OF WATER MANAGEMENT PRIOR TO THE CONSTRUCTION OF, OR REPAIR TO UNDERGROUND SEWERS, DRAIN CONNECTIONS OR SEWER STRUCTURES, INCLUDING ADJUSTMENT OF SEWER STRUCTURES AND REMOVAL/REPLACEMENT OF FRAMES AND LIDS. THE PERMIT MUST BE OBTAINED BY A DRAINLAYER CURRENTLY LICENSED BY THE DEPARTMENT OF SEWERS.

GENERAL NOTES (CONTINUED)

- 25. THE CITY DOES NOT GUARANTEE THE COMPLETENESS OR ACCURACY OF THE INFORMATION SHOWN ON THE PLANS REGARDING UTILITIES, EITHER PUBLIC OR PRIVATE, SUCH AS SEWERS, MANHOLES, CATCH BASINS, GAS AND WATER MAINS, TELEPHONE AND ELECTRICAL DUCT LINES AND SIMILAR STRUCTURES. THE CONTRACTOR SHALL VERIFY THE EXACT LOCATION OF ALL UTILITIES THAT MAY INTERFERE WITH CONSTRUCTION OPERATIONS, AND SHALL REPORT TO THE COMMISSIONER ANY OMISSIONS AND DIFFERENCES FROM THE LOCATIONS SHOWN ON THE PLANS. THE COST OF THIS WORK WILL BE CONSIDERED INCLUDED IN THE COST OF
- 26. THE CONTRACTOR SHALL MAINTAIN THE SURFACE DRAINAGE OF THE ROAD DURING CONSTRUCTION OF THIS PROJECT.
- 27. PRIOR TO STARTING CONSTRUCTION AN INSPECTION OF EXISTING MANHOLES AND CATCH BASINS WILL BE MADE BY THE CITY AND THE CONTRACTOR TO DETERMINE THE AMOUNT OF EXISTING DEBRIS IN THESE STRUCTURES. UPON COMPLETION OF THE CONTRACT, THE CONTRACTOR SHALL CLEAN ONLY THOSE STRUCTURES WHERE DEBRIS HAS BEEN ADDED DUE TO CONSTRUCTION. THIS WORK WILL NOT BE PAID FOR SEPARATELY, BUT SHALL BE CONSIDERED INCLUDED IN THE COST OF MOBILIZATION.
- 28. WHEN DIRECTED BY THE COMMISSIONER EXISTING CATCH BASINS SHALL BE REMOVED AND REPLACED WITH NEW DEPARTMENT OF WATER MANAGEMENT (DOWM) STANDARD CATCH BASINS. THIS WORK SHALL BE PAID FOR UNDER REMOVING CATCH BASINS AND UNDER CATCH BASINS, TYPE 1, 4' DIAMETER (INCLUDING FRAMES AND LIDS).
- 29. WHERE PROPOSED CATCH BASINS CAN NOT BE INSTALLED DUE TO EXISTING CONFLICTS, INLETS MAY BE SUBSTITUTED PENDING APPROVAL OF THE COMMISSIONER WITH CONCURRENCE OF THE DEPARTMENT OF WATER MANAGEMENT.
- 30. EXISTING CATCH BASIN LATERALS TO BE REUSED MUST BE RODDED AND FLUSHED IN THE PRESENCE OF THE DEPARTMENT OF WATER MANAGEMENT INSPECTOR. A NEW CONNECTION TO THE MAIN SEWER IS REQUIRED IF THE EXISTING CATCH BASIN LATERAL IS NOT APPROVED BY THE SEWER INSPECTOR. THE FLUSHING OF THE EXISTING LATERAL WILL BE CONSIDERED INCLUDED IN THE COST OF THE SEWER AND CATCH BASIN WORK ITEMS.
- 31. IN LOCATIONS WHERE THE MAIN SEWER IS NOT BEING REPLACED AND THE EXISTING DRAINAGE FACILITIES ARE DISTURBED OR DAMAGED DURING CONSTRUCTION BY THE CONTRACTOR, IT WILL BE THE CONTRACTOR'S RESPONSIBILITY TO RESTORE AND REPLACE THE DAMAGED FACILITIES AT HIS EXPENSE TO THE SATISFACTION OF THE COMMISSIONER.

CONSTRUCTION NOTES

- THE CONTRACTOR MUST OBTAIN ALL NECESSARY PERMITS FROM THE CITY OF CHICAGO PRIOR TO COMMENCING CONSTRUCTION. THE COSTS ARE CONSIDERED INCLUDED IN THE COST OF "MOBILIZATION".
- 2. WORK ON THE RETAINING WALL TO THE NORTHEAST, SOUTHEAST AND SOUTHWEST, ACCESS WILL BE THROUGH ADJACENT PROPERTIES. THE ACCESS WILL OCCUR ON 7-FOOT TO 10-FOOT OR 10-FOOT WIDE TEMPORARY EASEMENT ACCESS AREAS ADJACENT TO WEBSTER AVENUE. WORK ON THESE PROPERTIES WILL BE ONLY FOR A LIMITED PERIOD OF TIME WHEN THE ACTUAL WORK IS BEING COMPLETED. PRIOR WRITTEN NOTIFICATION OF 2-WEEKS FOR WORK START IS REQUIRED FOR ALL 3 PROPERTIES. NO HEAVY EQUIPMENT, NOR VEHICLES, INCLUDING CONCRETE TRUCKS, WILL BE ALLOWED ON ANY OF THE THREE TEMPORARY ACCESS AREAS. WORK ON THE ENCLOSURE WALLS NEED TO BE PERFORMED FROM THE BRIDGE LEVEL MATERIALS CANNOT BE STORED WITHIN THE TEMPORARY ACCESS AREAS. THE CONTRACTOR SHALL NOT DAMAGE THE ADJACENT PROPERTIES INSIDE AND OUTSIDE THE ACCESS AREAS, AND SHALL AT THE CONTRACTOR'S EXPENSE REPAIR ANY DAMAGE THAT THE CONTRACTOR OR SUBCONTRACTOR MAY CAUSE. ANY DAMAGE SHALL BE REPAIRED TO PRE-CONSTRUCTION CONDITION, INCLUDING LANDSCAPING, PLANTERS, AND BENCHES

THE CONTRACTOR SHALL ERECT AND MAINTAIN A 7-FOOT TO 10-FOOT OR 10-FOOT WIDE PERIMETER FENCE AROUND THE ACCESS AREA TO THE NORTHEAST AND SOUTHEAST. ON THE SOUTHEAST PROPERTY, THE FENCE SHALL NOT ENCAPSLATE THE EXISTING PLANTERS AND BENCHES. ON THE NORTHEAST PROPERTY, THERE IS AN EXISTING FENCE. THIS EXISTING FENCE SHALL BE TEMPORARILY ROLLED BACK TO ALLOW FOR ACCESS TO THE ACCESS AREA AND THIS EXISTING FENCE WILL BE CLOSED OFF DURING CONSTRUCTION.

FOR THE MWRD TEMPORARY ACCESS AREA, THE CONTRACTOR SHALL CONTACT JOSEPH MEYER, ENGINEERING TECHNICIAN V OF THE DISTRICT'S MAINTENANCE & OPERATIONS DEPARTMENT AT (847)568-8224 OR VIA EMAIL AT MEYERJI@MWRD.ORG, PRIOR TO COMMENCING ANY ACTIVITY, AND JEFFREY YOURELL, DISTRICT INVESTIGATOR AT (312)751-6552 TO COORDINATE ACCESS TO THE ACCESS PREMISES. CONTRACTOR WILL NOT LEAVE THE MWRD PROPERTY OPEN WHEN NOT WORKING ON SITE.

CONSTRUCTION NOTES (CONTINUED)

- 3. A US COAST GUARD (USCG) PERMIT IS NOT REQUIRED, HOWEVER, THE CONTRACTOR MUST PROVIDE A DETAILED DESCRIPTION OF CONSTRUCTION MEANS AND METHODS AND AN EXPECTED DURATION OF CONSTRUCTION ACTIVITIES TO THE USCG A MINIMUM OF 30 DAYS PRIOR TO INITIATING MOBILIZATION AND CONSTRUCTION ACTIVITIES. THE DIMENSIONS OF ANY IN-WATER EQUIPMENT, TO INCLUDE THE LENGTH, BEAM, AND DRAFT OF ANY NEEDED CONSTRUCTION BARGES, SHOULD ALSO BE INCLUDED. AS BUILT PLANS MUST ALSO BE SUBMITTED TO THE NINTH COAST GUARD DISTRICT OFFICE AT THE COMPLETION OF THE PROJECT.
- 4. STORAGE OF CONSTRUCTION EQUIPMENT AND CONSTRUCTION MATERIAL IS NOT PERMITTED ON THE BRIDGE. THE CONTRACTOR SHALL WORK WITH THE CITY OF CHICAGO TO DETERMINE AN APPROPRIATE CONSTRUCTION STORAGE LOCATION.
- 5. THE CONTRACTOR IS PROHIBITED FROM STORING A BARGE IN THE RIVER WITHOUT PRIOR APPROVAL FROM THE US COAST GUARD AND THE COMMISSIONER.
- 5. BEARING SEAT SURFACES SHALL BE CONSTRUCTED OR ADJUSTED TO THE DESIGNATED ELEVATIONS WITHIN A TOLERANCE OF 1/8 INCH. ADJUSTMENT SHALL BE MADE EITHER BY GRINDING THE SURFACE OR BY SHIMMING THE BEARING. TWO ADJUSTING SHIMS OF THE DIMENSIONS OF THE BOTTOM OF THE PLATE SHALL BE PROVIDED FOR EACH BEARING PLACED AS DETAILED. THE FINAL NUMBER AND THICKNESS OF SHIM PLATES MUST BE DETERMINED IN THE FIELD. THE COST OF THE SHIM PLATES IS INCLUDED IN THE COST OF "FURNISHING AND ERECTING STRUCTURAL STEEL".
- 7. NO CONSTRUCTION JOINT EXCEPT THOSE SHOWN ON THE PLANS WILL BE ALLOWED UNLESS ORDERED OR APPROVED BY THE COMMISSIONER.
- 8. THE CONTRACTOR SHALL COMPLETE ALL CONSTRUCTION OPERATIONS (MOVABLE PART) DETAILED IN THE CONTRACT PLANS AND SPECIFICATIONS WHILE THE BASCULE BRIDGE LEAVES ARE IN THE CLOSED POSITION UNITESS PERMISSION IS OBTAINED FROM THE COMMISSIONER.
- 9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE STABILITY OF THE BASCULE BRIDGE IN ALL PHASES OF CONSTRUCTION. PRIOR TO REPLACEMENT OF STEEL MEMBERS DETAILED ON THE STRUCTURAL DRAWINGS, THE CONTRACTOR SHALL SUBMIT A CONSTRUCTION PROCEDURE SHOWING EACH STEP OF THE REMOVAL AND REPLACEMENT PROCESS. THE CONTRACTOR SHALL ALSO SUBMIT CALCULATIONS SEALED BY A LICENSED STRUCTURAL ENGINEER IN THE STATE OF ILLINOIS SHOWING THAT ALL MEMBERS WILL REMAIN STABLE AND NOT BECOME OVERSTRESSED DURING ANY STEP OF THE REMOVAL AND REPLACEMENT PROCESS. THE COST ASSOCIATED WITH PREPARING CALCULATIONS AND REMOVAL AND REPLACEMENT PROCEDURE IS INCLUDED IN THE COST OF REMOVAL OF EXISTING STRUCTURAL STEEL.
- 10. THE CONTRACTOR SHALL UTILIZE APPROPRIATE CONSTRUCTION PROCEDURES TO ENSURE THAT ALL STRUCTURAL MEMBERS ARE SQUARE DURING THE INSTALLATION OF THE LATERAL BRACING FOR THE ROADWAY STRINGERS, SIDEWALK STRINGERS, FLOORBEAMS AND THE CHORDS OF THE MAIN TRUSS MEMBERS.
- 11. THE CONTRACTOR IS RESPONSIBLE FOR PROPER ALIGNMENT OF THE TWO LEAVES IN ORDER TO ENGAGE THE NEW CENTER LOCK BAR AND ALLOW FOR PROPER FIT. THE ALIGNMENT IS NECESSARY TO ENSURE THAT THE CENTERLINE OF THE BOTTOM CHORD MEMBERS OF EACH LEAF CONFORM TO THE DESIGN GRADES AND ALIGN WITHIN A TOLERANCE OF $\pm 1/8^{\prime\prime}$ AT THE CENTER OF THE STRUCTURE. NECESSARY VERTICAL ADJUSTMENTS CAN BE MADE AT THE ANCHOR COLUMN BUMPERS AND LIVE LOAD SHOES BY THE USE OF SHIM PLATES TO ENSURE THAT THE BOTTOM CHORDS OF EACH LEAF ARE ALIGNED.
- 12. THE CONTRACTOR IS RESPONSIBLE FOR TRAFFIC CONTROL ON LOCAL ROADS AND CITY STREETS NEAR THE PROJECT SITE. REFER TO MAINTENANCE OF TRAFFIC SHEETS FOR DETAILS. THE CONTRACTOR SHALL COORDINATE CONSTRUCTION STAGING AND TRAFFIC CONTROL OPERATIONS WITH ADJOINING OR OVERLAPPING CONSTRUCTION CONTRACTS, INCLUDING BARRICADE PLACEMENT NECESSARY TO PROVIDE A UNIFORM DETOUR PATTERN PRIOR TO AND THROUGHOUT THE DURATION OF THE PROJECT. SEE SPECIAL PROVISIONS.
- 13. PEDESTRIAN TRAFFIC ACROSS WEBSTER AVE BASCULE BRIDGE MUST BE MAINTAINED AT ALL TIMES DURING CONSTRUCTION.
- 14. DURING A LIMITED ASBESTOS SURVEY CONDUCTED IN SEPTEMBER 2013, ASBESTOS CONTAINING MATERIALS (ACM) WERE IDENTIFIED ON EXISTING CLOTH WIRE INSULATION AND PIPE FITTINGS LOCATED INSIDE THE BRIDGE HOUSES. SHOULD SIMILAR MATERIAL BE FOUND IN CURRENTLY UNEXPOSED AREAS OF THE BUILDING, IT SHOULD ASSUMED TO BE ASBESTOS CONTAINING MATERIAL AND TREATED ACCORDINGLY. THE CONTRACTOR SHALL TAKE APPROPRIATE PRECAUTIONS TO DEAL WITH THE PRESENCE OF ASBESTOS ON THIS PROJECT. SEE SPECIAL PROVISIONS.



USER NAME = PJLAUX	DESIGNED - IJL	REVISED -	
	CHECKED - PJL	REVISED -	
PLOT SCALE =	DRAWN - IJL	REVISED -	
PLOT DATE = 1/5/2021	CHECKED - JIG	REVISED -	

CITY OF CHICAGO

DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

GENERAL NOTES - SHEET 1 OF 2

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	G-2
CDOT	PROJECT NO. E-1-525		3 of 210

STEEL FABRICATION NOTES

- DOMESTIC STEEL ACT: ALL IRON AND STEEL PRODUCTS, WHICH ARE TO BE INCORPORATED INTO STATE PROJECTS SHALL BE DOMESTICALLY MANUFACTURED OR PRODUCED AND FABRICATED. THE CONTRACTOR SHALL OBTAIN FROM THE IRON OR STEEL PRODUCER AND/OR FABRICATOR, IN ADDITION TO THE MILL ANALYSIS, A CERTIFICATION THAT ALL IRON OR STEEL MATERIALS MEET THESE DOMESTIC SOURCE REQUIREMENTS. THE APPLICATION OF ALL COATINGS, EPOXY, GALVANIZING, PAINTING, ETC., TO METAL PRODUCTS SHALL BE DOMESTICALLY APPLIED.
- 2. ALL STRUCTURAL STEEL IS TO BE AASHTO M270, (ASTM A709 GRADE 50) UNLESS
- 3. FASTENERS ARE TO BE %" DIAMETER HIGH STRENGTH BOLTS, AASHTO M164 (ASTM A325), TYPE I MECHANICALLY GALVANIZED, UNLESS NOTED OTHERWISE, OPEN HOLES ARE TO BE 15/6" DIAMETER, UNLESS OTHERWISE NOTED, AND EXCEPT AS REQUIRED BY THE EXISTING DETAILS.
- 4. ALL CONTACT SURFACES ON NEW AND EXISTING STEEL, INCLUDING CONNECTION BOLTS, NUT OR WASHER CONTACT AREAS, ARE TO BE FREE OF SCALE, BURRS, DIRT, OTHER FOREIGN MATERIALS, OIL, PREVIOUSLY APPLIED PAINT, LACOUER, OR OTHER COATINGS THAT WOULD PREVENT SOLID SEATING OF THE CONNECTED PARTS.
- 5. FIELD WELDING IS NOT ALLOWED EXCEPT WHEN APPROVED BY THE COMMISSIONER.
- 6 FIELD SPLICES SHALL BE AS SHOWN ON THE PLANS OR OTHERWISE APPROVED IN WRITING BY THE COMMISSIONER.
- 7. FABRICATION DRAWINGS SHALL SHOW THE WEIGHT OF ALL PARTS.
- 8. ROLLED SHAPED MEMBERS SHALL BE FABRICATED WITH NATURAL CAMBER UP.
- MAIN TRUSS CONNECTIONS ARE TO BE CONSIDERED SLIP-CRITICAL IN ACCORDANCE WITH AASHTO CRITERIA. CONTACT SURFACES OF BOLTED PARTS MUST MEET THE REQUIREMENTS OF CLASS A AS DESIGNATED IN AASHTO CRITERIA.
- 10. THE FABRICATOR SHALL NOTE THAT BOLTS ARE USED IN PLACE OF ORIGINAL RIVETS. IT IS ANTICIPATED THAT THIS CHANGE MAY CAUSE SOME INTERFERENCES AND ERECTION CONFLICTS FOR BOLT INSTALLATION. THE CONTACTOR AND THE FABRICATOR SHALL REVIEW THE DRAWINGS FOR SUCH CONFLICTS IN DETAIL AND BRING THEM TO THE ATTENTION OF THE COMMISSIONER FOR RESOLUTION PRIOR TO FABRICATION. LIKEWISE. THE CONTRACTOR SHALL VERIFY THE FIT AND CLEARANCE OF ALL NEW COMPONENTS THAT CONNECT TO EXISTING OR REPLACED SECTIONS OF THE PRIMARY TRUSS MEMBERS. WHERE INTERFERENCES OCCUR, THE CONTRACTOR SHALL USE CLIPPED WASHERS TRIM THE EDGE OF INTERFERING MEMBER OR INSTALL COUNTERSUNK BOLTS AS POSSIBLE MITIGATING SOLUTIONS. ALL MODIFICATIONS MUST BE APPROVED BY THE COMMISSIONER PRIOR TO IMPLEMENTATION.
- 11. AT LOCATIONS WHERE RIVETS ARE TO BE REPLACED WITH HIGH STRENGTH BOLTS, THE BOLTS SHALL BE ORIENTED SO THAT THE HEAD OF THE BOLT IS PLACED ON THE OUTSIDE EXPOSED SURFACE OF THE MEMBER.
- 12. BOLTS, WASHERS AND NUTS ARE NOT INCLUDED IN THE ESTIMATED WEIGHT OF STRUCTURAL STEEL, BUT ARE CONSIDERED PART OF THE MEMBER TO BE PROVIDED. THE COST OF THESE ITEMS ARE INCLUDED IN THE COST OF "FURNISHING AND ERECTING STRUCTURAL STEEL".
- 13. IT IS THE CONTRACTOR'S RESPONSIBILITY TO TAKE MEASUREMENTS OF THE EXISTING STRUCTURE WHEREVER NEW STEEL IS TO BE INSTALLED OR CONNECTED INTO THE EXISTING MATERIAL PRIOR TO ORDERING OR FABRICATING NEW STEEL. THE CONTRACTOR IS RESPONSIBLE FOR THE PROPER FITTING AND ASSEMBLY OF ALL PARTS OF THIS WORK THE CONTRACTOR'S SHOP DRAWINGS MUST INDICATE WHICH DIMENSIONS WERE OBTAINED BY ACTUAL FIELD MEASUREMENTS.
- 14. THE LOCATION AND DIAMETER OF HOLES IN NEW CONNECTING MATERIAL MUST MATCH HOLES IN THE EXISTING STRUCTURE. HOLES IN EXISTING STRUCTURE MAY BE ENLARGED ONLY WITH THE PERMISSION OF COMMISSIONER AND IN ACCORDANCE WITH THE SPECIFICATIONS. HOLES MAY BE SUB-DRILLED OR SUB-PUNCHED IN NEW MATERIAL WHERE HOLES IN EXISTING MATERIAL ARE TO BE MATCHED. FIELD REAMING OF HOLES IN NEW MATERIAL THAT ARE SUB-PUNCHED OR SUB-DRILLED AND ARE TO MATCH EXISTING HOLES IS ACCEPTABLE, PROVIDED THE SUB-HOLE IS FULLY CONTAINED IN THE OUTLINE OF THE REAMED HOLE. FINAL HOLES MUST BE ROUND AND MAY NOT BE
- 15. BOLT AND RIVET SPACINGS AND SIZES MUST BE VERIFIED IN THE FIELD BY THE CONTRACTOR PRIOR TO ORDERING MATERIAL FOR FABRICATION
- 16. BOTTOM FLANGES AND WEBS OF FLOORBEAMS SHALL BE CONSIDERED NON-REDUNDANT MAIN LOAD CARRYING ELEMENTS SUBJECT TO TENSILE STRESSES. REQUIRING CONFORMANCE TO FRACTURE CRITICAL MEMBER REQUIREMENTS. FCM. UNLESS OTHERWISE NOTED (SEE SPECIAL PROVISIONS), FABRICATION OF SUCH ELEMENTS SHALL BE ACCORDING TO CLAUSE 12 OF THE AASHTO/AWS D1.5 BRIDGE WELDING CODE

REMOVAL OF EXISTING STRUCTURAL STEEL NOTES

- 1. THE CONTRACTOR MUST PERFORM ALL WORK WITH CARE SUCH THAT ALL MATERIALS WHICH ARE TO REMAIN IN PLACE WILL NOT BE DAMAGED. IF THE CONTRACTOR DAMAGES ANY MATERIALS WHICH ARE TO REMAIN IN PLACE, THE DAMAGED MATERIALS MUST BE REPAIRED OR REPLACED IN A MANNER SATISFACTORY TO THE COMMISSIONER AT NO ADDITIONAL EXPENSE
- FLAME CUTTING OF EXISTING STRUCTURAL STEEL MEMBERS WHICH ARE TO REMAIN IN PLACE IS NOT ALLOWED.
- EXISTING RIVETS ARE TO BE REMOVED AND REPLACED WITH HIGH STRENGTH BOLTS BY MECHANICAL METHODS FLAME CUTTING FOR THE PURPOSE OF REMOVING EXISTING RIVETS WILL NOT BE ALLOWED.
- EXISTING RIVETS ARE TO BE REMOVED ONE RIVET AT A TIME AND REPLACED WITH A HIGH STRENGTH BOLT BEFORE REMOVING THE NEXT
- ALTHOUGH PLANS DESIGNATE EXISTING HOLES AS CONTAINING RIVETS, SOME RIVETS MAY HAVE BEEN REPLACED WITH H.S. BOLTS UNDER PREVIOUS REPAIR CONTRACTS. WHERE THESE LOCATIONS REQUIRE RIVET REMOVAL AND REPLACEMENT WITH H.S. BOLTS, REPLACE EXISTING H.S. BOLTS WITH NEW H.S. BOLTS.
- WHERE EXISTING STRUCTURAL STEEL TO REMAIN HAS BEEN CUT OR NEW HOLES HAVE BEEN DRILLED, THE EDGES MUST BE DRESSED TO A SMOOTH, UNIFORM SURFACE WITH NO NOTCHES OR GOUGES.
- REMOVAL OF EXISTING BRIDGE STRUCTURAL STEEL SHALL BE DONE IN A MANNER AND SEQUENCE CONSISTENT WITH THE REHABILITATION OF THE
- THE EXISTING STRUCTURAL STEEL COATING ON THE BRIDGE IS ASSUMED TO CONTAIN LEAD. THE CONTRACTOR SHALL TAKE APPROPRIATE PRECAUTIONS TO DEAL WITH THE PRESENCE OF LEAD ON THIS PROJECT. SEE SPECIAL
- THE EXISTING COATINGS INSIDE THE BRIDGE HOUSES ON THE STRUCTURAL STEEL, CONDUIT, STAIRS, SAFETY RAILING, DOORS, DOOR FRAMES, WINDOW FRAMES, WALLS, AND MACHINERY ARE ASSUMED TO CONTAIN LEAD. THE CONTRACTOR SHALL TAKE APPROPRIATE PRECAUTIONS TO DEAL WITH THE PRESENCE OF LEAD ON THIS PROJECT. SEE SPECIAL PROVISIONS.

HIGHWAY STANDARDS

000001-05	STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS
001001-01	AREAS OF REINFORCEMENT REBARS
001006	DECIMAL OF AN INCH AND OF A FOOT
280001-04	TEMPORARY EROSION CONTROL SYSTEMS
420001-07	PAVEMENT JOINTS
420401-06	BRIDGE APPROACH PAVEMENT
424001-05	CURB RAMPS FOR SIDEWALKS
701606-10	MULTILANE, SINGLE LANE CLOSURE, 2W W/ MOUNTABLE SIGN
701901-08	TRAFFIC CONTROL DEVICES
704001-04	TEMPORARY CONCRETE BARRIER
420406	PAVEMENT CONNECTOR (HMA) FOR BRIDGE APPROACH SLAB
515001	NAME PLATE

PROJECT COMMITMENTS

STRICT ADHERENCE BY THE CONTRACTOR TO BEST MANAGEMENT PRACTICES FOR EROSION AND SEDIMENT CONTROL SHALL BE USED TO MINIMIZE THE POSSIBILITY OF ANY ADVERSE IMPACTS TO THE NORTH BRANCH OF THE CHICAGO RIVER AND THE VICINITY LISTED

CONCRETE AND REINFORCEMENT NOTES

- 1. ALL CONCRETE ON NEW BRIDGE DECK SHALL BE HIGH PERFORMANCE CONCRETE (HPC). SEE SPECIAL PROVISIONS
- 2. REINFORCEMENT BARS SHALL BE NEW, DEFORMED, EPOXY COATED BARS CONFORMING TO THE REQUIREMENTS OF AASHTO M31 OR M322, GRADE
- 3. ALL CONSTRUCTION JOINTS SHALL BE BONDED ACCORDING TO IDOT STANDARD SPECIFICATION FOR ROAD AND BRIDGE CONSTRUCTION ARTICLE 503.09, UNLESS OTHERWISE NOTED.
- 4. REINFORCEMENT BARS MUST CONFORM TO THE REQUIREMENTS OF ASTM A706, GRADE 60. SEE SPECIAL PROVISIONS.
- 5. ALL REINFORCING BARS SHALL BE EPOXY COATED UNLESS OTHERWISE
- 6. HORIZONTAL AND VERTICAL CONSTRUCTION JOINTS SHOWN OR NOTED ON THE PLANS ARE RECOMMENDED. ANY DEVIATION FROM THOSE SHOWN MUST HAVE APPROVAL OF THE COMMISSIONER.
- 7. ALL CONSTRUCTION JOINTS SHALL BE BONDED ACCORDING TO IDOT STANDARD SPECIFICATION FOR ROAD AND BRIDGE CONSTRUCTION ART 503.09, UNLESS OTHERWISE NOTED.
- 8. ALL EXPOSED EDGES OF CONCRETE MUST BE CHAMFERED 3/4" UNLESS OTHER MEMBERS ARE ERECTED FLUSH WITH THEM AND UNLESS OTHERWISE NOTED IN THE PLANS
- 9 ALL EXPOSED CONCRETE SURFACES MUST BE TREATED WITH SILICONE SEALER. LINSEED OIL OR OTHER SURFACE TREATMENTS ARE NOT ACCEPTABLE
- 10. CONCRETE COVER FOR REINFORCEMENT:

EXCEPT AS OTHERWISE NOTED OR SHOWN ON THE DRAWINGS, REINFORCEMENT SHALL HAVE A MINIMUM CONCRETE COVER AS FOLLOWS.

- BOTTOM OF FOUNDATIONS 3"
- BACKFILLED SURFACES 2½'
- SURFACES IN CONTACT WITH WATER 21/2"
- INTERIOR SURFACES (NOT EXPOSED TO WATER) 2"
- EXTERIOR WALLS AND SLABS (ABOVE GROUND SURFACE) 2"

SUBSTRUCTURE, BEARINGS AND **ANCHOR BOLT NOTES**

- 1. REINFORCEMENT BARS SHALL BE NEW, DEFORMED, EPOXY COATED BARS CONFORMING TO THE REQUIREMENTS OF AASHTO M31 OR M322, GRADE 60.
- 2. ANCHOR RODS SHALL CONFORM TO ASTM F1554 GRADE 105 WITH SUPPLEMENTARY REQUIREMENT S4. TOP ENDS OF ANCHOR RODS SHALL BE GROUND FLAT AT 90 DEGREES TO ROD AXIS, FINAL SURFACE ROUGHNESS SHALL NOT EXCEED 125 MICRO-INCH.
- 3. ANCHOR ROD WASHERS SHALL CONFORM TO ASTM F436.
- 4. ANCHOR ROD NUTS SHALL CONFORM TO ASTM A563, GRADE DH WITH SUPPLEMENTARY REQUIREMENTS S1 AND S2.

115)	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684

ER NAME = IJLOPEZ	DESIGNED - IJL	REVISED -	
	CHECKED - PJL	REVISED -	
DT SCALE =	DRAWN - IJL	REVISED -	
DT DATE = \$DATE\$	CHECKED - JIG	REVISED -	

LAN SURVEYED BY	
	DATE
CADD FILE NAME	

1		BY	DATE
PROFILE	SURVEYED		
	PLOTTED		
NOTE BOOK	GRADES CHECKED		
100	B.M. NOTED		
ON	CONTRACTOR POLICE		

1 6560020 MIN-SPECIAL WASTE DISCORD. C.V. 70 70 70 70 70 70 70 70	ITEM NO.	CODE NO.	ITEM	UNIT	TOTAL QUANTITY
5	1	66900200	NON-SPECIAL WASTE DISPOSAL	CU YD	70
4 69901002 CR-STEE POWITCHING OF RECOLATED SUBTRACES CAL 04 30 30 30 30 30 30 30	2	66900530	SOIL DISPOSAL ANALYSIS	EACH	2
1 1 1 1 1 1 1 1 1 1	3	66901001	REGULATED SUBSTANCES PRE-CONSTRUCTION PLAN	LUMP SUM	1
6 6710350 MOBILIZATION LIMP SUM 1	4	66901002	ON-SITE MONITORING OF REGULATED SUBSTANCES	CAL DA	30
7 COT6700010 EMINEER'S FISCUS OFFICE CAL MO 15 8 20076000 TRAINESS ALLOWANCE 1.00 9 10 ************************************	5	66901003	REGULATED SUBSTANCES FINAL CONSTRUCTION REPORT	LUMP SUM	1
8 Z0076600 TRAINESS	6	67100100	MOBILIZATION	LUMP SUM	1
P	7	CDOT6700010	ENGINEER'S FIELD OFFICE	CAL MO	15
FUNDISH AND INSTALL PROJECT SIGN, TYPE B FACH THE STATES AND INSTALL PROJECT SIGN, BAMBER FACH THE STATES AND INSTALL PROJECT SIGN, BAMBER ALLOWANCE BS. 50.00 ALLOWANCE BS	8	Z0076600	TRAINEES	ALLOWANCE	1,000
FORMISH AND INSTALL PROJECT SIGN. BANNER FORMISH AND INSTALL PRO	9	******	FURNISH AND INSTALL PROJECT SIGN, TYPE A	EACH	2
ALLOWANCE 35,000	10	******	FURNISH AND INSTALL PROJECT SIGN, TYPE B	EACH	1
13	11	******	FURNISH AND INSTALL PROJECT SIGN, BANNER	EACH	1
14	12	******	ASBESTOS ABATEMENT	ALLOWANCE	35,000
15 20200100 FASTH EXCAVATION CLU YD 70	13	******	LEAD-BASED PAINT ABATEMENT	ALLOWANCE	150,000
16 25200110 SODDING, SALT TOLERANT	14	******	HAZARDOUS MATERIALS ABATEMENT	ALLOWANCE	30,000
17 28800510 INLET FILTERS	15	20200100	EARTH EXCAVATION	CU YD	70
18 3101200 SUBBASE GRAMULAR MATERIAL, TYPE B 4"	16	25200110	SODDING, SALT TOLERANT	SQ YD	81
19 35501320 HOT-MIX ASPHALT BASE COURSE, 9" 50 YD 33 33 32 4050325 BITUMINOUS MATERIALS (PRIME COAT) POUND 74 75 75 75 75 75 75 75	17	28000510	INLET FILTERS	EACH	4
20	18	31101200	SUBBASE GRANULAR MATERIAL, TYPE B 4"	SQ YD	440
21	19	35501320	HOT-MIX ASPHALT BASE COURSE, 9"	SQ YD	33
22 40603080 HOT-MIX ASPHALT BINDER COURSE. IL-19.0, N50 TON 139 23 40603340 HOT-MIX ASPHALT SURFACE COURSE, MIX "D", N70 TON 82 24 42000070 PAVEMENT CONNECTOR, (HMA) FOR BRIDGE APPROACH SLAB SQ YD 48 25 44000100 PAVEMENT REMOVAL SQ YD 201 26 44000105 HOT-MIX ASPHALT SURFACE REMOVAL, 1 1/2" SQ YD 15 27 44000155 HOT-MIX ASPHALT SURFACE REMOVAL, 4" SQ YD 841 28 44000500 COMBINATION CURB AND GUTTER REMOVAL FOOT 245 29 44000600 SIDEWALK REMOVAL SQ FT 2,903 30 72000100 SIGN PANEL - TYPE 1 SQ FT 11 31 78000100 THERMOPLASTIC PAVEMENT MARKING - LINE 4" FOOT 523 32 78000200 THERMOPLASTIC PAVEMENT MARKING TYPE I - LETTERS AND SYMBOLS SQ FT 180 34 78008200 POLYUREA PAVEMENT MARKING TYPE I - LINE 4" FOOT 506 35 78008210 POLYUREA PAVEMENT MARKIN	20	40600275	BITUMINOUS MATERIALS (PRIME COAT)	POUND	74
23 40603340 HOT-MIX ASPHALT SURFACE COURSE, MIX "D", N70 TON 82 24 42000070 PAVEMENT CONNECTOR (HMA) FOR BRIDGE APPROACH SLAB SQ YD 48 25 44000100 PAVEMENT REMOVAL SQ YD 201 26 44000155 HOT-MIX ASPHALT SURFACE REMOVAL, 1 1/2" SQ YD 15 27 44000165 HOT-MIX ASPHALT SURFACE REMOVAL, 4" SQ YD 841 28 44000500 COMB INATION CURB AND GUTTER REMOVAL FOOT 245 29 44000600 SIGWALK REMOVAL SQ FT 2,903 30 72000100 SIGWALK REMOVAL SQ FT 11 31 78000100 THERMOPLASTIC PAVEMENT MARKING - LETTERS AND SYMBOLS SQ FT 11 31 78000200 THERMOPLASTIC PAVEMENT MARKING - LINE 24" FOOT 523 33 78008200 POLYUREA PAVEMENT MARKING TYPE 1 - LINE 4" FOOT 20 36 78008230 POLYUREA PAVEMENT MARKING TYPE 1 - LINE 6" FOOT 20 38 78008250 POLYUREA PAVEMENT MARKING TYPE 1 - LIN	21	40600290	BITUMINOUS MATERIALS (TACK COAT)	POUND	576
24 42000070 PAVEMENT CONNECTOR (HMA) FOR BRIDGE APPROACH SLAB SQ YD 48 25 44000100 PAVEMENT REMOVAL SQ YD 201 26 44000155 HOT-MIX ASPHALT SURFACE REMOVAL, 1" SQ YD 15 27 44000165 HOT-MIX ASPHALT SURFACE REMOVAL, 4" SQ YD 841 28 44000500 COMBINATION CURB AND GUTTER REMOVAL FQOT 245 29 44000600 SIDEWALK REMOVAL SQ FT 2,903 30 72000100 SIGN PANEL - TYPE 1 SQ FT 11 31 78000100 THERMOPLASTIC PAVEMENT MARKING - LETTERS AND SYMBOLS SQ FT 69 32 78000200 THERMOPLASTIC PAVEMENT MARKING - LINE 24" FOOT 516 34 78008200 POLYUREA PAVEMENT MARKING TYPE I - LETTERS AND SYMBOLS SQ FT 180 35 78008210 POLYUREA PAVEMENT MARKING TYPE I - LINE 4" FOOT 20 36 78008230 POLYUREA PAVEMENT MARKING TYPE I - LINE 6" FOOT 20 37 78008230 POLYUREA PAVEMENT MARKING TYPE I - LINE 12" FOOT 20 38 7800823	22	40603080	HOT-MIX ASPHALT BINDER COURSE, IL-19.0, N50	TON	139
25 44000100 PAVEMENT REMOVAL SQ YD 201 26 44000155 HOT-MIX ASPHALT SURFACE REMOVAL, 1 1/2" SQ YD 15 27 44000165 HOT-MIX ASPHALT SURFACE REMOVAL, 4" SQ YD 841 28 44000500 COMBINATION CURB AND GUTTER REMOVAL FOOT 245 29 44000600 SIDEWALK REMOVAL SQ FT 2,903 30 72000100 SIGN PANEL - TYPE 1 SQ FT 11 31 78000100 THERMOPLASTIC PAVEMENT MARKING - LETTERS AND SYMBOLS SQ FT 69 32 78000200 THERMOPLASTIC PAVEMENT MARKING - LINE 4" FOOT 523 33 78008200 POLYUREA PAVEMENT MARKING TYPE I - LETTERS AND SYMBOLS SQ FT 180 34 78008200 POLYUREA PAVEMENT MARKING TYPE I - LINE 4" FOOT 50 35 78008210 POLYUREA PAVEMENT MARKING TYPE I - LINE 4" FOOT 20 36 78008230 POLYUREA PAVEMENT MARKING TYPE I - LINE 24" FOOT 20 38 78008250 POLYUREA PAVEMENT MARKING TYPE I - LINE 24" FOOT 23 39 CDOT4240010	23	40603340	HOT-MIX ASPHALT SURFACE COURSE, MIX "D", N70	TON	82
26 44000155 HOT-MIX ASPHALT SURFACE REMOVAL, 1 1/2" SQ YD 15 27 44000165 HOT-MIX ASPHALT SURFACE REMOVAL, 4" SQ YD 841 28 44000500 COMBINATION CURB AND GUTTER REMOVAL FOOT 245 29 44000600 SIDEWALK REMOVAL SQ FT 2,903 30 72000100 SIGN PANEL - TYPE 1 SQ FT 11 31 78000100 THERMOPLASTIC PAVEMENT MARKING - LETTERS AND SYMBOLS SQ FT 69 32 78000200 THERMOPLASTIC PAVEMENT MARKING - LINE 4" FOOT 523 33 78000650 THERMOPLASTIC PAVEMENT MARKING TYPE I - LETTERS AND SYMBOLS SQ FT 180 34 78008200 POLYUREA PAVEMENT MARKING TYPE I - LINE 4" FOOT 50 35 78008210 POLYUREA PAVEMENT MARKING TYPE I - LINE 6" FOOT 20 36 78008250 POLYUREA PAVEMENT MARKING TYPE I - LINE 6" FOOT 20 38 78008250 POLYUREA PAVEMENT MARKING TYPE I - LINE 24" FOOT 23 39 CDOT4240010 PORTLAND CEMENT CONCRETE SIDEWALK 5 INCH SQ FT 2,507	24	42000070	PAVEMENT CONNECTOR (HMA) FOR BRIDGE APPROACH SLAB	SQ YD	48
27 44000165 HOT-MIX ASPHALT SURFACE REMOVAL, 4" SQ YD 841 28 44000500 COMBINATION CURB AND GUTTER REMOVAL FOOT 245 29 44000600 SIDEWALK REMOVAL SQ FT 2,903 30 72000100 SIGN PANEL - TYPE 1 SQ FT 11 31 78000100 THERMOPLASTIC PAVEMENT MARKING - LETTERS AND SYMBOLS SQ FT 69 32 78000200 THERMOPLASTIC PAVEMENT MARKING - LINE 4* FOOT 523 33 78008200 POLYUREA PAVEMENT MARKING TYPE I - LETTERS AND SYMBOLS SQ FT 180 34 78008210 POLYUREA PAVEMENT MARKING TYPE I - LINE 4" FOOT 906 36 78008230 POLYUREA PAVEMENT MARKING TYPE I - LINE 6" FOOT 201 37 78008250 POLYUREA PAVEMENT MARKING TYPE I - LINE 12" FOOT 20 38 78008270 POLYUREA PAVEMENT MARKING TYPE I - LINE 24" FOOT 23 39 CDOT4240010 PORTLAND CEMENT CONCRETE SIDEWALK 5 INCH SQ FT 2,507 40 CDOT4240020 PORTLAND CEMENT CONCRETE SIDEWALK 8 INCH SQ FT 436	25	44000100	PAVEMENT REMOVAL	SQ YD	201
28 44000500 COMBINATION CURB AND GUTTER REMOVAL FOOT 245 29 44000600 SIDEWALK REMOVAL SQ FT 2,903 30 72000100 SIGN PANEL - TYPE 1 SQ FT 11 31 78000100 THERMOPLASTIC PAVEMENT MARKING - LETTERS AND SYMBOLS SQ FT 69 32 78000200 THERMOPLASTIC PAVEMENT MARKING - LINE 4* FOOT 523 33 78008500 THERMOPLASTIC PAVEMENT MARKING - LINE 24* FOOT 516 34 78008200 POLYUREA PAVEMENT MARKING TYPE 1 - LETTERS AND SYMBOLS SQ FT 180 35 78008210 POLYUREA PAVEMENT MARKING TYPE 1 - LINE 4** FOOT 906 36 78008230 POLYUREA PAVEMENT MARKING TYPE 1 - LINE 6** FOOT 201 37 78008250 POLYUREA PAVEMENT MARKING TYPE 1 - LINE 12* FOOT 20 38 78008270 POLYUREA PAVEMENT MARKING TYPE 1 - LINE 24* FOOT 23 39 CDOT4240010 PORTLAND CEMENT CONCRETE SIDEWALK 5 INCH SQ FT 2,507 40 CDOT4240020 PORTLAND CEMENT CONCRETE SIDEWALK 8 INCH SQ FT 436 <td>26</td> <td>44000155</td> <td>HOT-MIX ASPHALT SURFACE REMOVAL, 1 1/2"</td> <td>SQ YD</td> <td>15</td>	26	44000155	HOT-MIX ASPHALT SURFACE REMOVAL, 1 1/2"	SQ YD	15
29 44000600 SIDEWALK REMOVAL SQ FT 2,903 30 72000100 SIGN PANEL - TYPE 1 SQ FT 11 31 78000100 THERMOPLASTIC PAVEMENT MARKING - LETTERS AND SYMBOLS SQ FT 69 32 78000200 THERMOPLASTIC PAVEMENT MARKING - LINE 4" FOOT 523 33 7800650 THERMOPLASTIC PAVEMENT MARKING - LINE 24" FOOT 516 34 78008200 POLYUREA PAVEMENT MARKING TYPE I - LETTERS AND SYMBOLS SQ FT 180 35 78008210 POLYUREA PAVEMENT MARKING TYPE I - LINE 4" FOOT 906 36 78008230 POLYUREA PAVEMENT MARKING TYPE I - LINE 6" FOOT 201 37 78008250 POLYUREA PAVEMENT MARKING TYPE I - LINE 12" FOOT 23 38 78008270 POLYUREA PAVEMENT MARKING TYPE I - LINE 24" FOOT 23 39 CDOT4240010 PORTLAND CEMENT CONCRETE SIDEWALK 5 INCH SQ FT 2,507 40 CDOT4240020 PORTLAND CEMENT CONCRETE SIDEWALK 8 INCH SQ FT 436	27	44000165	HOT-MIX ASPHALT SURFACE REMOVAL, 4"	SQ YD	841
30 72000100 SIGN PANEL - TYPE 1 SQ FT 11	28	44000500	COMBINATION CURB AND GUTTER REMOVAL	FOOT	245
31 78000100 THERMOPLASTIC PAVEMENT MARKING - LETTERS AND SYMBOLS SQ FT 69	29	44000600	SIDEWALK REMOVAL	SQ FT	2,903
32 78000200 THERMOPLASTIC PAVEMENT MARKING - LINE 4" FOOT 523 33 78000650 THERMOPLASTIC PAVEMENT MARKING - LINE 24" FOOT 516 34 78008200 POLYUREA PAVEMENT MARKING TYPE I - LETTERS AND SYMBOLS SQ FT 180 35 78008210 POLYUREA PAVEMENT MARKING TYPE I - LINE 4" FOOT 906 36 78008230 POLYUREA PAVEMENT MARKING TYPE I - LINE 6" FOOT 201 37 78008250 POLYUREA PAVEMENT MARKING TYPE I - LINE 12" FOOT 20 38 78008270 POLYUREA PAVEMENT MARKING TYPE I - LINE 24" FOOT 23 39 CDOT4240010 PORTLAND CEMENT CONCRETE SIDEWALK 5 INCH SQ FT 2,507 40 CDOT4240020 PORTLAND CEMENT CONCRETE SIDEWALK 8 INCH SQ FT 419.5 41 CDOT4240040 PORTLAND CEMENT CONCRETE ADA RAMP 8 INCH SQ FT 436	30	72000100	SIGN PANEL - TYPE 1	SQ FT	11
33 78000650 THERMOPLASTIC PAVEMENT MARKING - LINE 24" FOOT 516	31	78000100	THERMOPLASTIC PAVEMENT MARKING - LETTERS AND SYMBOLS	SQ FT	69
34 78008200 POLYUREA PAVEMENT MARKING TYPE I - LETTERS AND SYMBOLS SQ FT 180 35 78008210 POLYUREA PAVEMENT MARKING TYPE I - LINE 4" FOOT 906 36 78008230 POLYUREA PAVEMENT MARKING TYPE I - LINE 6" FOOT 201 37 78008250 POLYUREA PAVEMENT MARKING TYPE I - LINE 12" FOOT 20 38 78008270 POLYUREA PAVEMENT MARKING TYPE I - LINE 24" FOOT 23 39 CDOT4240010 PORTLAND CEMENT CONCRETE SIDEWALK 5 INCH SQ FT 2,507 40 CDOT4240020 PORTLAND CEMENT CONCRETE SIDEWALK 8 INCH SQ FT 419.5 41 CDOT4240040 PORTLAND CEMENT CONCRETE ADA RAMP 8 INCH SQ FT 436	32	78000200	THERMOPLASTIC PAVEMENT MARKING - LINE 4"	FOOT	523
35 78008210 POLYUREA PAVEMENT MARKING TYPE I - LINE 4" FOOT 906 36 78008230 POLYUREA PAVEMENT MARKING TYPE I - LINE 6" FOOT 201 37 78008250 POLYUREA PAVEMENT MARKING TYPE I - LINE 12" FOOT 20 38 78008270 POLYUREA PAVEMENT MARKING TYPE I - LINE 24" FOOT 23 39 CDOT4240010 PORTLAND CEMENT CONCRETE SIDEWALK 5 INCH SQ FT 2,507 40 CDOT4240020 PORTLAND CEMENT CONCRETE SIDEWALK 8 INCH SQ FT 419.5 41 CDOT4240040 PORTLAND CEMENT CONCRETE ADA RAMP 8 INCH SQ FT 436	33	78000650	THERMOPLASTIC PAVEMENT MARKING - LINE 24"	FOOT	516
36 78008230 POLYUREA PAVEMENT MARKING TYPE I - LINE 6" FOOT 201 37 78008250 POLYUREA PAVEMENT MARKING TYPE I - LINE 12" FOOT 20 38 78008270 POLYUREA PAVEMENT MARKING TYPE I - LINE 24" FOOT 23 39 CDOT4240010 PORTLAND CEMENT CONCRETE SIDEWALK 5 INCH SQ FT 2,507 40 CDOT4240020 PORTLAND CEMENT CONCRETE SIDEWALK 8 INCH SQ FT 419.5 41 CDOT4240040 PORTLAND CEMENT CONCRETE ADA RAMP 8 INCH SQ FT 436	34	78008200	POLYUREA PAVEMENT MARKING TYPE I - LETTERS AND SYMBOLS	SQ FT	180
3778008250POLYUREA PAVEMENT MARKING TYPE I - LINE 12"FOOT203878008270POLYUREA PAVEMENT MARKING TYPE I - LINE 24"FOOT2339CDOT4240010PORTLAND CEMENT CONCRETE SIDEWALK 5 INCHSQ FT2,50740CDOT4240020PORTLAND CEMENT CONCRETE SIDEWALK 8 INCHSQ FT419.541CDOT4240040PORTLAND CEMENT CONCRETE ADA RAMP 8 INCHSQ FT436	35	78008210	POLYUREA PAVEMENT MARKING TYPE I - LINE 4"	FOOT	906
3878008270POLYUREA PAVEMENT MARKING TYPE I - LINE 24"FOOT2339CD0T4240010PORTLAND CEMENT CONCRETE SIDEWALK 5 INCHSQ FT2,50740CD0T4240020PORTLAND CEMENT CONCRETE SIDEWALK 8 INCHSQ FT419.541CD0T4240040PORTLAND CEMENT CONCRETE ADA RAMP 8 INCHSQ FT436	36	78008230	POLYUREA PAVEMENT MARKING TYPE I - LINE 6"	FOOT	201
39 CDOT4240010 PORTLAND CEMENT CONCRETE SIDEWALK 5 INCH 40 CDOT4240020 PORTLAND CEMENT CONCRETE SIDEWALK 8 INCH 41 CDOT4240040 PORTLAND CEMENT CONCRETE ADA RAMP 8 INCH 50 FT 419.5	37	78008250	POLYUREA PAVEMENT MARKING TYPE I - LINE 12"	FOOT	20
40 CDOT4240020 PORTLAND CEMENT CONCRETE SIDEWALK 8 INCH 41 CDOT4240040 PORTLAND CEMENT CONCRETE ADA RAMP 8 INCH 5Q FT 419.5	38	78008270	POLYUREA PAVEMENT MARKING TYPE I - LINE 24"	FOOT	23
41 CDOT4240040 PORTLAND CEMENT CONCRETE ADA RAMP 8 INCH SQ FT 436	39	CDOT4240010	PORTLAND CEMENT CONCRETE SIDEWALK 5 INCH	SQ FT	2,507
	40	CDOT4240020	PORTLAND CEMENT CONCRETE SIDEWALK 8 INCH	SQ FT	419.5
42 CDOT4240065 RADIAL DETECTABLE WARNING TILES (CAST IRON) SQ FT 61.5	41	CDOT4240040	PORTLAND CEMENT CONCRETE ADA RAMP 8 INCH	SQ FT	436
	42	CDOT4240065	RADIAL DETECTABLE WARNING TILES (CAST IRON)	SQ FT	61.5

wsp	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684	
-----	---	--

USER NAME = IJLOPEZ	DESIGNED - IJL	REVISED -
	CHECKED - PJL	REVISED -
PLOT SCALE =	DRAWN - IJL	REVISED -
PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -

CITY OF CHICAGO

DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

	RTE.	SECTION	COUNTY	SHEET NO.
ARY OF QUANTITIES - SHEET 1 OF 4	1388	11-E1525-00-BR	COOK	G-4
	CDOT	PROJECT NO. E-1-525		5 of 210

PLAN SURNEYED PLOTTED PLOTTED ALIGNMENT CHECKED	84	DATE
BOOK		
TE BOOK		
TE BOOK		
NO. CADD FILE NAME		

PROFILE	SURVEYED	BY	DATE
	PLOTTED		
NOTE BOOK	GRADES CHECKED		
	B.M. NOTED		

ITEM NO.	CODE NO.	ITEM	UNIT	TOTAL QUANT I TY
43	CDOT6020010	CATCH BASINS, TYPE 1, 4'-DIAMETER, TYPE 1 FRAME, OPEN LID (CITY OF CHICAGO)	EACH	S
44	CDOT6050020	REMOVING CATCH BASINS	EACH	2
45	CDOT6060020	COMBINATION CURB AND GUTTER TYPE B V.12	FOOT	265.5
46	X0326243	SEDIMENT CONTROL, SILT CURTAIN	LUMP SUM	1
47	X0327980	PAVEMENT MARKING REMOVAL - WATER BLASTING	SQ FT	516
48	******	REMOVE, STORE AND RE-ERECT SIGN PANEL	EACH	8
49	******	SIDEWALK REMOVAL (SPECIAL)	SQ FT	805
50	******	STORM SEWERS, TYPE 2, 8-INCH (EXTRA STRENGTH VITRIFIED CLAY PIPE)	FOOT	5
51	CDOT6640010	TEMPORARY CHAIN LINK FENCE WITH SCREENING, 6'	FOOT	80
52	X1400347	DETOUR TRAFFIC SIGNAL MODIFICATIONS AND MAINTENANCE	EACH	1
53	X7010218	TRAFFIC CONTROL AND PROTECTION, (SPECIAL)	LUMP SUM	1
54	20900110	POROUS GRANULAR BACKFILL	CU YD	375.0
55	50157300	PROTECTIVE SHIELD	SQ YD	1,188
56	50200100	STRUCTURE EXCAVATION	CU YD	86.0
57	50300260	BRIDGE DECK GROOVING	SQ YD	588
58	50300285	FORM LINER TEXTURED SURFACE	SQ FT	2,732
59	50500505	STUD SHEAR CONNECTORS	EACH	6,492
60	51500100	NAME PLATES	EACH	1
61	52100010	ELASTOMERIC BEARING ASSEMBLY, TYPE I	EACH	32
62	50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	108,630
63	52000110	PREFORMED JOINT STRIP SEAL	FOOT	108
64	52100520	ANCHOR BOLTS, 1"	EACH	64
65	59000200	EPOXY CRACK INJECTION	FOOT	176
66	CDOT5010030	CONCRETE REMOVAL	CU YD	253.8
67	CDOT5030020	HIGH PERFORMANCE CONCRETE STRUCTURES	CU YD	258.0
68	CDOT5030030	HIGH PERFORMANCE CONCRETE SUPERSTRUCTURES	CU YD	256.3
69	CDOT5030050	CLASS "SI" CONCRETE (MISCELLANEOUS)	CU YD	32.6
70	CDOT5870010	PROTECTIVE CONCRETE SEALER	SQ YD	1,100
71	X0323444	DECORATIVE STEEL RAILING	FOOT	210
72	X0326519	STEEL RAILING REMOVAL	FOOT	206
73	Z0001903	STRUCTURAL STEEL REMOVAL	POUND	486,420
74	Z0007101	CONTAINMENT AND DISPOSAL OF LEAD PAINT CLEANING RESIDUES NO. 1	LUMP SUM	1
75	Z0012754	STRUCTURAL REPAIR OF CONCRETE (DEPTH EQUAL TO OR LESS THAN 5 INCHES)	SQ FT	589
76	Z0012755	STRUCTURAL REPAIR OF CONCRETE (DEPTH GREATER THAN 5 INCHES)	SQ FT	99
77	******	BALANCING OF BRIDGE AND ALTERATION OF COUNTERWEIGHTS	LUMP SUM	1
78	******	BRIDGE OPERATION AND MAINTENANCE	LUMP SUM	1
79	******	CLEANING AND PAINTING EXISTING STEEL STRUCTURES	LUMP SUM	1
80	******	COUNTERWEIGHT PIT CLEANING	EACH	2
81	******	DOLPHINS	EACH	4
82	******	DRAINAGE SYSTEM	LUMP SUM	1
83	******	FURNISHING AND ERECTING 5-INCH GRATING, HALF CONCRETE FILLED	SQ FT	6,114
84	******	FURNISHING AND ERECTING FRP GRATING	SQ FT	3,227

11	<u> </u>)	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200	
•	<i>-</i> [CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684	

USER NAME = IJLOPEZ DESIGNED - IJL REVISED -REVISED -CHECKED - PJL DRAWN - IJL CHECKED - JIG PLOT SCALE = REVISED -PLOT DATE = 9/16/2020 REVISED -

CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

SECTION COUNTY SHEET NO. SUMMARY OF QUANTITIES - SHEET 2 OF 4 1388 11-E1525-00-BR COOK CDOT PROJECT NO. E-1-525

			RY	DATE
300K	PLAN	SURVEYED	i	
TE BOOK		DIOTTED		
E BOOK	MOTE BOOK	ALIGNMENT CHECKED		
	NOTE BOOK	RT. OF WAY CHECKED		
	NO.	CADD FILE NAME		

1000		BY	DATE
PROFILE	SURVEYED		
	PLOTTED		
MOTE BOOK	GRADES CHECKED		
100	B.M. NOTED		
CIZ	CALLO TIMEATOR POLITORIO		

ITEM NO.	CODE NO.	ITEM	UNIT	TOTAL QUANTITY
85	******	FURNISHING AND ERECTING STRUCTURAL STEEL	LUMP SUM	1
86	******	FURNISHING AND ERECTING STRUCTURAL STEEL, FIELD DISCOVERED CONDITIONS REPAIRED AS DIRECTED BY THE COMMISSIONER	POUND	20,000
87	******	FLOOR ACCESS HATCH	EACH	4
88	******	METAL LADDERS	EACH	4
89	******	PIER PROTECTION REPLACEMENT	FOOT	301
90	******	REMOVAL OF DETERIORATED CONNECTORS AND REPLACEMENT WITH HIGH STRENGTH BOLTS	EACH	51
91	******	REFURBISHING OF LIVE LOAD BEARINGS	EACH	4
92	******	REMOVAL OF EXISTING SUPERSTRUCTURES	EACH	1
93	*****	REMOVAL OF EXISTING GRID DECK	LUMP SUM	1
94	64300240	IMPACT ATTENUATORS (FULLY REDIRECTIVE, NARROW), TEST LEVEL 2	EACH	4
95	******	STEEL RAILING (SPECIAL)	FOOT	506
96	******	STRUCTURAL STEEL REPAIRS	POUND	61,620
97	*****	TEMPORARY SUPPORT	LUMP SUM	1
98	******	REMOVE EXISTING BRIDGE HOUSES	EACH	2
99	******	REMOVE EXISTING CONCRETE RAILINGS	LIN FOOT	180
100	*****	PRECAST CONCRETE WALL	SQ FT	1,500
101	******	PRECAST CONCRETE RAILINGS	LIN FOOT	180
102	******	METAL CLADDED WALL ASSEMBLY	SQ FT	590
103	******	PAINT GYPSUM BOARD CEILING	SQ FT	320
104	*****	LIFE RINGS	EACH	2
105	******	INTERIOR PAINTING	SQ FT	400
106	******	INSTALL AND PAINT STAIR RAILINGS	FOOT	100
107	******	PAINT CONCRETE FLOORS AND STAIRS	SQ FT	1,600
108	******	ALUMINUM FRAMED WINDOWS	SQ FT	485
109	******	STANDING SEAM METAL ROOFING	SQ FT	320
110	*****	EXTERIOR DOORS	EACH	4
111	******	BREAKDOWN FOUNDATION	EACH	5
112	******	BRIDGE HOUSE ELECTRICAL WORK	LUMP SUM	2
113	******	CHICAGO 2000 LUMINAIRE ARM, 8 FOOT, WITH SCROLL, 8'	EACH	4
114	******	CHICAGO 2000 MAST HEAD AND FINIAL FOR 10" POLE	EACH	7
115	******	CHICAGO 2000 POLE BASE	EACH	11
116	*****	CLEAN EXISTING MANHOLE OR HANDHOLE	EACH	2
117	******	COILABLE CONDUIT, HDPE, SCH# 80, DIRECTIONAL BORING, 2"	LIN FOOT	473
118	******	COILABLE CONDUIT, HDPE, SCH# 80, DIRECTIONAL BORING, 3"	LIN FOOT	229
119	******	CONCRETE FOUNATION, 28" DIAMETER, 1 1/4" ANCHOR RODS, 15" BOLT CIRCLE, 7 FEET	LIN FOOT	28
120	******	CONTROLLER STREET LIGHTING, RESIDENTIAL, 240V	EACH	1
121	******	DRILL EXISTING MANHOLE OR HANDHOLE	EACH	3
122	******	ELECTRICAL CABLE IN CONDUIT, 1/C #10	LIN FOOT	1,200
123	******	ELECTRICAL CABLE IN CONDUIT, 1/C #12	LIN FOOT	600
124	******	ELECTRICAL CABLE IN CONDUIT, 1/C #350 KCMIL	LIN FOOT	1410
125	******	ELECTRICAL CABLE IN CONDUIT, 1/C #2/0	LIN FOOT	840
126	******	ELECTRICAL CABLE IN CONDUIT, TRIPLEX 2 1/C NO.6, 1/C NO.8	LIN FOOT	1,893

wsp	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684	
-----	---	--

USER NAME = IJLOPEZ DESIGNED - IJL REVISED -CHECKED - PJL REVISED -DRAWN - IJL CHECKED - JIG PLOT SCALE = REVISED -PLOT DATE = 9/16/2020 REVISED -

CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

SECTION COUNTY SHEET NO. SUMMARY OF QUANTITIES - SHEET 3 OF 4 G-6 1388 11-E1525-00-BR COOK CDOT PROJECT NO. E-1-525

	BY	DATE
SURVEYED		
PLOTTED		
ALIGNMENT CHECKED	0	
JF WAY CHECK	a.	
O FILE NAME		

1000		BY	DATE
PROFILE	SURVEYED		
	PLOTTED		
MOTE BOOK	GRADES CHECKED		
100	B.M. NOTED		
NO.	STRUCTURE NOTATINS CHIKD		

ITEM NO.	CODE NO.	ITEM	UNIT	TOTAL QUANTITY
127	******	ELECTRICAL HANDHOLE, 30" DIAMETER WITH 24" FRAME AND LID	EACH	2
128	******	INSTALL CONDUIT INTO EXISTING HELIX FOUNDATION	EACH	1
129	******	INTERCEPT EXISTING CONDUIT	EACH	2
130	******	JUNCTION BOX ATTACHED TO STRUCTURE	EACH	1
131	******	LED CHANNEL CENTER SIGNAL NAVIGATIONAL LIGHT	EACH	2
132	******	LED PIER SIGNAL NAVIGATIONAL LIGHT	EACH	4
133	******	LED RESIDENTIAL LUMINAIRE - 108W	EACH	8
134	******	LUMINAIRE CHICAGO 2000 PENDANT LED	EACH	11
135	******	MAINTAIN LIGHTING SYSTEM	LUMP SUM	1
136	******	MAST ARM STEEL 4'	EACH	8
137	******	PAINT EXISTING POLE COMPLETE	EACH	5
138	******	POLE, STEEL, ANCHOR BASE, 7" DIAMETER, 3 GAUGE, 20'	EACH	4
139	******	POLE, STEEL, ANCHOR BASE, 10" DIAMETER, 7 GAUGE, 34'-6"	EACH	2
140	******	CONDUIT ATTACHED TO STRUCTURE, 1"	LIN FOOT	1,236
141	******	CONDUIT ATTACHED TO STRUCTURE, 3"	LIN FOOT	55
142	******	CONDUIT ATTACHED TO STRUCTURE, 4"	LIN FOOT	52
143	******	PVC CONDUIT IN TRENCH, 3" PVC, SCH#80	LIN FOOT	15
144	******	REMOVE ANCHOR BASE POLE	EACH	5
145	******	REMOVE BRANCH WIRES / CABLES 2#6	LIN FOOT	1,414
146	******	REMOVE LUMINAIRE	EACH	10
147	******	REMOVE MAST ARM	EACH	10
148	******	REMOVE NAVIGATIONAL SIGNAL	EACH	6
149	******	REMOVE TS HEAD, 1-FACE	EACH	1
150	******	REMOVE PEDESTRIAN SIGNAL HEAD	EACH	1
151	*****	REMOVE MONOTUBE M.A. 20'	EACH	1
152	******	REMOVE JUNCTION BOX, TSS 18	EACH	1
153	******	REINSTALL SIGNAL HEAD, 3 SECTION, MAST ARM MOUNTED	EACH	1
154	******	REINSTALL PEDESTRIAN SIGNAL, BRACKET MOUNTED	EACH	1
155	******	REINSTALL MAST ARM, MONOTUBE, 20'	EACH	1
156	******	REINSTALL JUNCTION BOX	EACH	1
157	******	ROD AND CLEAN DUCT IN EXISTING DUCT SYSTEM	LIN FOOT	295
158	******	REMOVE ELECTRICAL BRIDGE HOUSE EQUIPMENT	LUMP SUM	1
159	******	SERVICE INSTALLATION - 300A	EACH	1
160	******	CLEANING, PAINTING, AND LUBRICATING OPERATING MACHINERY ASSEMBLIES	LUMP SUM	1
161	*****	REPLACEMENT OF CENTER LOCKS	EACH	2
162	*****	FURNISH AND INSTALL NEW SUMP PUMPS	EACH	2

USER NAME = IJLOPEZ DESIGNED - IJL REVISED -CHECKED - PJL REVISED -DRAWN - IJL CHECKED - JIG PLOT SCALE =
PLOT DATE = \$DATE\$ REVISED -REVISED -

CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

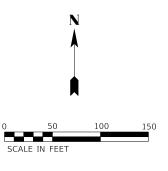
	RTE.	SECTION	COUNTY	SHEET NO.
ARY OF QUANTITIES - SHEET 4 OF 4	1388	11-E1525-00-BR	COOK	G-7
	CDOT	PROJECT NO. E-1-525		8 of 210

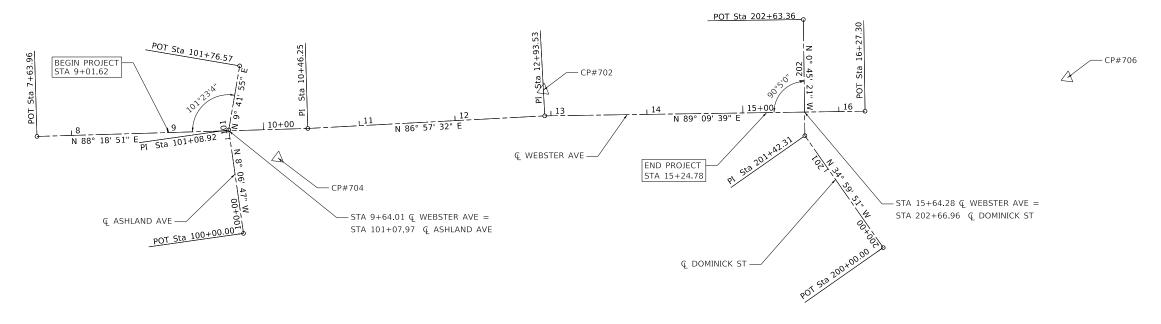


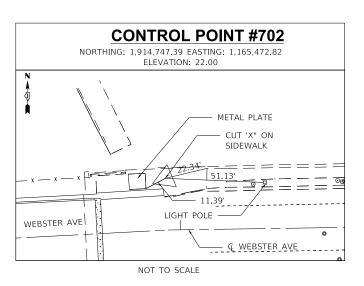


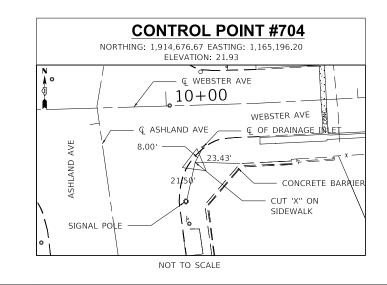
STATION	NORTH I NG	EASTING			
WEBSTER AVENUE					
7+63.96	1,914,699.05	1,164,945.19			
10+46.25	1,914,707.36	1,165,227.35			
12+93.53	1,914,720.48	1,165,474.29			
16+27.30	1,914,725.37	1,165,808.02			
ASHLAND AVENUE					
100+00.00	1,914,598.05	1,165,160.39			
101+08.92	1,914,705.87	1,165,145.02			
101+76.57	1,914,772.56	1,165,156.42			
	DOMINICK STREET				
200+00.00	1,914,583.22	1,165,826.95			
201+42.31	1,914,699.80	1,165,745.33			
202+63.36	1,914,820.84	1,165,743.73			

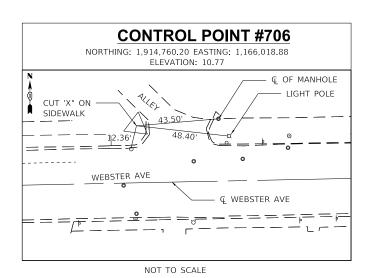
			BENCHMARKS	
MONUMENT	NORTHING	EASTING	ELEVATION	DESCRIPTION
BM 1	1,914,761.23	1,165,204.01	23.99	SQUARE CUT ON TOP OF STONE DOOR SILL OF ASHLAND AVE BRIDGE HOUSE DOORWAY (NE WEBSTER/ASHLAND)











WSP USA Inc.	Ų
30 N. LASALLE STREET SUITE 4200	
CHICAGO, IL 60602 TEL: (312) 782-8150	F

USER NAME = MMA PLOT DATE = \$DATE\$

DESIGNED - MMA REVISED CHECKED - RPH REVISED DRAWN REVISED CHECKED - RPH REVISED

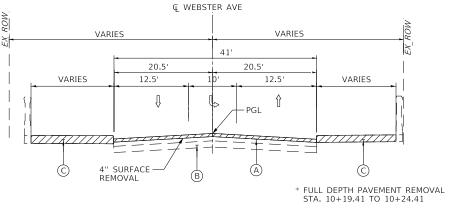
CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

ALIGNMENT, TIES, AND BENCHMARKS

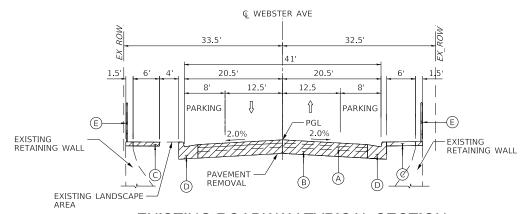
F.A.U.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	C-1
CDOT	PROJECT NO. E-1-525	ı	9 of 210





EXISTING ROADWAY TYPICAL SECTION

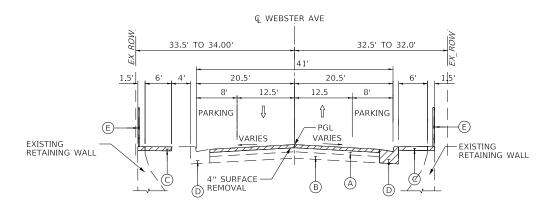
STA 10+02.43 TO STA 10+24.41 BRIDGE OMMISION STA 10+24.41 TO 13+14.35



EXISTING ROADWAY TYPICAL SECTION

STA 13+14.35 TO STA 13+55.19

* CURB AND GUTTER REMOVAL STA. 13+14.15 TO 13+44.37



EXISTING ROADWAY TYPICAL SECTION

STA 13+55.19 TO STA 15+20.65

EXISTING

A 6" HMA PAVEMENT B) 6" BRICK PAVERS (C) 5" PCC SIDEWALK D CURB AND GUTTER

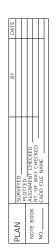
(E) PEDSTRIAN RAILING REMOVAL

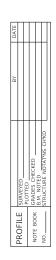
NOTES

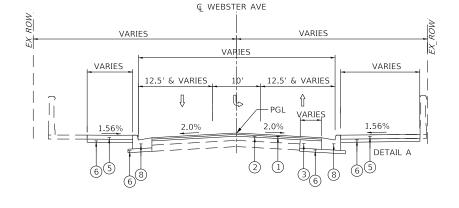
- 1. TYPICAL SECTIONS NOT TO SCALE.
- 2. SEE PROPOSED STRUCTURE PLANS FOR TOP OF THE WALL RECONSTRUCTION.
- 3. AS INDICATED ON THE ROADWAY PLANS, TYPICAL PROPOSED
- SIDWALK MUST HAVE A CROSS SLOPE OF 1:64 OR LESS.

 4. EXISTING PAVEMENT STRUCTURE HAS BEEN OBTAINED FROM PAVEMENT CORING, EXACT PAVEMENT LAYERS ARE NOT KNOWN.

NAME = MMA	DESIGNED - MMA	REVISED -
	CHECKED - RPH	REVISED -
SCALE =	DRAWN - MMA	REVISED -
DATE = \$DATE\$	CHECKED - RPH	REVISED -

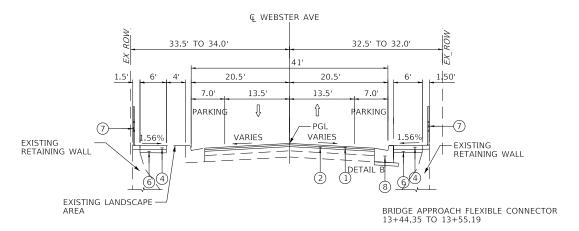






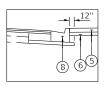
PROPOSED ROADWAY TYPICAL SECTION

STA 10+02.43 TO STA 10+24.41

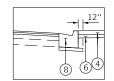


PROPOSED ROADWAY TYPICAL SECTION

STA 13+50.35 TO STA 15+20.65



DETAIL A AGGREGATE SUB-BASE AT
CONCRETE CURB AND GUTTER TY B V.12
WEST OF THE WEBSTER STRUCTURE



DETAIL B AGGREGATE SUB-BASE AT

CONCRETE CURB AND GUTTER TY B V.12 EAST OF THE WEBSTER STRUCTURE

BRIDGE OMMISION STA 10+24.41 TO 13+14.35 BRIDGE APPROACH STA 13+14.35 TO 13+44.35 BRIDGE APPROACH FLEXIBLE CONNECTOR 13+44.35 TO 13+55.19

PROPOSED

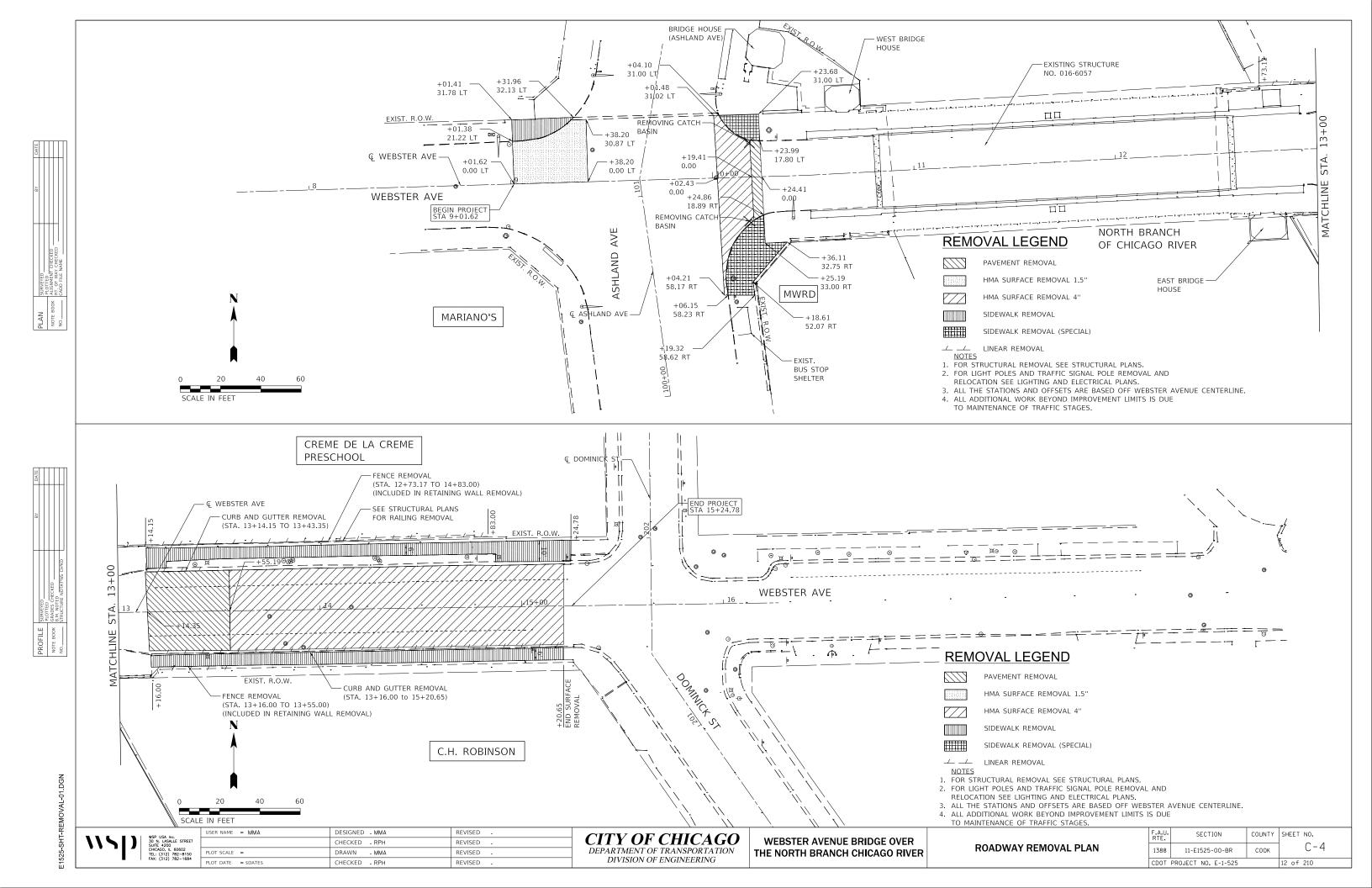
- (1) HOT-MIX ASPHALT SURFACE COURSE,
- MIX "D", N70. 1.5"
- (2) HOT-MIX ASPHALT BINDER COURSE, IL-19.0, N50, 2.5"
- 3) HOT-MIX ASPHALT BASE COURSE, 9"
- 4) PORTLAND CEMENT CONCRETE SIDEWALK 5 INCH
- 5) PORTLAND CEMENT CONCRETE SIDEWALK 8 INCH
- (6) SUBBASE GRANULAR MATERIAL, TYPE B 4" 7) PEDESTRAIN RAILING (SEE STRUCTURAL PLANS)
- (8) CONCRETE CURB AND GUTTER TYPE B V.12

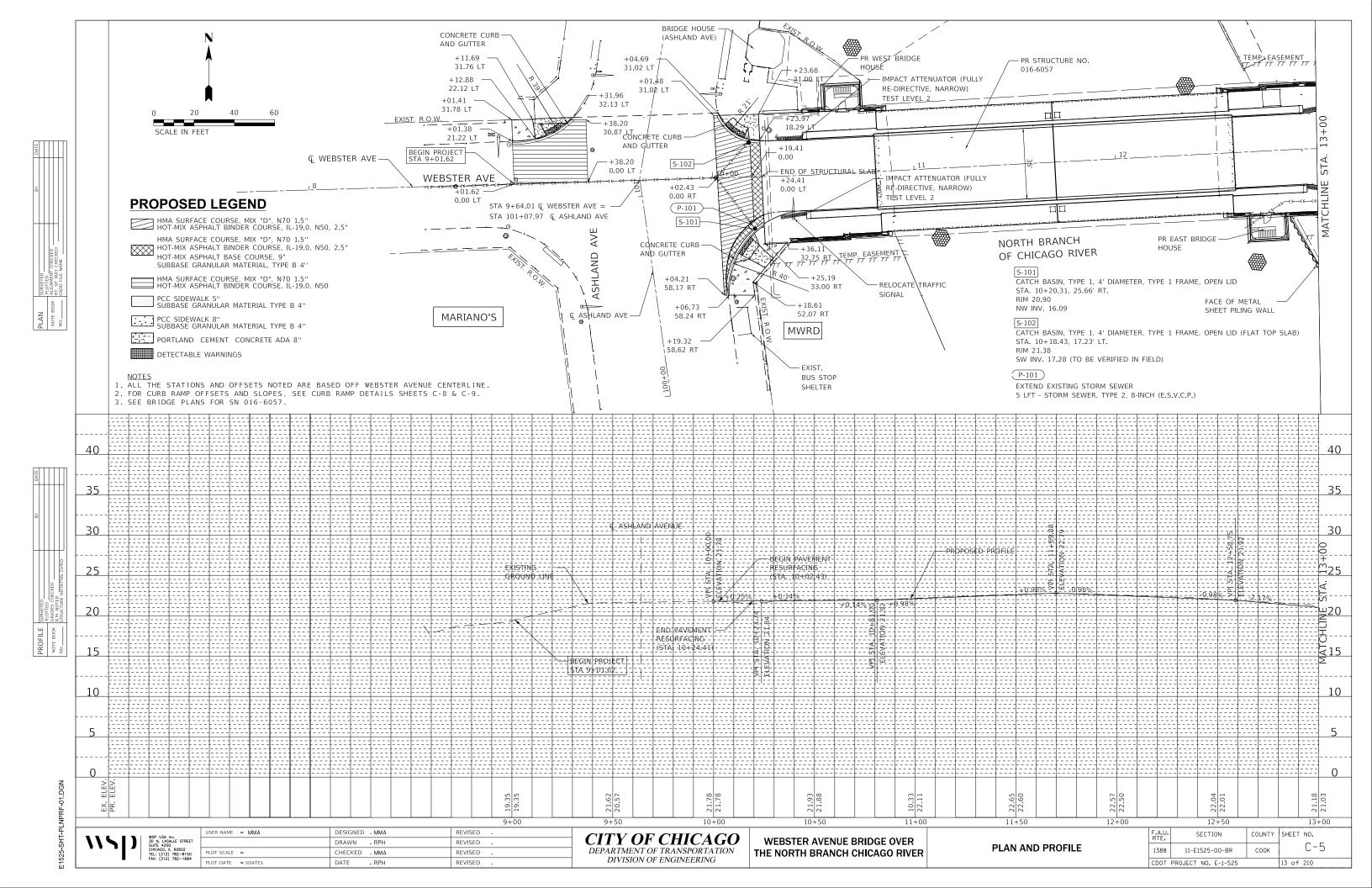
NOTES

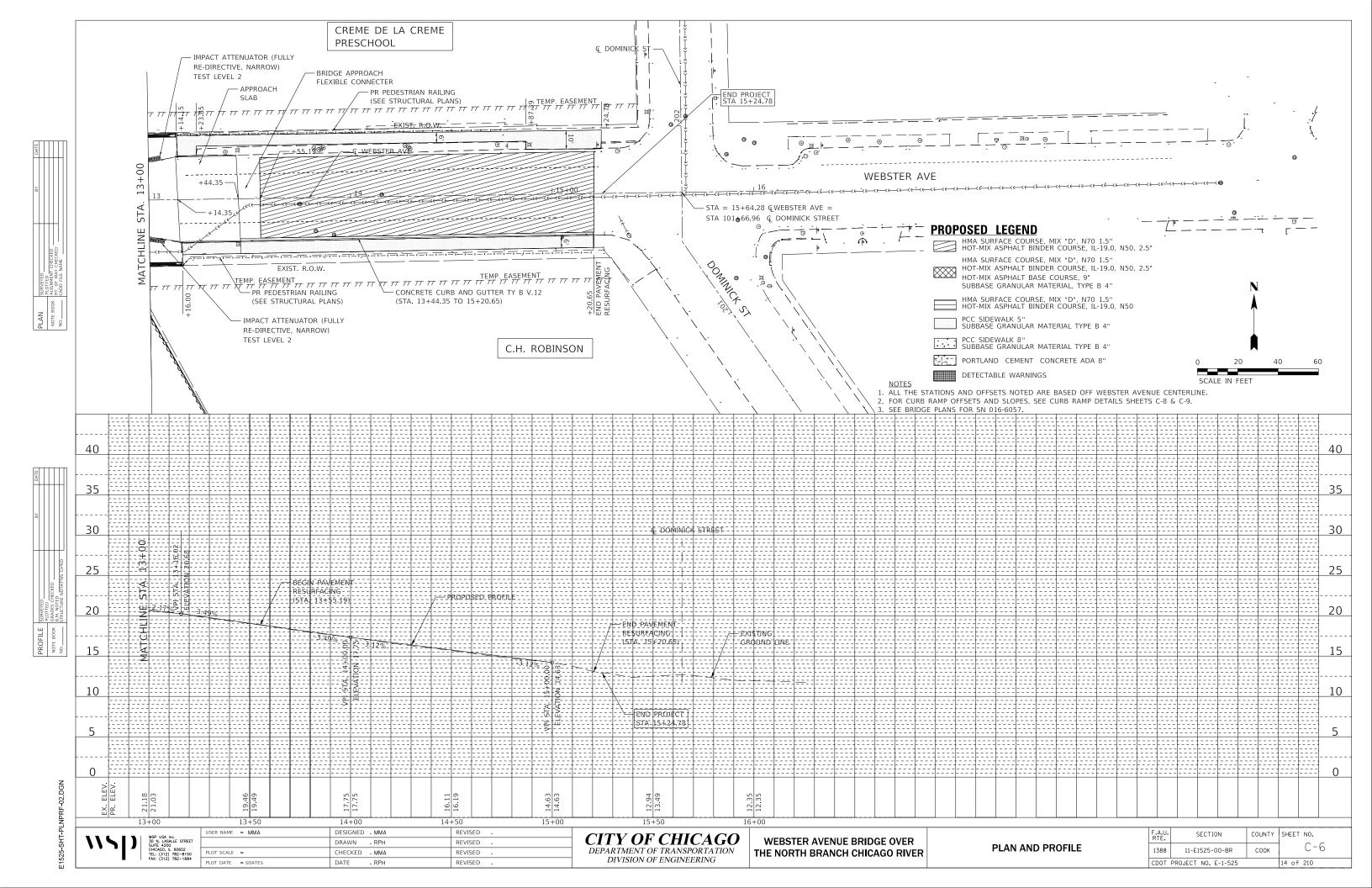
- TYPICAL SECTIONS NOT TO SCALE.
 SEE PROPOSED STRUCTURE PLANS FOR TOP OF THE WALL RECONSTRUCTION.
- 3. AS INDICATED ON THE ROADWAY PLANS, TYPICAL PROPOSED SIDWALK MUST HAVE A CROSS SLOPE OF 1:64 OR LESS.
- 4. EXISTING PAVEMENT STRUCTURE HAS BEEN OBTAINED FROM PAVEMENT CORING, EXACT PAVEMENT LAYERS ARE NOT KNOWN.

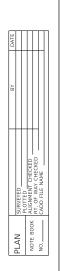
HOT-MIX ASPHALT MIXTURE REQUIRE	QUALITY	
MIXTURE TYPE AIR VOIDS @ NDES		MANAGEMENT
PAVEMENT RESURFACING		
HMA SURFACE COURSE, MIX "D", N70	4%@ 70 Gyr.	QC/QA OR QCP
HMA BINDER COURSE, IL-19.0, N50	4%@ 50 Gyr.	QC/QA OR QCP
PAVEMENT WIDENING		
HMA SURFACE COURSE, MIX "D", N70	4%@ 70 Gyr.	QC/QA OR QCP
HMA BINDER COURSE, IL-19.0, N50	4%@ 50 Gyr.	QC/QA OR QCP
HMA BASE COURSE, N50	4%@ 50 Gyr.	QC/QA OR QCP

SER NAME =	DESIGNED - MMA	REVISED -
	CHECKED - RPH	REVISED -
LOT SCALE =	DRAWN - MMA	REVISED -
LOT DATE = \$DATE\$	CHECKED - MMA	REVISED -

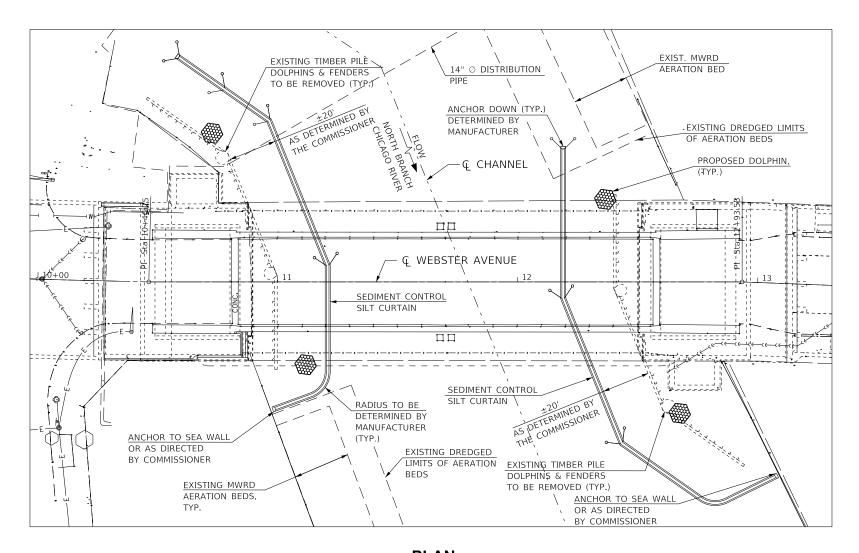


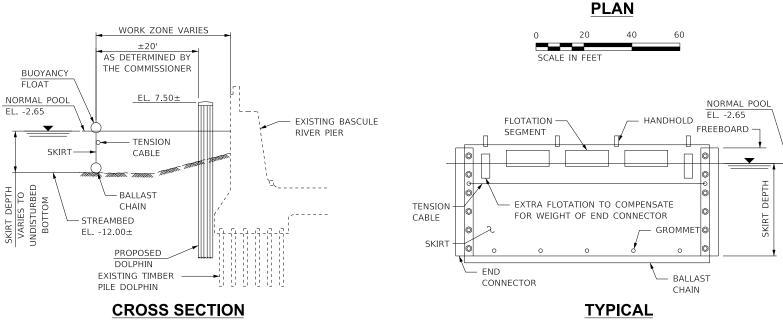












NOTES:

- 1. THE CONTRACTOR SHALL INSTALL A FLOATING BOOM AROUND THE WORK BARGE AREA IN THE EVENT OF OIL AND/OR FUEL LEAKAGE FROM CONSTRUCTION EQUIPMENT AND TO ACCUMULATE EXCESS BROKEN TIMBER PILE PIECES DURING THE DOLPHIN REMOVAL ACTIVITIES. THIS COST SHALL BE INCLUDED IN "SEDIMENT CONTROL SILT
- 2. ANCHOR CURTAIN TO MAINTAIN STATIONARY LOCATION THROUGHOUT CONSTRUCTION.
- 3. SEE SPECIAL PROVISIONS FOR SEDIMENT CONTROL SILT CURTAIN.

SEDIMENT CONTROL SILT CURTAIN DETAILS (NTS)

THRU WORKZONE

	 WSP USA Inc.	I
">	30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684	

USER NAME = PJLAUX PLOT SCALE = PLOT DATE = \$DATE\$

DESIGNED - PJL REVISED CHECKED - KKS REVISED DRAWN REVISED CHECKED - KKS REVISED

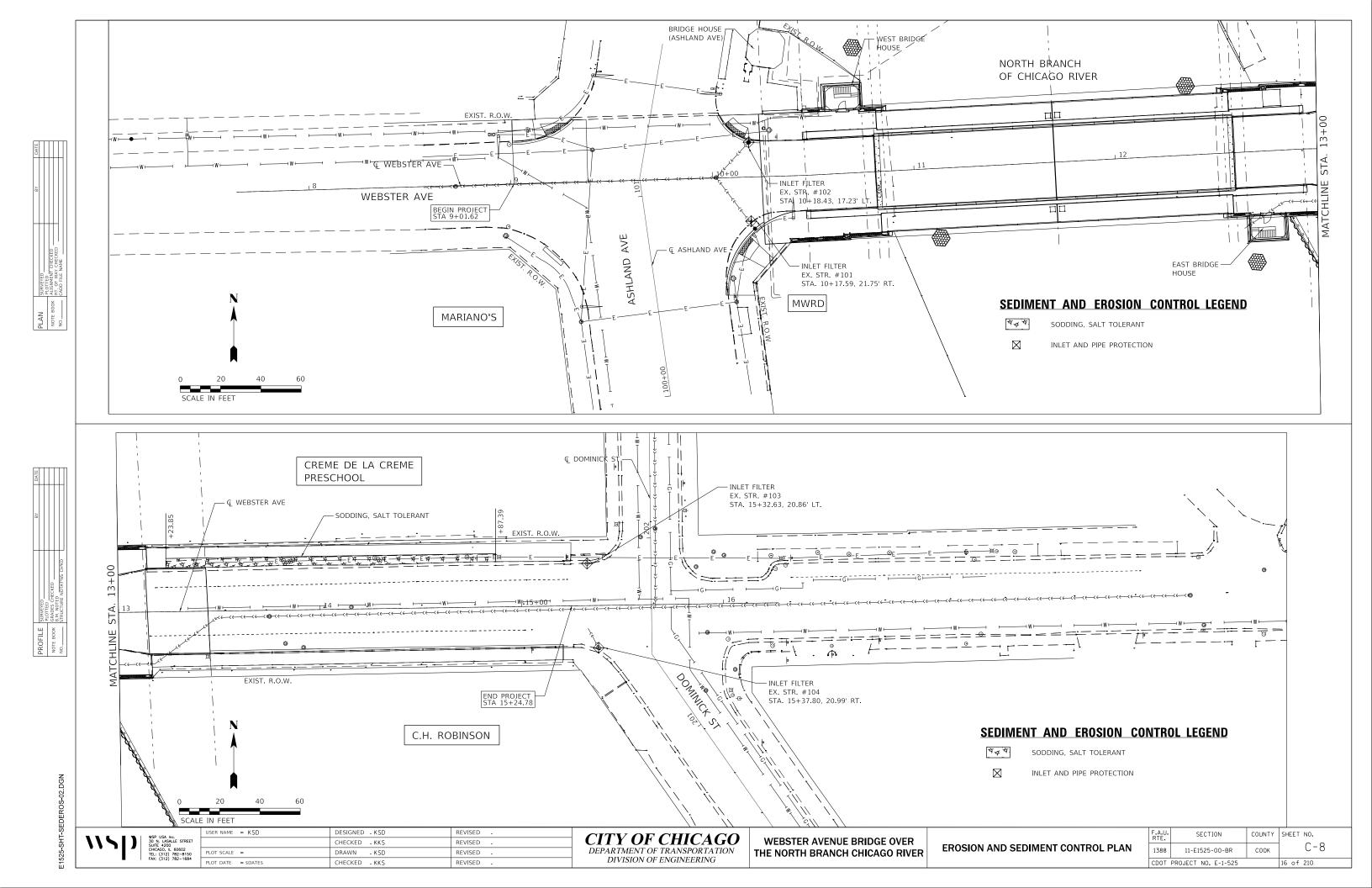
CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

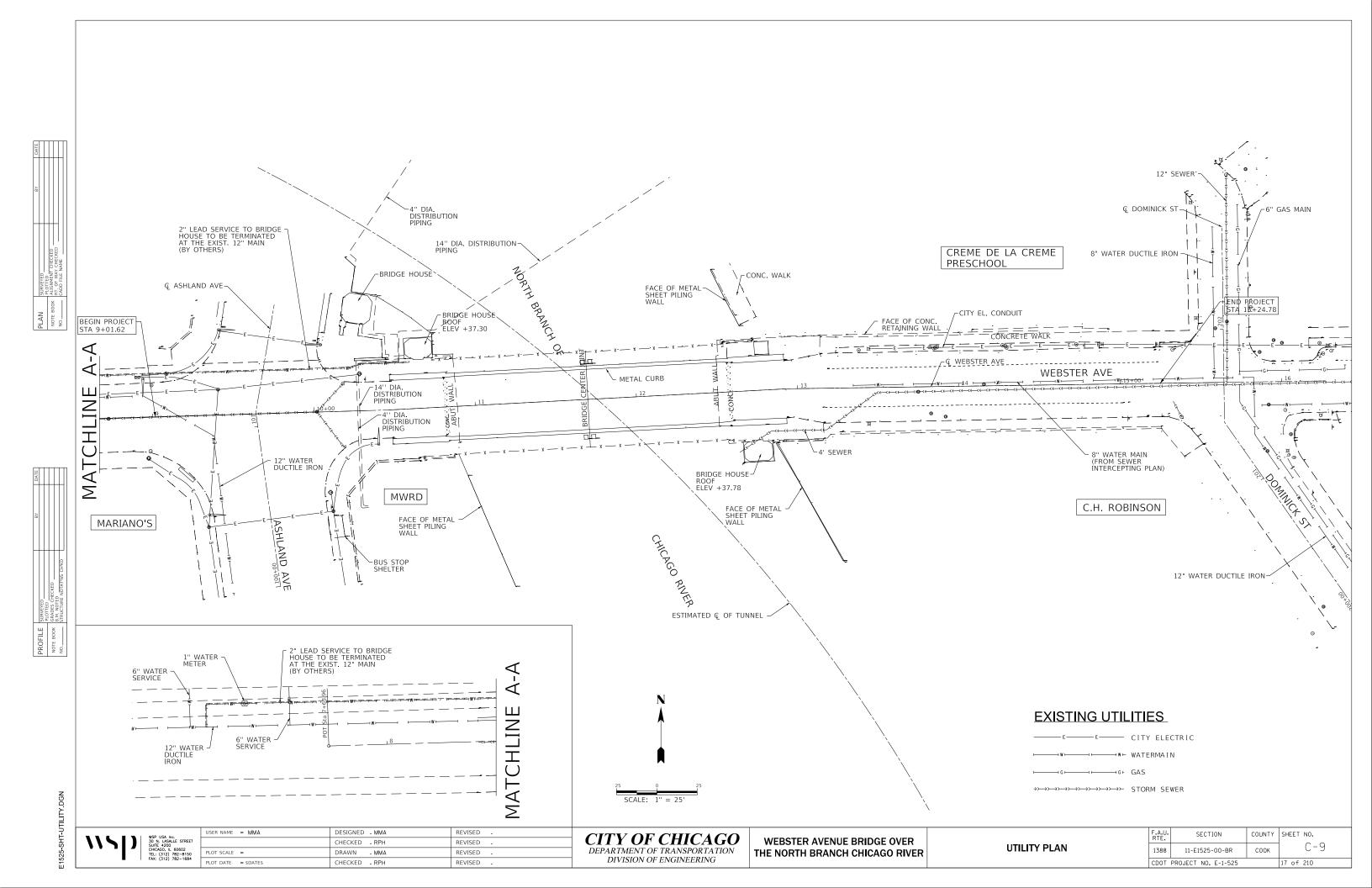
ELEVATION

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

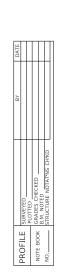
SEDIMENT CONTROL SILT CURTAIN

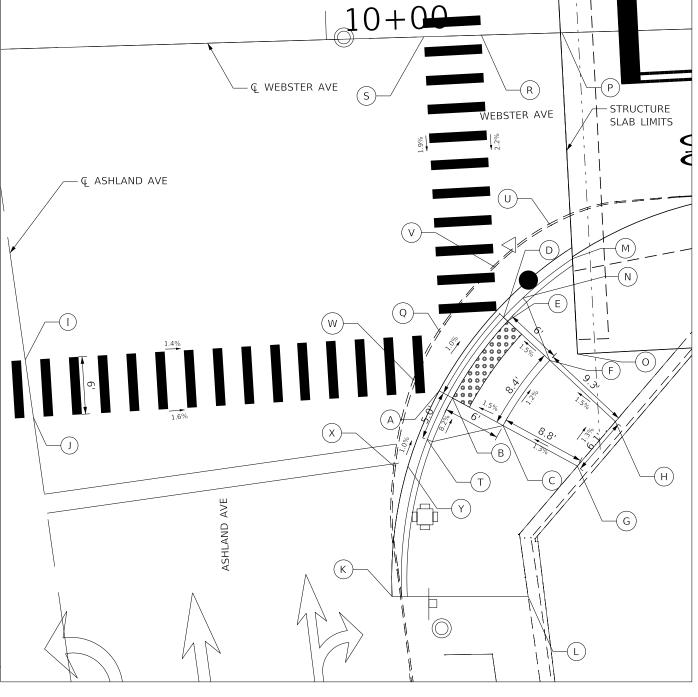
COUNTY SHEET NO. C-7 1388 11-E1525-00-BR СООК CDOT PROJECT NO. E-1-525







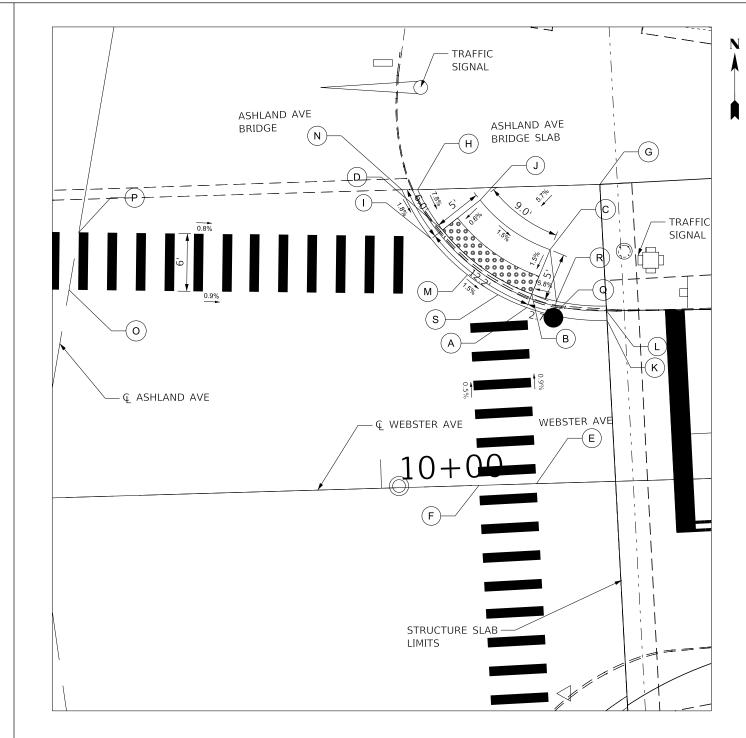




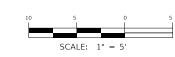
	STATION	OFFSET	ELEVATION		STATION	OFFSET	ELEVATION
Α	10+10.70	36.90 R	21.57	Р	10+24.58	0.00 R	21.84
В	10+12.60	37.71 R	21.57	Q	10+10.95	30.83 R	21.21
С	10+17.23	40 73 R	21.66	R	10+16.16	00 00 R	21.82
D	10+17.62	29 46 R	21.47	S	10+10.16	00.00 R	21.81
Е	10+18.03	29 87 R	21.47	Т	10+09.28	41 97 R	21.45
F	10+22.29	34.09 R	21.56	U	10+22.66	19.87 R	21.72
G	10+24.85	45 18 R	21.78	V	10+16.70	24 10 R	21.37
Н	10+28.94	40 66 R	21.70	W	10+08.01	35 68 R	21.34
I	9+67.66	32.42 R	21.80	Х	10+05.72	44 61 R	21.21
J	9+68.34	38 45 R	21.73	Υ	10+07.19	44 68 R	21.12
K	10+05.15	58 20 R	21.26		NOTES		
L	10+19.32	58 61 R	22.05		NOTES:	C AND OFF	TETE NOTED A
М	10+25.10	23 56 R	22.07	ALL STATIONS AND OFFSETS NOTED . WEBSTER AVENUE CENTERLINE.			
N	10+19.68	27.51 R	21.68	1	WEDSTER AV	ENUE CENT	EKLINE.
0	10+25.20	33 51 R	21.92				

WEBSTER AVE & ASHLAND AVE - SE CORNER

BASED OFF



		STATION	OFFSET	ELEVATION	М	10+09.61	21 96 L	21.70	
	4	10+15.69	18 37 L	21.62	N	10+03.05	31.00 L	21.91	
	В	10+16.25	19.76 L	21.62	0	9+68.59	24 18 L	22.07	
-		10+18.71	25 24 L	21.70	Р	9+69.80	30.21 L	22.09	
ı	5	10+07.15	26 71 L	21.80	Q	10+20.84	17 01 L	21.38	
	E	10+16.16	00 00 L	21.82	R	10+21.05	18 49 L	21.80	
	F	10+10.16	00 00 L	21.81	S	10+12.74	19.80 L	21.66	
(G	10+23.69	31 87 L	22.33					
- 1	+	10+04.69	30 97 L	22.26	N	OTES:			
	I	10+05.95	25 82 L	21.80		LL STATIONS A	AND OFFSET	S NOTED ARE	BASED OFF
	J	10+11.57	30 86 L	21.83	W	/EBSTER AVEN	UE CENTERI	LINE.	
-	<	10+24.01	16 79 L	21.74					
	L]	10+24.14	18 29 L	22.34					





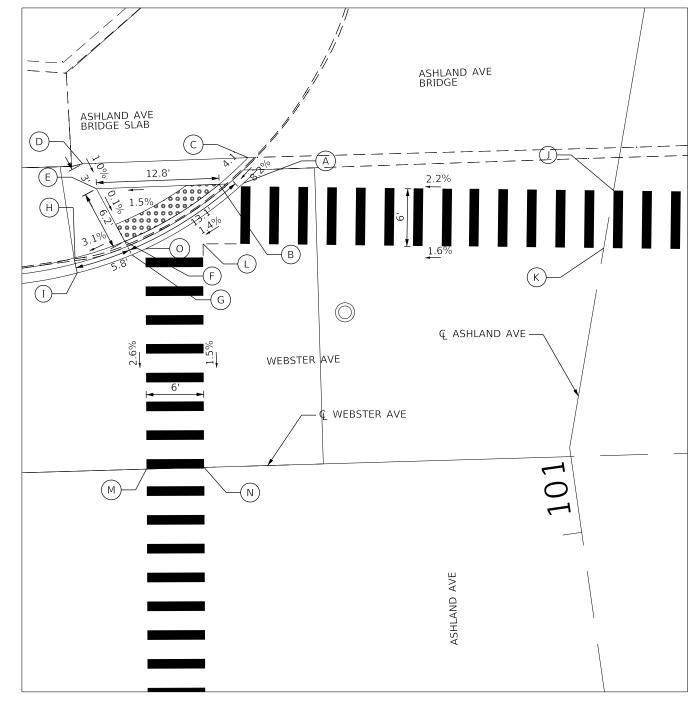
USER NAME = MMA	DESIGNED - MMA	REVISED -
	CHECKED - KSD	REVISED -
PLOT SCALE =	DRAWN - MMA	REVISED -
PLOT DATE = \$DATE\$	CHECKED - KSD	REVISED -

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	C-10
CDOT	PROJECT NO. E-1-525		18 of 210

WEBSTER AVE & ASHLAND AVE - NW CORNER



	STATION	OFFSET	ELEVATION
Α	9+30.74	29.48 L	21.28
В	9+28.22	29 44 L	21.28
С	9+31.28	32 13 L	21.62
D	9+14.09	32.00 L	21.12
Е	9+15.42	29 35 L	21.09
F	9+18.20	23.79 L	21.09
G	9+18.91	22.36 L	21.09
Н	9+12.87	22.20 L	20.91
I	9+13.15	20 64 L	20.33
J	9+69.80	30 21 L	20.42
Κ	9+68.59	24 18 L	21.84
L	9+21.15	23 30 L	21.18
M	9+19.77	00.00 L	20.51
N	9+25.77	00 00 L	20.84
0	9+20.34	22.94 L	21.11

NOTES: ALL STATIONS AND OFFSETS NOTED ARE BASED OFF WEBSTER AVENUE CENTERLINE.

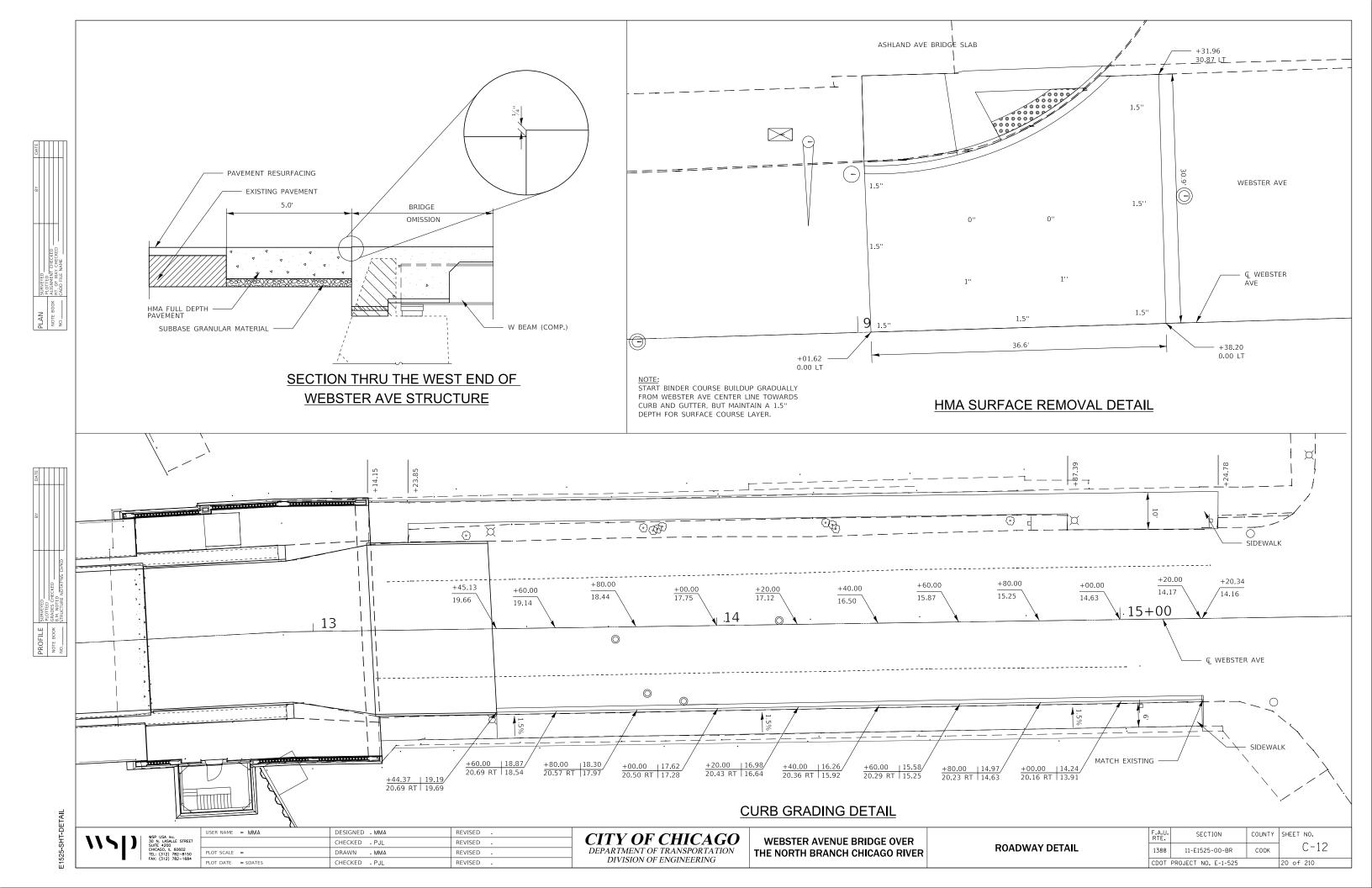
CHECKED - KSD REVISED DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

ADA RAMPS DETAILS

COUNTY SHEET NO. C-11 1388 11-E1525-00-BR соок CDOT PROJECT NO. E-1-525

MA	DESIGNED - MMA	REVISED -	CITY OF CHICAC
	CHECKED - KSD	REVISED -	CITY OF CHICAG
	DRAWN - MMA	REVISED -	DEPARTMENT OF TRANSPORTATION
			DIVISION OF ENGINEEDING



MAINTENANCE OF TRAFFIC AND DETOUR GENERAL NOTES

- . THE CONTRACTOR'S ATTENTION IS DIRECTED TO THE FACT THAT WHEN WORK COMMENCES, THE CONTRACTOR SHALL ASSUME THE MAINTENANCE OF ANY PAVEMENT, DRAINAGE FACILITIES, TRAFFIC CONTROL SIGNS, TRAFFIC SIGNALS, LIGHTING, PAVEMENT MARKINGS, AND OTHER APPURTENANCES OF ROADWAYS WITHIN THE LIMITS OF THE CONTRACT WHICH ARE USED BY THE PUBLIC DURING CONSTRUCTION.
- 2. THE CONTRACTOR SHALL RETAIN THIS MAINTENANCE RESPONSIBILITY UNTIL THE CITY ASSUMES THE MAINTENANCE. THE NEED FOR SNOW AND ICE CONTROL FOR THE DETOUR ROUTE DURING THE CONSTRUCTION PERIOD WILL BE ACCOMMODATED FOR BY OTHERS.
- 3. DETOUR TRAFFIC CONTROL SIGNS SHALL BE INSTALLED AT THE DIRECTION AND UNDER THE SUPERVISION OF THE ENGINEER. SEVEN (7) DAYS NOTICE SHALL BE GIVEN TO THE ENGINEER.
- 4. CHANGEABLE MESSAGE SIGN ONE EACH SHALL BE PLACED AT EASTBOUND AND WESTBOUND WEBSTER AVENUE AS DIRECTED BY THE ENGINEER. ALL CHANGEABLE MESSAGE SIGNS SHALL BE INSTALLED TWO WEEKS PRIOR TO ANY CONSTRUCTION ACTIVITY.
- 5. THE CONTRACTOR SHALL BE REQUIRED TO PROVIDE AND MAINTAIN ACCESS TO ALL PRIVATE AND COMMERCIAL PROPERTY WITHIN THE WORK AREAS DURING THE CONSTRUCTION PERIOD, WHICH MAY INCLUDE THE PROVISION OF TEMPORARY AGGREGATE IN THE WORK ZONE TO ALLOW TRUCKS TO TURN INTO LOADING BAYS.
- 6. DURING CONSTRUCTION OPERATIONS, THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO PROTECT ADJACENT TRAFFIC LANES OPEN TO TRAFFIC FROM DEBRIS BEING BLOWN OR OTHERWISE REMOVED FROM THE CONSTRUCTION AREAS. THE CONTRACTOR SHALL ALSO BE RESPONSIBLE FOR KEEPING DEBRIS OFF THE ADJACENT TRAVELED LANE SURFACE.
- 7. THE CONTRACTOR'S VEHICLES SHALL ALWAYS MOVE WITH AND NOT AGAINST OR ACROSS THE FLOW OF TRAFFIC. THESE VEHICLES SHALL ENTER AND LEAVE WORK AREAS IN A MANNER THAT WILL NOT BE HAZARDOUS TO OR INTERFERE WITH NORMAL TRAFFIC AND SHALL NOT STOP OR PARK EXCEPT WITHIN DESIGNATED WORK AREAS. PERSONAL VEHICLES SHALL NOT BE PERMITTED TO PARK WITHIN THE RIGHT-OF-WAY EXCEPT IN SPECIFIC AREAS DESIGNATED BY THE COMMISSIONER.
- 8. THE CONTRACTOR SHALL WALK THROUGH THE JOB BEFORE CONSTRUCTION ALONG WITH AUTHORIZED REPRESENTATIVE OF THE CITY OF CHICAGO TO CONFIRM THE COUNT OF STREET SIGNS. CONTRACTOR SHALL NOTIFY THE CITY OF CHICAGO AT LEAST 48 HOURS PRIOR TO SCHEDULE THE WALK THROUGH.
- ALL CONSTRUCTION SIGNS, BARRICADES, LIGHT AND FLAGGER SHALL BE PROVIDED BY THE CONTRACTOR.
- ADDITIONAL TRAFFIC CONTROL SIGNS MAY BE REQUIRED AS DIRECTED BY THE DIVISION OF INFRASTRUCTURE MANAGEMENT. THE COST SHALL BE CONSIDERED INCLUDED IN THE COST OF TRAFFIC CONTROL AND PROTECTION.
- 11. ALL SIGNS SHALL MEET MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES STANDARDS AND SHALL MEET WITH APPROVAL OF THE DIVISION OF INFRASTRUCTURE MANAGEMENT, REGARDING LOCATION, TYPE, SIZE, NUMBER, AND DURATION, OR AS DIRECTED BY THE ENGINEER.
- 12. ALL SIDEWALK CLOSURES SHALL FOLLOW CDOT STANDARD CROSSWALK AND SIDEWALK CLOSURE SHEET A-6-11. THE SIDEWALK SHALL BE OPEN ON AT LEAST ONE SIDE OF THE STREET AT ALL TIMES. THE CONTRACTOR SHALL INSTALL TEMPORARY ADA RAMPS WHEN EXISTING RAMPS CANNOT BE USED. THIS SHALL NOT BE PAID FOR SEPARATELY AND SHALL BE INCLUDED IN THE COST OF TRAFFIC CONTROL AND PROTECTION, SPECIAL.
- 13. AT LEAST ONE SIDEWALK SHALL BE OPEN AT ALL TIMES.
- 14. THE CONTRACTOR SHALL NOT BLOCK ACCESS TO DOMINICK STREET.
- 15. THE SUGGESTED TRAFFIC CONTROL CAN BE ALTERED WITH APPROVAL OF COMMISSIONER.
- 16. ALL DETOUR ROUTE SIGNS SHALL BE COVERED AFTER DETOUR IS REMOVED.
- 17. ALL WORK DESCRIBED IN GENERAL NOTES SHALL BE INCIDENTAL TO TRAFFIC CONTROL AND PROTECTION, SPECIAL UNLESS STATED OTHERWISE.

STAGING DESCRIPTION

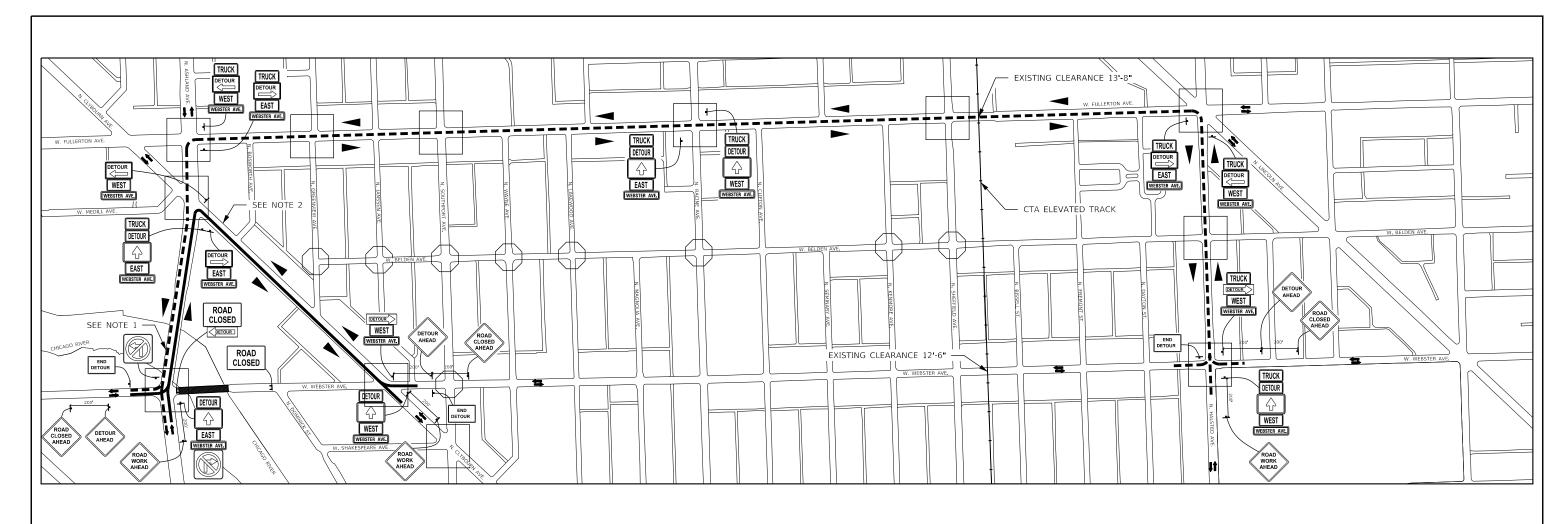
WORK: WEBSTER BRIDGE TRAFFIC: DETOUR

WORK: ADA RAMPS AND TRAFFIC SIGNAL POLE ON ASHLAND TRAFFIC: SINGLE LANE CLOSURE (OFF PEAK) ON ASHLAND.

OFF PEAK LANE CLOSURE HOURS				
ASHLAND AVENUE	9:30 AM TO 3:30 PM MONDAY THROUGH FRIDAY			
	8:00 PM TO 6:00 AM EVERY NIGHT			
	EXCEPT HOLIDAY/ HOLIDAY WEEKENDS			
ASHLAND AVENUE	8:00 PM TO 6:00 AM MONDAY THROUGH FRIDAY			
	EXCEPT HOLIDAY/ HOLIDAY WEEKENDS			

USER NAME = jturk	DESIGNED - JJT	REVISED -
	CHECKED - SA	REVISED -
PLOT SCALE = N.T.S.	DRAWN - JJT	REVISED -
PLOT DATE = 12/10/2020	CHECKED - SA	REVISED -

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	MOT-1
CDOT	PROJECT NO. E-1-525		21 OF 210





SIGNALIZED INTERSECTION



DETOUR M4-10L





M4-10R

INTERSECTION

M4-8







R11-2

WEST



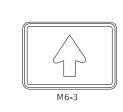










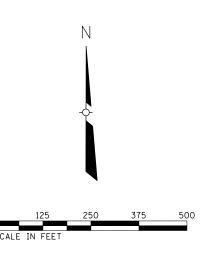






NOTES

- INSTALL BARRICADES TO CLOSE SOUTHBOUND LEFT TURN LANE AT INTERSECTION OF ASHLAND AVENUE AND WEBSTER AVENUE.
- RESTRICT PARKING ALONG NORTH SIDE OF CLYBOURN AVENUE BETWEEN ASHLAND AVENUE AND BOSWORTH AVENUE. INSTALL TEMPORARY LEFT TURN BAY FOR THE NORTHWESTBOUND LEFT TURN AT INTERSECTION OF CLYBOURN AVENUE AND ASHLAND



LEGEND

TT TYPE III BARRICADE

CONSTRUCTION ZONE

--- TRUCK DETOUR ROUTE

PASSENGER VEHICLE DETOUR ROUTE

LOCAL TRAFFIC DIRECTION

TRAFFIC DIRECTION/DETOUR ROUTE



	CITTLE OF CITTLE ! CO
	CITY OF CHICAGO
ı	
	DEPARTMENT OF TRANSPORTATION
	DIVISION OF ENGINEERING

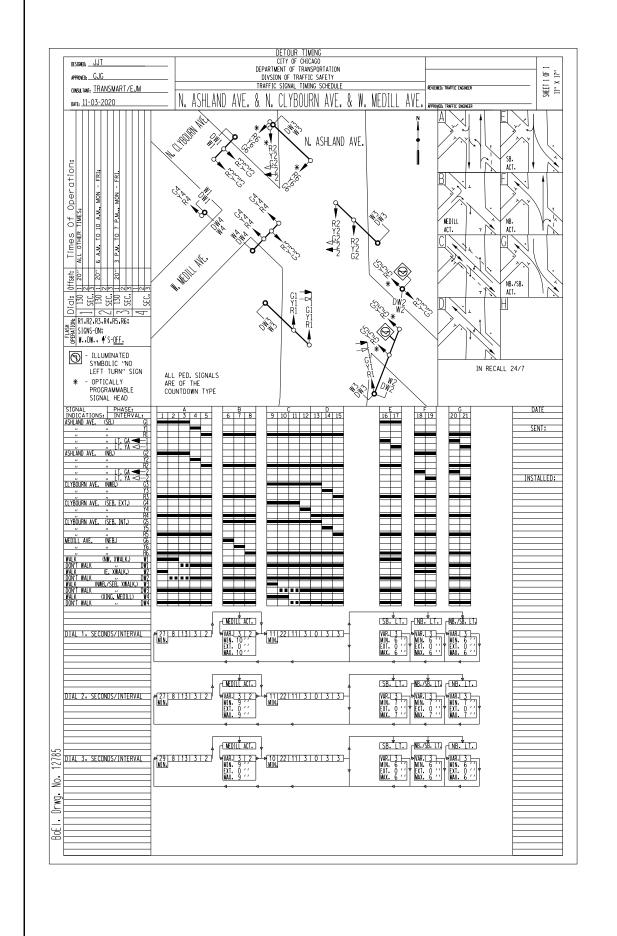
SECTION COUNTY SHEET NO. **DETOUR PLAN** MOT-2 11-E1525-00-BR соок CDOT PROJECT NO. E-1-525 22 OF 210

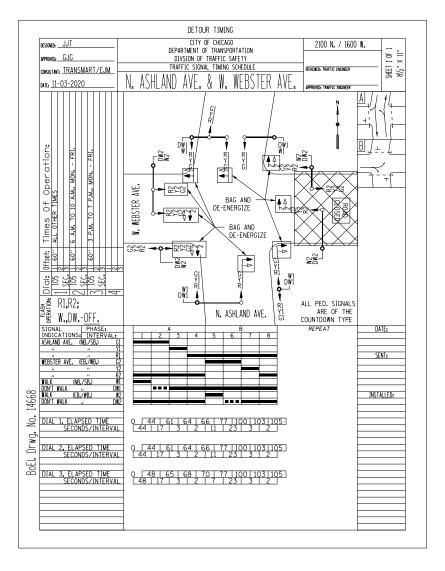
ranSmat(ใยJM

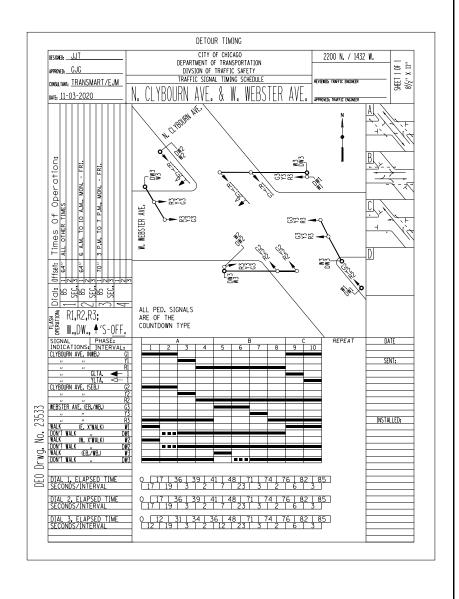
USER NAME = jturk DESIGNED - JJT REVISED CHECKED -SA REVISED LOT SCALE = N.T.S. DRAWN JJT REVISED PLOT DATE = 12/10/2020 CHECKED -SA REVISED

THE NORTH BRANCH CHICAGO RIVER

(STRUCTURE NO. 016-6057)







TranSmart/EJM Sul

CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

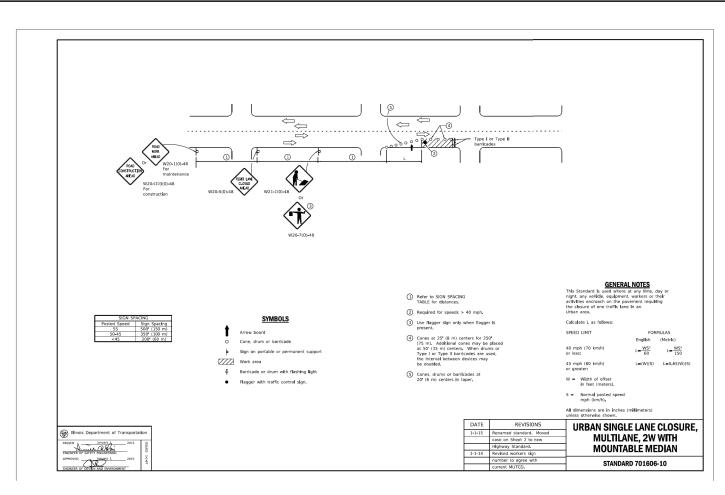
WEBSTER AVENUE BRIDGE OVER
THE NORTH BRANCH CHICAGO RIVER

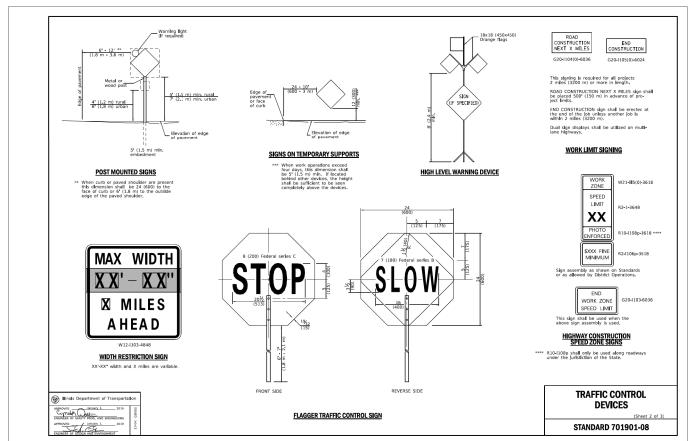
TRAFFIC SIGNAL
TIMING SCHEDULES
(STRUCTURE NO. 016-6057)

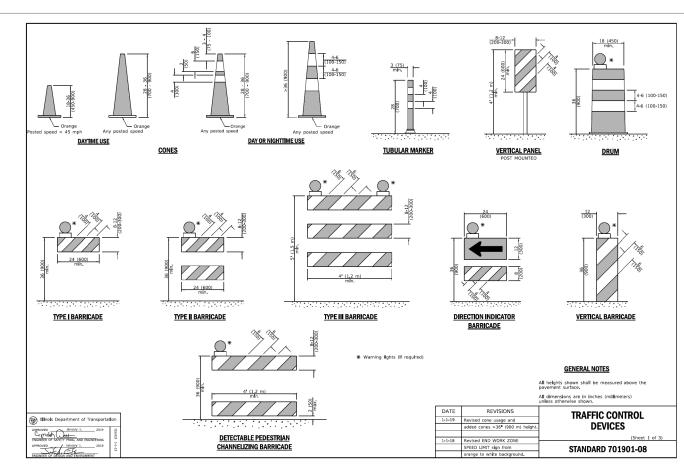
F.A.U. SECTION COUNTY SHEET NO.

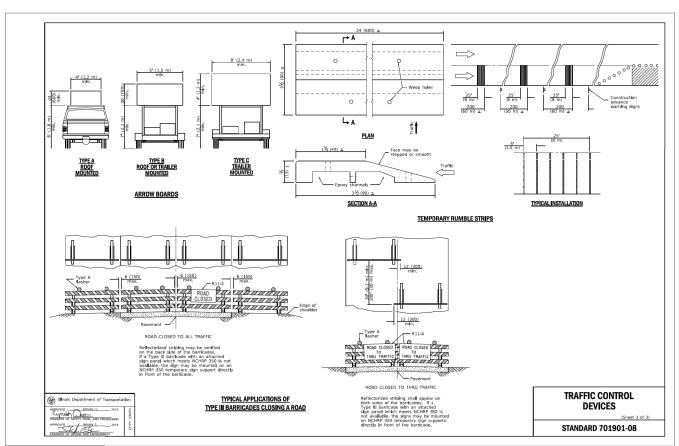
1388 11-E1525-00-BR COOK MOT - 3

CDOT PROJECT NO. E-1-525 23 0F 210











USER NAME = jturk DESIGNED - JJT REVISED CHECKED - SA REVISED LOT SCALE = N.T.S. DRAWN JJT REVISED PLOT DATE = 12/10/2020 CHECKED -SA REVISED

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

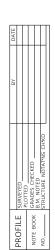
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

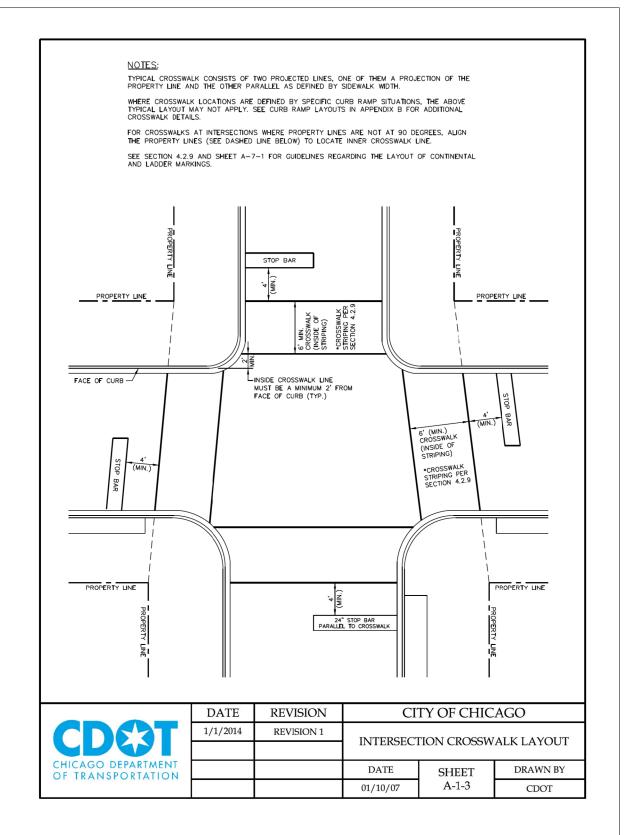
SECTION COUNTY SHEET NO. 11-E1525-00-BR соок CDOT PROJECT NO. E-1-525

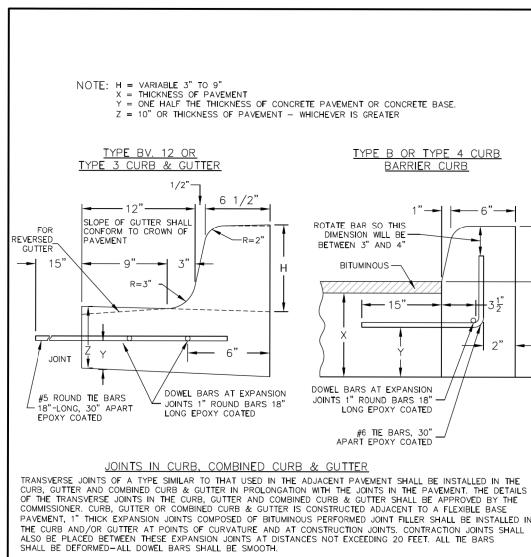
MOT-4

24 OF 210









COMMISSIONER. CURB, GUTTER OR COMBINED CURB & GUTTER IS CONSTRUCTED ADJACENT TO A FLEXIBLE BASE PAVEMENT, 1" THICK EXPANSION JOINTS COMPOSED OF BITUMINOUS PERFORMED JOINT FILLER SHALL BE INSTALLED IN THE CURB AND/OR GUTTER AT POINTS OF CURVATURE AND AT CONSTRUCTION JOINTS, CONTRACTION JOINTS SHALL ALSO BE PLACED BETWEEN THESE EXPANSION JOINTS AT DISTANCES NOT EXCEEDING 20 FEET. ALL TIE BARS

> NOTE: ALL TIE BARS AND DOWEL BARS TO BE EPOXY COATED.

*AT LOCATIONS REQUIRING DEPRESSED CURBS SEE THE ADA STANDARDS FOR CONSTRUCTION DETAILS

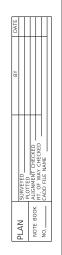
	DC3T
	CAGO DEPARTMENT
OF	TRANSPORTATION .

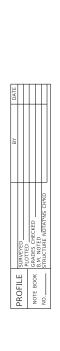
DATE	REVISION	CITY OF CHICAGO			
1/1/2014	REVISION 1	CONCRETE CURB		gUTTER	
		DATE	SHEET	DRAWN BY	
		12/12/06	A-2-6	CDOT	

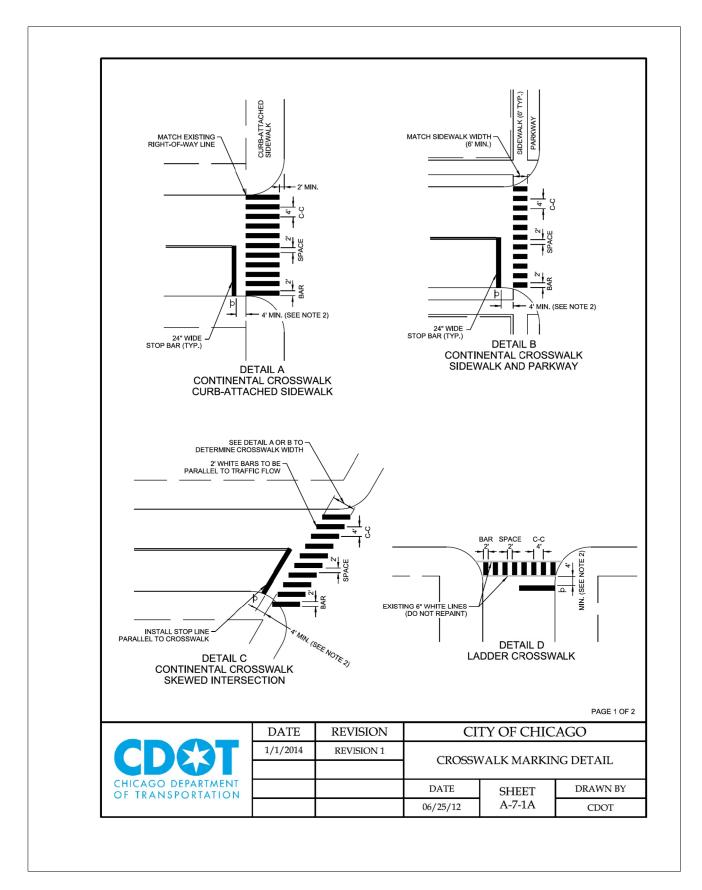
USER NAME = MMA	DESIGNED - MMA	REVISED -
	CHECKED -	REVISED -
PLOT SCALE =	DRAWN - MMA	REVISED -
PLOT DATE = \$DATE\$	CHECKED -	REVISED -

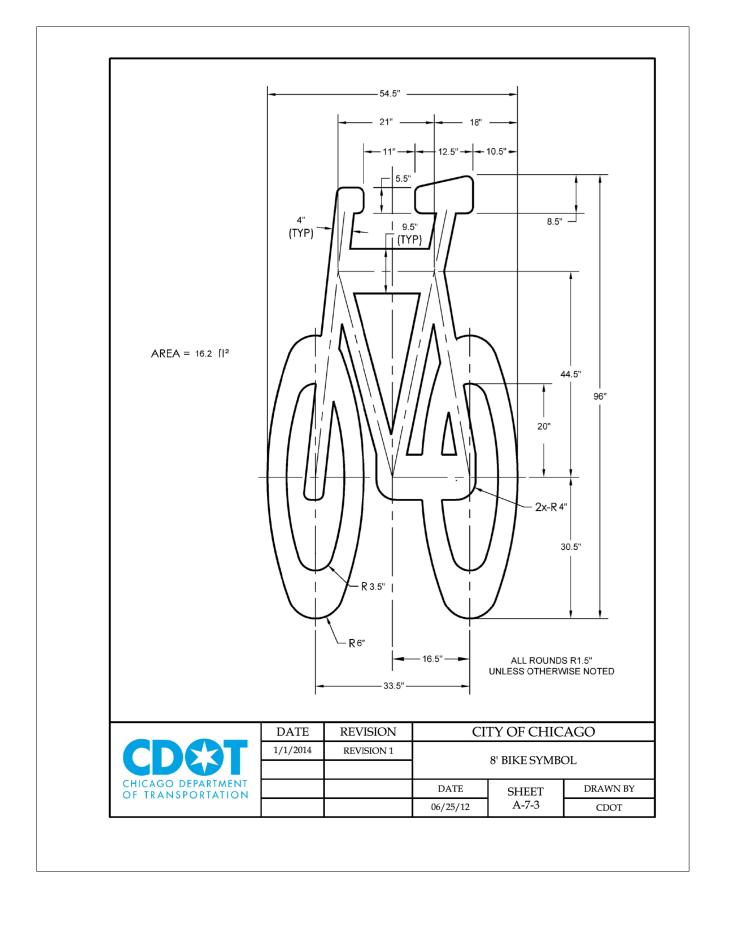
SD-1

25 of 210





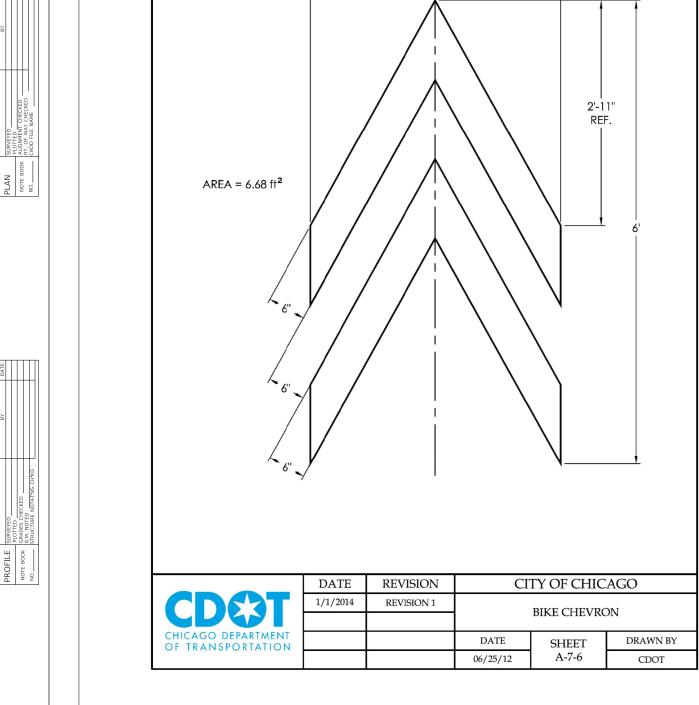


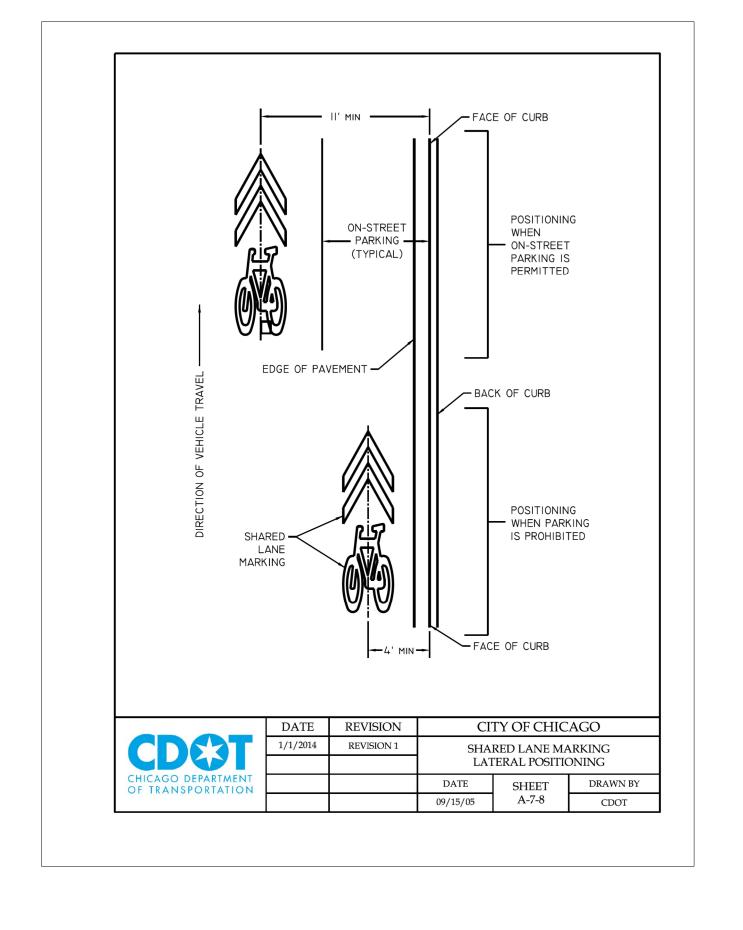


11	5)	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684

USER NAME = MMA	DESIGNED - MMA	REVISED -
	CHECKED -	REVISED -
PLOT SCALE =	DRAWN - MMA	REVISED -
PLOT DATE = \$DATE\$	CHECKED -	REVISED -









USER NAME = MMA PLOT DATE = \$DATE\$

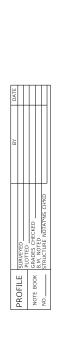
DESIGNED - MMA REVISED CHECKED -REVISED DRAWN REVISED CHECKED REVISED

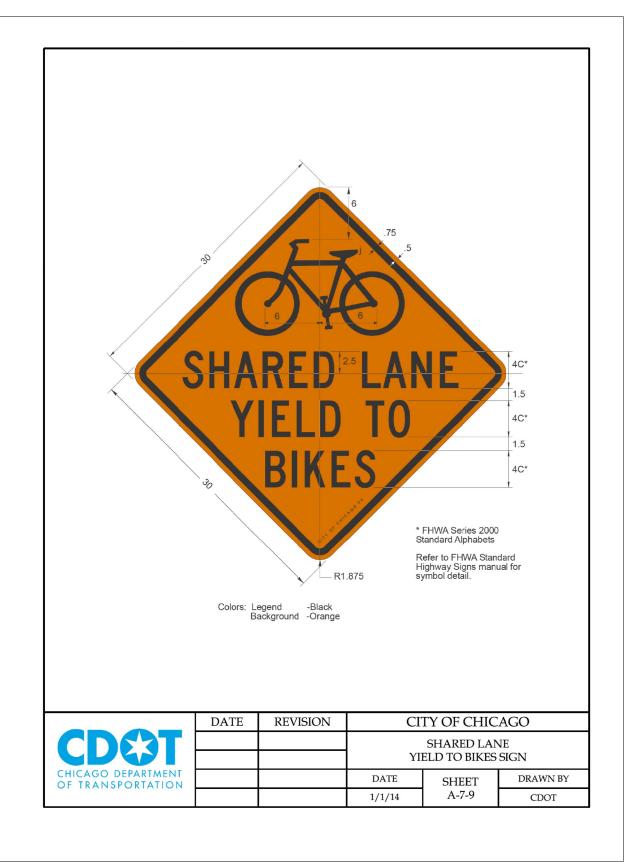
CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

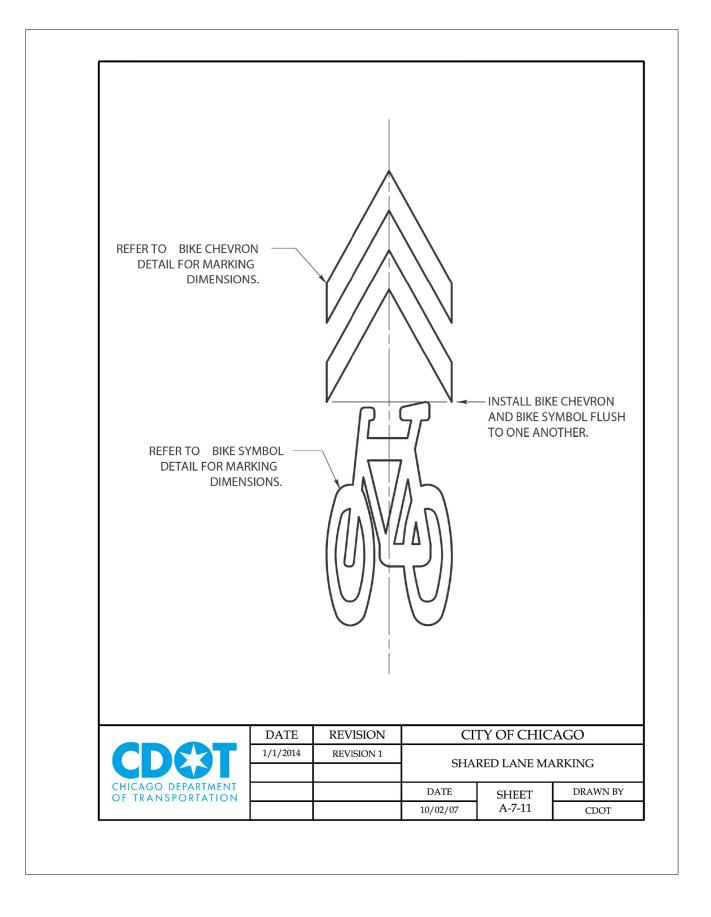
THE NORTH BRANCH CHICAGO RIVER

COUNTY SHEET NO. SECTION SD-3 1388 11-E1525-00-BR СООК CDOT PROJECT NO. E-1-525 27 of 210





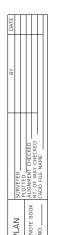


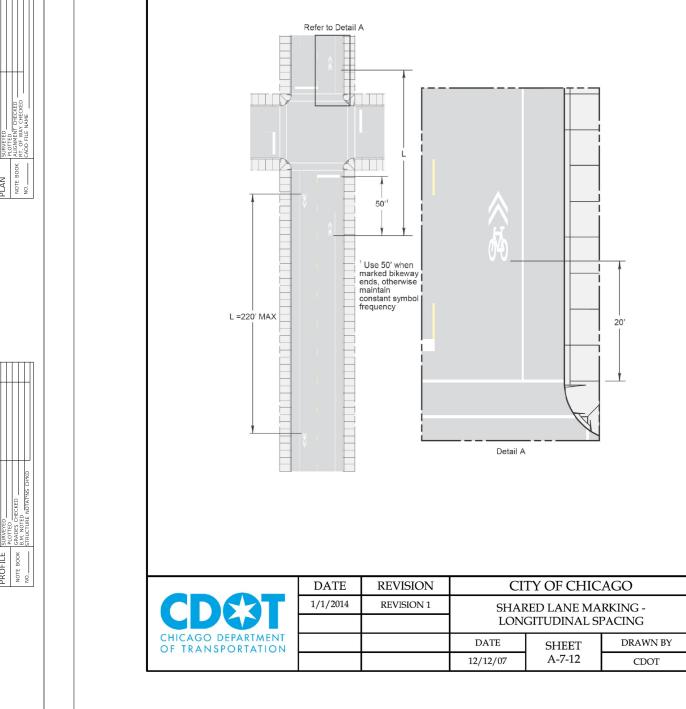


115)	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684

R NAME = MMA	DESIGNED - MMA	REVISED -
	CHECKED -	REVISED -
T SCALE =	DRAWN - MMA	REVISED -
T DATE = \$DATE\$	CHECKED -	REVISED -

CDOT STANDARDS





USER NAME = MMA PLOT DATE = \$DATE\$

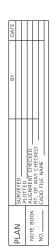
DESIGNED - MMA REVISED CHECKED -REVISED DRAWN - MMA REVISED CHECKED REVISED

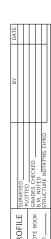
CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

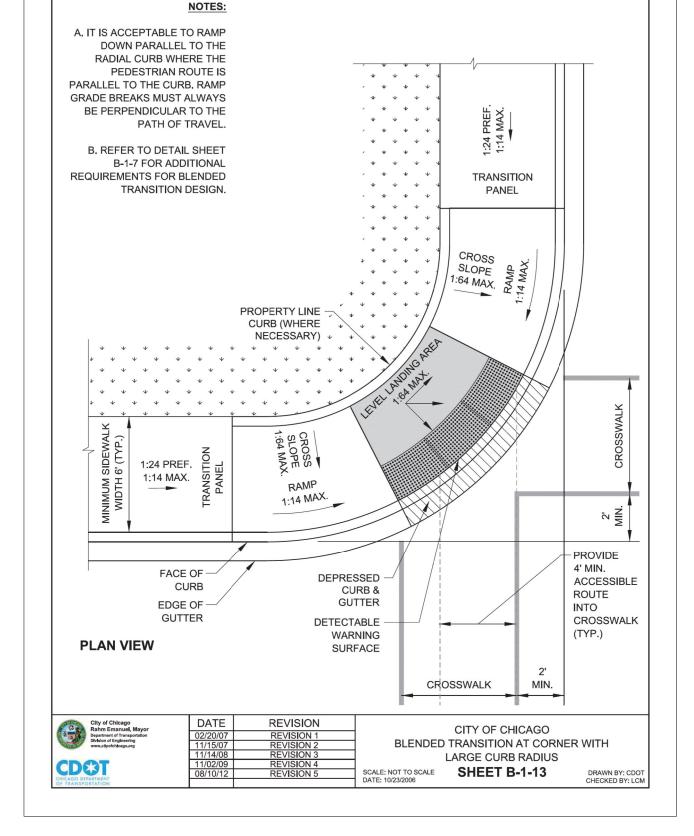
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

CDOT STANDARDS

COUNTY SHEET NO. SECTION SD-5 1388 11-E1525-00-BR соок 29 of 210 CDOT PROJECT NO. E-1-525







GENERAL NOTES:

- 1. THE DETECTABLE WARNING USED SHALL BE CHOSEN FROM THE CHICAGO DEPARTMENT OF TRANSPORTATION LIST OF APPROVED DETECTABLE WARNING PRODUCTS (AVAILABLE ON THE CITY OF CHICAGO WEBSITE). IT IS NOT ACCEPTABLE TO INSTALL TWO DIFFERENT DETECTABLE WARNING PRODUCTS ADJACENT TO ONE ANOTHER AT ANY LOCATION. IN THE CENTRAL BUSINESS DISTRICT, GRANITE OR OTHER SPECIALTY PAVING MATERIALS MAY BE SUBMITTED TO THE COMMISSIONER FOR APPROVAL.
- 2. THE DETECTABLE WARNING MUST BE INSTALLED A MAXIMUM OF 8" OR LESS FROM FACE OF CURB (SEE DETAIL SHEET B-4-2).
- 3. THE DETECTABLE WARNING MUST COVER FULL WIDTH OF RAMP EXCLUDING SIDE FLARES FOR A MINIMUM UNOBSTRUCTED DEPTH OF 24". THE DETECTABLE WARNING LOCATED ON THE SURFACES OF RAMPS IS TYPICALLY ORIENTED PERPENDICULAR TO THE RUN OF THE RAMP UNLESS SPECIAL CIRCUMSTANCES OCCUR (SEE DETAIL SHEET B-1-5). THE DETECTABLE WARNING MUST BE PROVIDED FOR A MINIMUM DEPTH OF 24" FOR THE ENTIRE LENGTH OF THE SIDEWALK WHERE THE SIDEWALK IS FLUSH WITH THE STREET (DEPRESSED CURB OR FLUSH TRANSITION). IF IT IS NECESSARY TO CUT A UNIT(S) IN THE PROVISION OF A COMPLIANT RAMP OR SIDEWALK WITH 24" MINIMUM DEPTH OF DETECTABLE WARNING. THE UNITS SHALL BE CUT IN A NEAT AND WORKMAN LIKE MANNER PER MANUFACTURER'S REQUIREMENTS WITH A MINIMUM OF THREE PINS OR ANCHOR POINTS (WHERE APPLICABLE). THE UNITS SHALL BE ARRANGED SO THAT THE CUT UNITS ARE LARGE ENOUGH TO BE PROPERLY AND ADEQUATELY SECURED. CUT UNITS SHALL NOT BE USED UNLESS ALL OTHER DESIGN OPTIONS HAVE BEEN EXHAUSTED. THE USE OF SALVAGE PIECES FROM UNITS THAT ARE CUT WILL NOT BE PERMITTED WITHOUT WRITTEN APPROVAL OF THE COMMISSIONER. CUT UNIT SALVAGE PIECES NOT APPROVED FOR USE MUST BE REMOVED FROM THE SITE AND DISPOSED OF PROPERLY.
- 4. WHERE APPLICABLE, A COMBINATION OF STRAIGHT AND RADIAL DETECTABLE WARNING UNITS MAY BE USED ON COMPOUND AND LARGE RADII. CONTRACTOR MUST MAKE THIS DETERMINATION AND VERIFY IN FIELD.
- 5. THE DETECTABLE WARNING MUST CONTRAST WITH ADJACENT PAVEMENT. IF LIGHT COLORED PAVEMENT IS USED THE DETECTABLE WARNING COLOR SHALL BE RED. IF A DARK COLORED PAVEMENT IS USED THE DETECTABLE WARNING COLOR SHALL BE YELLOW. CONTRACTOR TO VERIFY THAT PROPER CONTRAST IS OBTAINED.
- 6. PRIOR TO PLACING CONCRETE FOR DEPRESSED CURBS, RAMPS, OR SIDEWALKS THE CONTRACTOR SHALL VERIFY THAT LAYOUT OR DESIGN COMPLIES WITH THE REQUIREMENTS OF THE CDOT ADA STANDARDS.
- 7. RAMP WIDTH MUST BE A MINIMUM OF 6'-0" AND IN INCREMENTS OF 1'-0", EXCEPT WHEN USING THE PERPENDICULAR RAMP AT CORNER (OR OTHER SPECIAL CDOT APPROVED CONDITIONS), WHICH HAS A MINIMUM WIDTH OF 4'-0".
- 8. THE MAXIMUM ALLOWABLE RAMP RUNNING SLOPE IS 1:14, MEASURED AT ANY PORTION OF THE RAMP. IF POSSIBLE, A MORE GRADUAL SLOPE SHALL BE USED. GRADE BREAKS AT THE TOP AND BOTTOM OF RAMPS SHALL BE PERPENDICULAR TO THE DIRECTION OF RAMP RUN.
- 9. THE MAXIMUM ALLOWABLE RAMP CROSS SLOPE IS 1:64. MEASURED AT ANY PORTION OF THE RAMP. IF POSSIBLE, A MORE GRADUAL SLOPE SHALL BE USED.
- 10. THE MAXIMUM ALLOWABLE RAMP LANDING SLOPE IS 1:64, MEASURED AT ANY LOCATION AND IN ANY DIRECTION ON THE LANDING. THE RAMP LANDING WIDTH SHALL MATCH THE FULL WIDTH OF THE RAMP FOR A MINIMUM UNOBSTRUCTED DEPTH OF 4'-0". RAMP LANDINGS SHALL BE PROVIDED AT THE TOP AND/OR BOTTOM OF RAMPS WHERE TURNING IS REQUIRED.
- 11. RAMP SIDE FLARES SHALL BE INSTALLED AT ANY LOCATION WHERE THE SURFACE ADJACENT TO THE RAMP SURFACE IS INTENDED FOR PEDESTRIAN USE. TRIPPING HAZARDS, INCLUDING STEPS, DROP-OFFS, OR CURBS SHALL NOT BE LOCATED WITHIN THE LIMITS OF THE SIDEWALK. RAMP SIDE FLARES ARE NOT REQUIRED WHERE THE SURFACE ADJACENT TO THE RAMP SURFACE IS LANDSCAPED OR IS OCCUPIED BY A BARRIER THAT BLOCKS PEDESTRIAN ACCESS. EXCEPTIONS TO THIS RULE MAY BE SUBMITTED TO THE COMMISSIONER FOR APPROVAL.



DATE	REVISION
02/20/07	REVISION 1
11/15/07	REVISION 2
11/14/08	REVISION 3
11/02/09	REVISION 4
08/10/12	REVISION 5

CITY OF CHICAGO **GENERAL NOTES**

SHEET B-3-2 SCALE: NOT TO SCALE DRAWN BY: CDOT CHECKED BY: LCM

		·	
USER NAME = MMA	DESIGNED - MMA	REVISED -	
	CHECKED -	REVISED -	
PLOT SCALE =	DRAWN - MMA	REVISED -	
PLOT DATE = \$DATE\$	CHECKED -	REVISED -	

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	SD-6
CDOT	PROJECT NO. E-1-525		30 of 210

GENERAL NOTES (CONTINUED):

- 12. UTILITIES, SUCH AS LIGHT POLES, TRAFFIC POLES AND HYDRANTS, MAY BE LOCATED IN THE FLARE OF THE RAMP BUT ARE NOT ALLOWED ON THE RAMP SURFACE OR LANDING AREAS. EXISTING UTILITY STRUCTURE LIDS MAY REMAIN WITHIN THE FLARE OR ON THE SURFACE OF THE RAMP IF THE REQUIREMENTS OF GENERAL NOTE #19 ARE MET.
- 13. ALL LOCATIONS WITH TYPE 4 OR TYPE B CURB (EXCEPT ALLEY APRONS) SHALL BE CONSTRUCTED AS CURB AND GUTTER TYPE BV.12 THROUGH THE LIMITS OF THE CORNER AND THE CURB RAMPS.
- 14. ALTERATIONS SHALL NOT DECREASE THE ACCESSIBILITY TO EXISTING FACILITIES, SIDEWALKS LEADING TO EXISTING FACILITIES, OR DOOR OR GATE ACCESS POINTS TO FACILITIES. THE ELEVATION AT THE EXISTING PROPERTY LINE OR FACILITY ACCESS POINT SHALL BE MAINTAINED AT A MINIMUM. ANY ALTERATIONS ADJACENT TO OR AFFECTING A FACILITY ACCESS POINT SHALL RESULT IN IMPROVED ACCESS OR AT A MINIMUM A REPLICATION OF EXISTING CONDITIONS, INCLUDING SIDEWALK SLOPES AND SURFACE CONDITIONS. FACILITIES INCLUDE, BUT ARE NOT LIMITED TO PRIVATE BUSINESSES, PUBLIC BUILDINGS, RESIDENCES, BUS STOPS, PUBLIC BENCHES, PAY PHONES, AND PARKING METERS.
- 15. THE MINIMUM CROSSWALK WIDTH IS 6'-0". CROSSWALKS SHALL BE LOCATED AS SHOWN IN THE PLAN SHEETS DEPENDING ON THE TYPE OF CURB RAMP USED. BEYOND THE CURB FACE AT THE BASE OF CURB RAMPS, A CLEAR SPACE OF 4'-0" BY 4'-0" MINIMUM SHALL BE PROVIDED WITHIN THE STRIPES OF THE CROSSWALK (WHERE PROVIDED).
- 16. IF SIDEWALK AND ALLEY ARE AT THE SAME GRADE, A RAMP IS NOT REQUIRED. IF SIDEWALK AND DRIVEWAY ARE AT THE SAME GRADE, A RAMP IS NOT REQUIRED BUT DETECTABLE WARNING UNITS ARE STILL REQUIRED IF THE DRIVEWAY HAS TRAFFIC CONTROL DEVICES (I.E. TRAFFIC SIGNALS).
- 17. MAIN LINE SIDEWALK SHALL HAVE A MAXIMUM CROSS SLOPE NOT TO EXCEED 1:64 FOR THE FULL WIDTH OF WALK UNLESS OTHERWISE APPROVED BY THE COMMISSIONER. WHERE TURNING IS REQUIRED AND WHERE SIDEWALKS INTERSECT, THE SLOPE OF THE SIDEWALK SHALL NOT EXCEED 1:64 IN ANY DIRECTION.
- 18. MAIN LINE SIDEWALK RUNNING SLOPES SHALL NOT EXCEED 1:24 OR THE GENERAL GRADE ESTABLISHED FOR THE ADJACENT STREET, WHICH EVER IS HIGHER.
- 19. THERE SHALL BE NO VERTICAL LEVEL DIFFERENCES BETWEEN SURFACES GREATER THAN 1/4" ON THE MAIN LINE SIDEWALK. THERE SHALL BE NO HORIZONTAL GAPS OR OPENINGS GREATER THAN 1/2" ON THE MAIN LINE
- 20. WHERE OBSTRUCTIONS EXIST ON THE MAINLINE SIDEWALK, THE CLEAR WIDTH OF USEABLE SIDEWALK SHALL NOT BE LESS THAN 4'-0". OBSTRUCTIONS INCLUDE, BUT ARE NOT LIMITED TO SIDEWALK BENCHES, FIRE HYDRANTS, SIGNAL OR LIGHT POLES, NEWSPAPER DISPENSERS, TRASH RECEPTACLES, AND UTILITY PEDESTALS.
- 21. CURB RAMPS AND LANDING (KEYSTONE) TO BE CONSTRUCTED WITH 8" THICK CONCRETE AT ALL TRAFFIC SIGNALIZED INTERSECTIONS AND INDUSTRIAL STREET INTERSECTIONS. AT ALL OTHER LOCATIONS, 5" THICK CONCRETE TO BE USED.
- 22. DEPRESSED CURB, RAMP, OR SIDEWALK DESIGNS OR LAYOUTS SHALL MAINTAIN OR IMPROVE EXISTING DRAINAGE AND THE EXISTING INTERSECTION GEOMETRY SHALL NOT BE MODIFIED WITHOUT CDOT APPROVAL.
- 23. ALL CONSTRUCTION DOCUMENTS MUST BE STAMPED BY A LICENSED ARCHITECT/LANDSCAPE ARCHITECT/ ENGINEER TO CERTIFY THAT THEY ARE IN ACCORDANCE WITH THE AMERICANS WITH DISABILITIES ACT (ADA) AND ALL CODES AND BUILDING ORDINANCES OF THE CITY OF CHICAGO AND THE STATE OF ILLINOIS.
- 24. NO DEVIATIONS FROM THESE STANDARDS ARE ALLOWED WITHOUT WRITTEN APPROVAL FROM THE COMMISSIONER.

REVISION

		_
O GOOD	City of Chicago	
100 m	Rahm Emanuel, Mayor	
men Pill	Department of Transportation	
3	Division of Engineering	
Just 3	www.cityofchicago.org	

REVISION 1 REVISION 2 REVISION 3 11/14/08 CDØT **REVISION 5**

DATE

CITY OF CHICAGO **GENERAL NOTES (CONTINUED)**

SHEET B-3-3 SCALE: NOT TO SCALE

DRAWN BY: CDO

USER NAME = MMA	DESIGNED - MMA	REVISED -
	CHECKED -	REVISED -
PLOT SCALE =	DRAWN - MMA	REVISED -
PLOT DATE = \$DATE\$	CHECKED -	REVISED -

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	SD-7
CDOT	PROJECT NO. E-1-525		31 of 210

ADA COMPLIANCE AND TRANSITION GUIDELINES

POLICY STATEMENT: ANY ALTERATION OF THE PUBLIC WAY MUST BE RESTORED IN AN ADA COMPLIANT MANNER

I. STREET/ALLEY RESTORATION

FOR ANY PROJECT WHERE, WITHIN THE PROJECT LIMITS, A CROSSWALK IS ENCOUNTERED OR WHERE THE PROJECT LIMITS TERMINATE WITHIN 4' OR LESS OF A CROSSWALK, THOSE CROSSWALKS AND THE ASSOCIATED CURB RAMPS MUST BE IMPROVED TO CURRENT ADA STANDARDS IF THEY ARE NOT COMPLIANT (SEE APPENDIX A.)

WHEN A PROJECT CALLS FOR ONLY AN INTERSECTION TO BE REPAVED, THE INTERSECTION LIMITS AS DEFINED BY THE AREA OUTLINED BY OUTERMOST CROSSWALK LINES AND ADJACENT CURB FACES AND ALL ADJOINING CROSSWALKS AND CURB RAMPS MUST BE IMPROVED TO CURRENT ADA STANDARDS IF THEY ARE NOT COMPLIANT (SEE APPENDIX A).

WHEN WORK IS LIMITED TO A SINGLE CORNER OF AN INTERSECTION, THE CURB RAMP MUST BE IMPROVED TO CURRENT ADA STANDARDS AND THE ADJACENT PAVEMENT MUST BE RESURFACED, AS NECESSARY TO PROVIDE FOR A FLUSH TRANSITION (SEE APPENDIX A).

WHEN ADA WORK IS LIMITED TO A SINGLE CORNER OF AN INTERSECTION, THE ADJACENT PAVEMENT MUST BE RESTORED TO THE 1/4-POINT OF THE ROADWAY.

FOR ANY CONSTRUCTION WHERE, WITHIN THE PROJECT LIMITS, AN ALLEY APRON IS ENCOUNTERED, THE ASSOCIATED CURB RAMPS, ALLEY APRON, AND SIDEWALKS MUST BE IMPROVED TO CURRENT ADA STANDARDS IF THEY ARE NOT COMPLIANT (SEE APPENDIX A).

II. SIDEWALK INSTALLATION / REPAIRS / RECONSTRUCTION

THE LIMITS OF ANY MAINLINE SIDEWALK REPLACEMENT, GREATER THAN TEN FEET (10') IN LENGTH, THAT ABUT AN EXISTING RAMP, KEYSTONE, TRANSITION PANEL, AND/OR LANDING AREA (THIS TOTAL LENGTH INCLUDES THE PRIOR ELEMENTS), SHALL BE EXTENDED TO INCLUDE THE AFFECTED RAMPS AND THESE RAMPS SHALL BE RECONSTRUCTED TO CURRENT ADA STANDARDS. IN ADDITION, ALL NEWLY PLACED SIDEWALK TEN FEET (10') OR MORE IN LENGTH SHALL BE CONSTRUCTED IN ACCORDANCE WITH ALL CURRENT APPLICABLE STANDARDS WHICH INCLUDE PROVIDING A MINIMUM FOUR FEET (4') WIDTH ACCESSIBLE PATHWAY WITH A CROSS SLOPE NOT TO EXCEED 1:64 (SEE APPENDIX A).

III. GUIDELINES FOR TRANSITIONING TO EXISTING NON-COMPLIANT CONDITION

NEW SIDEWALK PLACEMENTS GREATER THAN TEN FEET IN CONTIGUOUS LENGTH:

THE LIMITS OF ANY MAINLINE SIDEWALK REPLACEMENT, GREATER THAN TEN FEET (10') IN LENGTH, MUST BE EXTENDED FOR A MINIMUM FIVE ADDITIONAL FEET (5') EITHER SIDE IN ORDER TO PROVIDE A TRANSITION TO MATCH THE EXISTING SIDEWALK. THE LENGTH OF TRANSITION SHALL BE LENGTHENED AS NECESSARY TO ENSURE THAT THE RUNNING SLOPE OF THE TRANSITION DOES NOT EXCEED A SLOPE OF 1:24 (PREFERRED) OR 1:14 (MAXIMUM) AT ANY POINT.

NEW SIDEWALK REPLACEMENTS TEN FEET OR LESS IN CONTIGUOUS LENGTH (REPAIRS):

IT IS ACCEPTABLE PRACTICE TO MATCH ADJACENT SIDEWALKS AT THE EXISTING SLOPE.

CURB RAMP REPLACEMENTS

WHEN REPLACING AN ADA RAMP, THE SIDEWALK REPLACEMENT MUST EXTEND BEYOND THE LIMITS OF THE LANDING AREA AND/OR THE "KEYSTONE" A MINIMUM OF AN ADDITIONAL FIVE FEET (5') ON EITHER SIDE IN ORDER TO PROVIDE A TRANSITION TO MATCH THE EXISTING SIDEWALK. THE TRANSITION PANEL SHALL BE LENGTHENED AS NECESSARY TO ENSURE THAT THE RUNNING SLOPE OF THE TRANSITION PANEL DOES NOT EXCEED A SLOPE OF 1:24 (PREFERRED) OR 1:14 (MAXIMUM) AT ANY POINT.

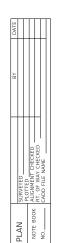
NO EXCEPTIONS TO THE ABOVE WILL BE ALLOWED WITHOUT WRITTEN APPROVAL FROM THE COMMISSIONER.

DATE	REVISION
02/20/07	REVISION 1
11/15/07	REVISION 2
11/14/08	REVISION 3
11/02/09	REVISION 4
08/10/12	REVISION 5
01/01/14	REVISION 6

CITY OF CHICAGO ADA COMPLIANCE AND TRANSITION GUIDELINES

SHEET B-3-4 SCALE: NOT TO SCALE

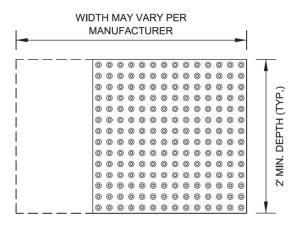
DRAWN BY: CDOT CHECKED BY: GK





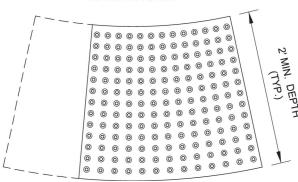


STRAIGHT DETECTABLE WARNING UNITS



RADIAL DETECTABLE WARNING UNITS

RADII & WIDTH MAY VARY PER MANUFACTURER



DETECTABLE WARNING UNIT SIZES

- VERIFY ALL DIMENSIONS WITH THE PRODUCT MANUFACTURER.
- IF USING RADIAL UNITS, VERIFY THAT THE CURB RADIUS MATCHES AVAILABLE UNIT RADII WITH THE PRODUCT MANUFACTURER.
- APPROVED LIST OF DETECTABLE WARNING PRODUCTS CAN BE FOUND ON CDOT'S WEBSITE (www.cityofchicago.org).



DATE	REVISION
02/20/07	REVISION 1
11/15/07	REVISION 2
11/14/08	REVISION 3
11/02/09	REVISION 4
08/10/12	REVISION 5

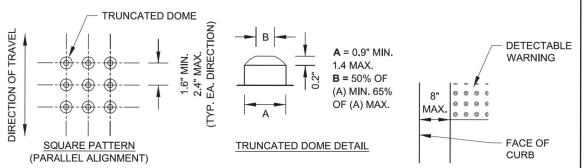
CITY OF CHICAGO DETECTABLE WARNING UNIT SIZES

SHEET B-4-1

DRAWN BY: CDOT

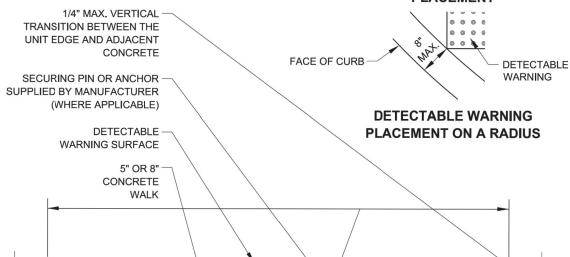
GENERAL NOTE:

THE ROWS OF DOMES IN THE DETECTABLE WARNING MATERIAL MUST BE ALIGNED WITH THE PATH OF WHEELCHAIR TRAVEL WHICH IS REQUIRED TO BE PERPENDICULAR TO THE GRADE BREAK AT THE BOTTOM OF THE RAMP TO PERMIT TRACKING BETWEEN DOME ROWS. ON BLENDED TRANSITIONS OR FLUSH TRANSITIONS, WHERE RADIAL UNITS ARE SITUATED ABOUT THE CURB RADIUS, DOME ORIENTATION IS NOT SIGNIFICANT.



UNIT PATTERN & DOME DETAIL

TYPICAL DETECTABLE WARNING PLACEMENT



DETECTABLE WARNING MATCHES FULL RAMP WIDTH OR SIDEWALK WHERE FLUSH WITH STREET

RAMP SIDE FLARE (OR SIDE **CURB WHERE** APPLIES)

DETECTABLE WARNING UNIT SECTION

	City of Chicago Rahm Emanuel, Mayor Department of Transportation Division of Engineering www.cityofchicago.org			
CD	∌T			

hicago nanuel, Mayor	DATE	REVISION
of Transportation	02/20/07	REVISION 1
hlcago.org	11/15/07	REVISION 2
	11/14/08	REVISION 3
	11/02/09	REVISION 4
	08/10/12	REVISION 5

CITY OF CHICAGO **DETECTABLE WARNING UNIT DETAILS**

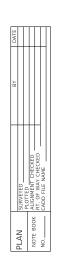
SHEET B-4-2 SCALE: NOT TO SCALE DATE: 10/23/2006

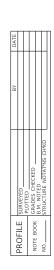
DRAWN BY: CDOT CHECKED BY: LCM

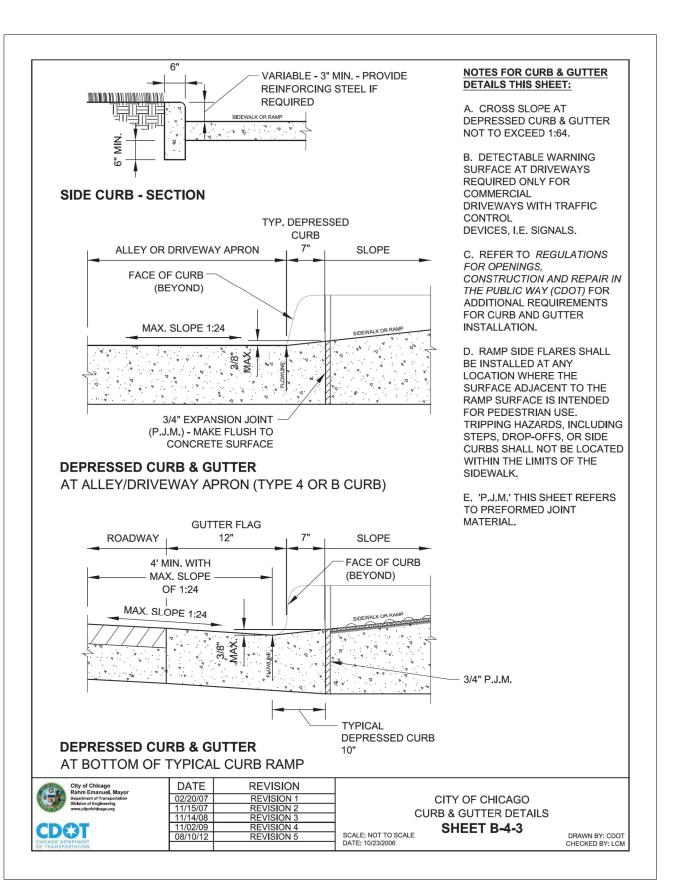
USER NAME = MMA	DESIGNED - MMA	REVISED -
	CHECKED -	REVISED -
PLOT SCALE =	DRAWN - MMA	REVISED -
PLOT DATE = \$DATE\$	CHECKED -	REVISED -

SCALE: NOT TO SCALE

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	SD-8
CDOT	PROJECT NO. E-1-525		32 of 210
	The state of the s		· ·





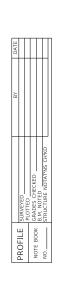


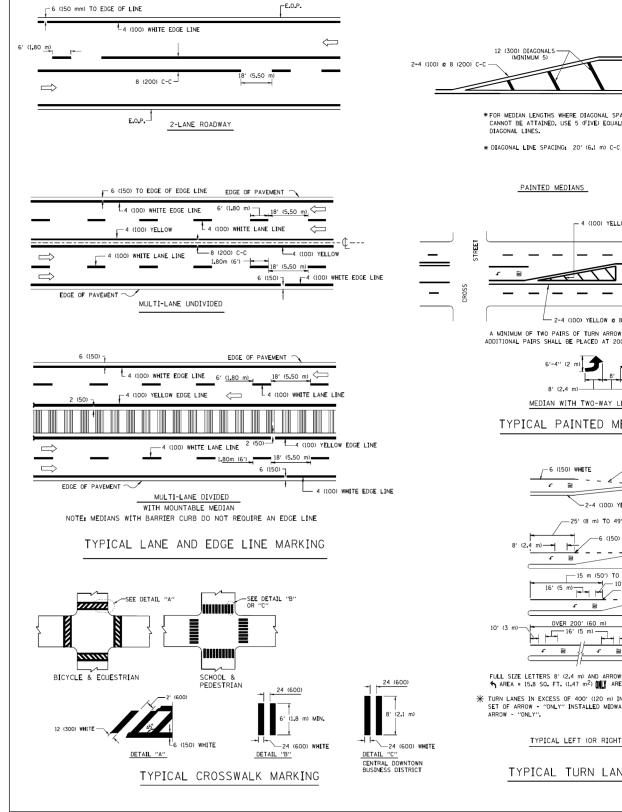
	USA Inc.
30	N. LASALLE STREET F 4200
TEL:	AGO, IL 60602 (312) 782-8150
■ I FAX:	(312) 782-1684

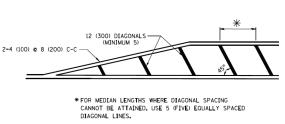
USER NAME = MMA	DESIGNED - MMA	REVISED -
	CHECKED -	REVISED -
PLOT SCALE =	DRAWN - MMA	REVISED -
PLOT DATE = SDATES	CHECKED -	REVISED -
	·	-

	F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
A STANDARDS	1388	11-E1525-00-BR	соок	SD-9
	CDOT	PROJECT NO. E-1-525		33 of 210

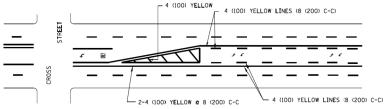




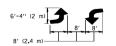




PAINTED MEDIANS

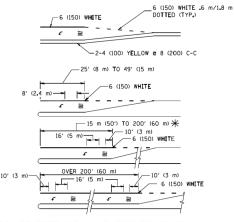


A MINIMUM OF TWO PAIRS OF TURN ARROWS SHALL BE USED, WHITE IN COLOR. ADDITIONAL PAIRS SHALL BE PLACED AT 200' (60 m) TO 300' (90 m) INTERVALS.



MEDIAN WITH TWO-WAY LEFT TURN LANE

TYPICAL PAINTED MEDIAN MARKING



FULL SIZE LETTERS 8' (2.4 m) AND ARROWS SHALL BE USED. \P AREA = 15.8 SO. FT. (1.47 m²) ONLY AREA = 22.9 SO. FT. (2.13 m²)

* TURN LANES IN EXCESS OF 400' (120 m) IN LENGTH MAY HAVE AN ADDITIONAL SET OF ARROW - "ONLY" INSTALLED MIDWAY BETWEEN THE OTHER TWO SETS OF

TYPICAL LEFT (OR RIGHT) TURN LANE

TYPICAL TURN LANE MARKING

3 (200) WHITE 12 (300) WHITE DIAGONALS @ 6' (1.8 m) SPACING ISLAND OFFSET FROM PAVEMENT EDGE 8 (200) WHITE —

> RAISED ISLAND AT PAVEMENT EDGE

TYPICAL ISLAND MARKING

TYPE OF MARKING	WIDTH OF LINE	PATTERN	COLOR	SPACING / REMARKS
CENTERLINE ON 2 LANE PAVEMENT	4 (100)	SKIP-DASH	YELLOW	6' (1.80 m) LINE WITH 18' (5.50 m) SPACE
CENTERLINE ON MULTI-LANE UNDIVIDED PAVEMENT	2 @ 4 (100)	SOLID	YELLOW	8 (200) C-C
NO PASSING ZONE LINES: FOR ONE DIRECTION FOR BOTH DIRECTIONS	4 (100) 2 @ 4 (100)	SOLID SOLID	YELLOW YELLOW	8 (200) C-C
LANE LINES	4 (100) 5 (125) ON FREEWAYS	SKIP-DASH SKIP-DASH	WHITE WHITE	6' (1.80 m) LINE WITH 18' (5.50 m) SPACE
DOTTED LINES (EXTENSIONS OF CENTER, LANE OR TURN LANE MARKINGS)	SAME AS LINE BEING EXTENDED	SKIP-DASH	SAME AS LINE BEING EXTENDED	2' (600) LINE WITH 6' (1.8) SPACE
EDGE LINES	4 (100)	SOLID	YELLOW-LEFT WHITE-RIGHT	OUTLINE MOUNTABLE MEDIANS IN YELLOW: EDGE LINES ARE NOT USED NEXT TO BARRIER CURB
TURN LANE MARKINGS	6 (150) LINE; FULL SIZE LETTERS & SYMBOLS (8' (2.4 m))	SOLID	WHITE	SEE TYPICAL TURN LANE MARKING DETAIL
TWO WAY LEFT TURN MARKING	2 @ 4 (100) EACH DIRECTION	SKIP-DASH AND SOLID	YELLOW	6' (1.8 m) LINE WITH 18' (5.50 m) SPACE FOR SKIP-DASH; 8 (200) C-C BETWEEN SOLID LINE AND SKIP-DASH LINE
	8' (2.4 m) LEFT ARROW	IN PAIRS	WHITE	SEE TYPICAL TWO-WAY LEFT TURN MARKING DETAIL
CROSSWALK LINES A. DIAGONALS (BIKE & EQUESTRIAN) B. LONGITUDINAL BARS (SCHOOL & PEDESTRIAN)	12 (300) e 45° 24 (600) e 90°	SOLID SOLID	WHITE WHITE	2' (600) APART 2' (600) APART SEE TYPICAL CROSSWALK MARKING DETAILS.
STOP LINES	24 (600)	SOLID	WHITE	PLACE 4' (1.2 m) IN ADVANCE OF AND PARALLEL TO CROSSWALK, IF PRESENT. OTHERNISE, PLACE AT DESIRED STOPPING POINT. PARALLEL TO CROSSROAD CENTERLINE, WHERE POSSIBLE
PAINTED MEDIANS	2 @ 4 (100) WITH 12 (300) DIAGONALS @ 45°	SOLID	YELLOW: TWO WAY TRAFFIC WHITE: ONE WAY TRAFFIC	8 (200) C-C FOR THE DOUBLE LINE SEE TYPICAL PAINTED MEDIAN MARKING.
GORE MARKING AND CHANNELIZING LINES	8 (200) WITH 12 (300) DIAGONALS @ 45°	SOLID	WHITE	DIAGONALS: 20' (6.1 m) (LESS THAN 30 MPH (50 km/h))
RAILROAD CROSSING	24 (600) TRANSVERSE LINES; "RR" IS 6' (1.8 m) LETTERS; 16 (400) LINE FOR "X"	SOLID	WHITE	SEE STATE STANDARD 780001 AREA OF: "R"=3.6 SO. FT. (0.33m ²) EACH "X"=54.0 SO. FT. (5.0 m ²)

FOR FURTHER DETAILS ON PAVEMENT MARKING REFER TO STREET MARKING STANDARDS, PRINTED BY CITY OF CHICACO, DEPARTMENT OF THE TOTAL OF THE T

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS)
UNLESS OTHERWISE SHOWN.

Г	E NAME =	USER NAME = drivakosgn	DESIGNED -	REVISED -T. RAMMACHER 12-07-00			CITY OF CHICAGO	F.A SECTION	COUNTY TOTAL SHEET SHEETS NO.
	pw_work\pwidot\drivakosgn\d0108315\tc	24 . dgn	DRAWN -	REVISED - K. ENG 02-28-12	STATE OF ILLINOIS			NIL.	SILE IS NOT
		PLOT SCALE = 50.000 ' / 10.	CHECKED -	REVISED -	DEPARTMENT OF TRANSPORTATION		TYPICAL PAVEMENT MARKINGS	TC-24	CONTRACT NO.
		PLOT DATE = 3/1/2012	DATE -	REVISED -		SCALE: NONE	SHEET NO. 1 OF 3 SHEETS STA. TO STA.	FED. ROAD DIST. NO. 1 ILLING	

			_
USER NAME = MMA	DESIGNED - MMA	REVISED -	
	CHECKED -	REVISED -	
PLOT SCALE =	DRAWN - MMA	REVISED -	
PLOT DATE = \$DATE\$	CHECKED -	REVISED -	

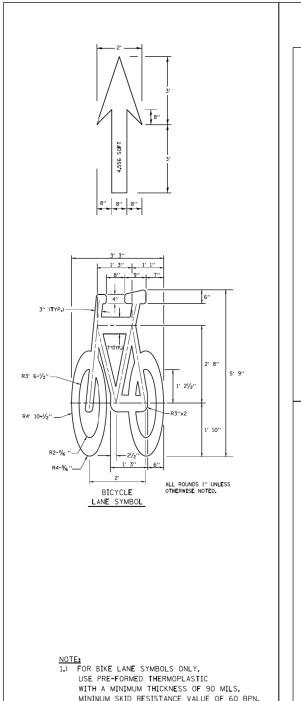
CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

	F.A.U. RTE.	SECTION	COUNTY	SHEET NO.	
OT STANDARDS	1388	11-E1525-00-BR	соок	SD-10	
	CDOT	PROJECT NO. E-1-525	34 of 210		



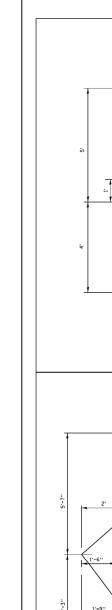


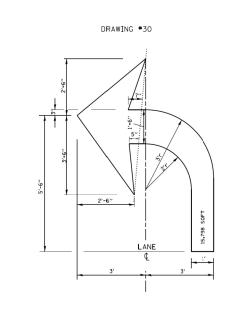


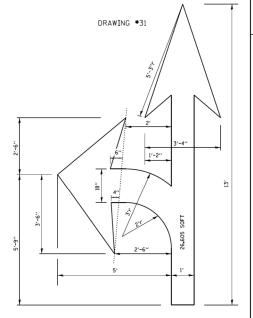
MINIMUM SKID RESISTANCE VALUE OF 60 BPN, & A MINIMUM INDEX OF REFRACTION OF 1.50.

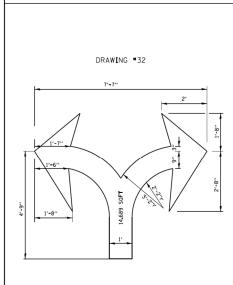
2.) THE RESIDENT ENGINEER SHALL CONTACT MR. BEN GOMBERG AT 312-744-8093 AT LEAST ONE CALENDAR WEEK PRIOR TO INSTALLING BIKE LANE SYMBOLS.

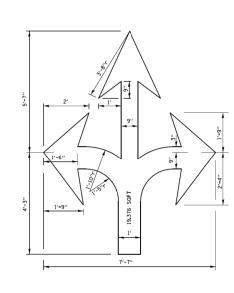
TYPICAL BIKE LANE SYMBOLS DRAWING #28







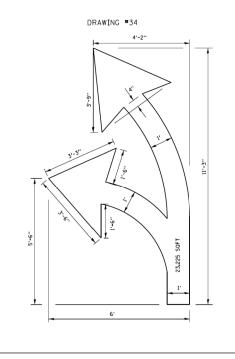


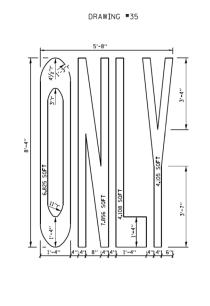


REVISED

DRAWING #33

DRAWING #29





ALL MARKINGS SHALL BE SOLID WHITE UNLESS OTHERWISE NOTED IN THE

ILE NAME =	USER NAME = drivakosgn	DESIGN
:\pw_work\pwidot\drivakosgn\d0108315\tc	24.dgn	DRAWN
	DLOT COM 5 - FO 000 4 4	CHECKE

PLOT DATE = 3/29/2012

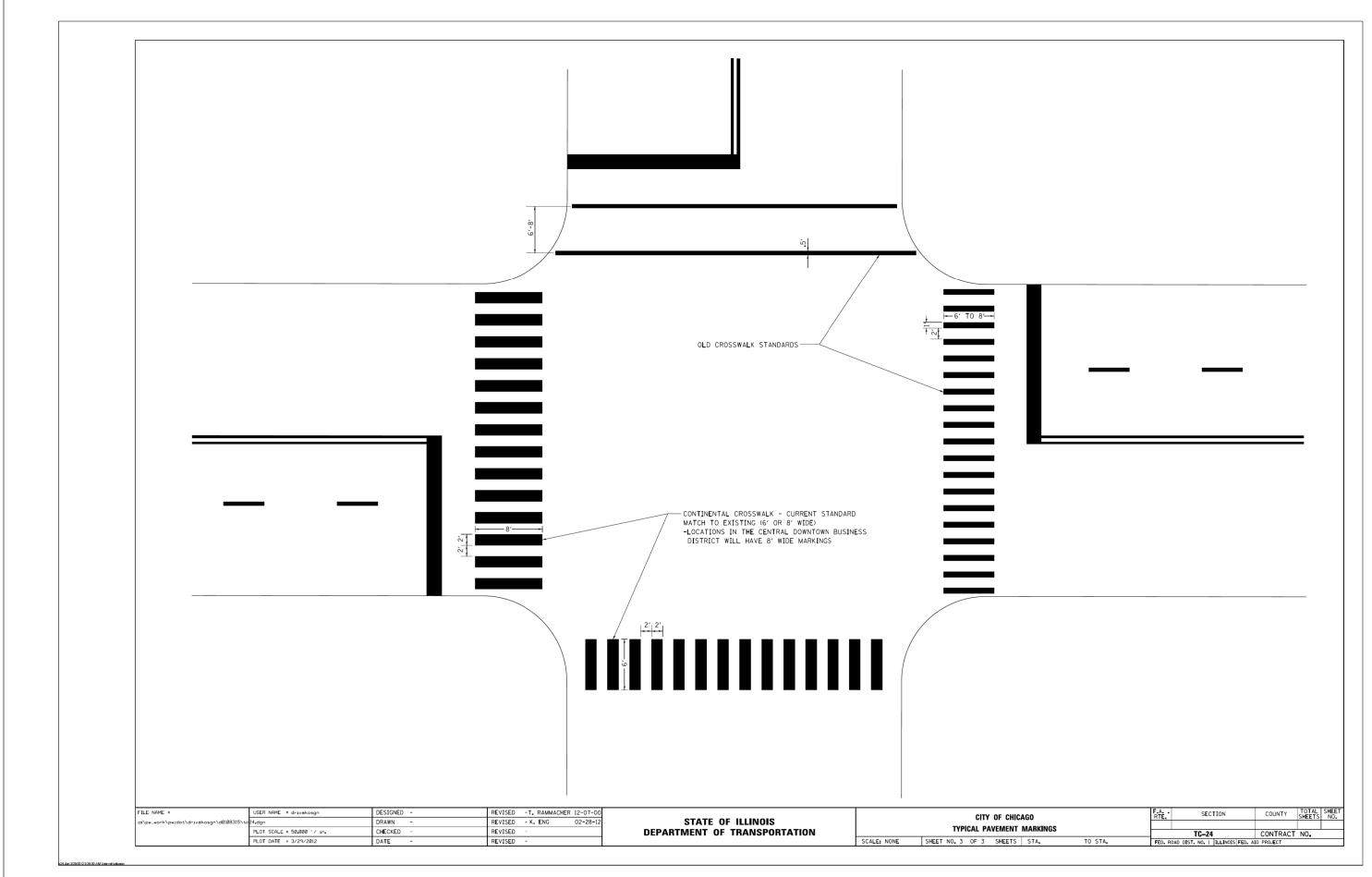
	REVISED	- T.	RAMMACHER	12-07-00	
	REVISED	- K.	ENG	02-28-12	
Ī	REVISED	-			DI
Ī	REVISED	-			

STATI	E OF	ILLINOIS
DEPARTMENT	OF	TRANSPORTATION

	CITY OF CHICAGO TYPICAL PAVEMENT MARKINGS					F.A RTE.	SECTION	COUNTY	TOTAL SHEETS	SHE	
								TC-24	CONTRACT	NO.	
	SCALE: NONE	SHEET NO. 2	OF 3	SHEETS	STA.	TO STA.	FED. R	OAD DIST. NO. 1 ILLINOIS FED. A	ID PROJECT		

	USER NAME = MMA	DESIGNED - MMA	REVISED -
		CHECKED -	REVISED -
	PLOT SCALE =	DRAWN - MMA	REVISED -
	PLOT DATE = \$DATE\$	CHECKED -	REVISED -
_			

DATE



USER NAME = MMA DESIGNED - MMA REVISED CHECKED -REVISED LOT SCALE = DRAWN REVISED PLOT DATE = \$DATE\$ CHECKED REVISED

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

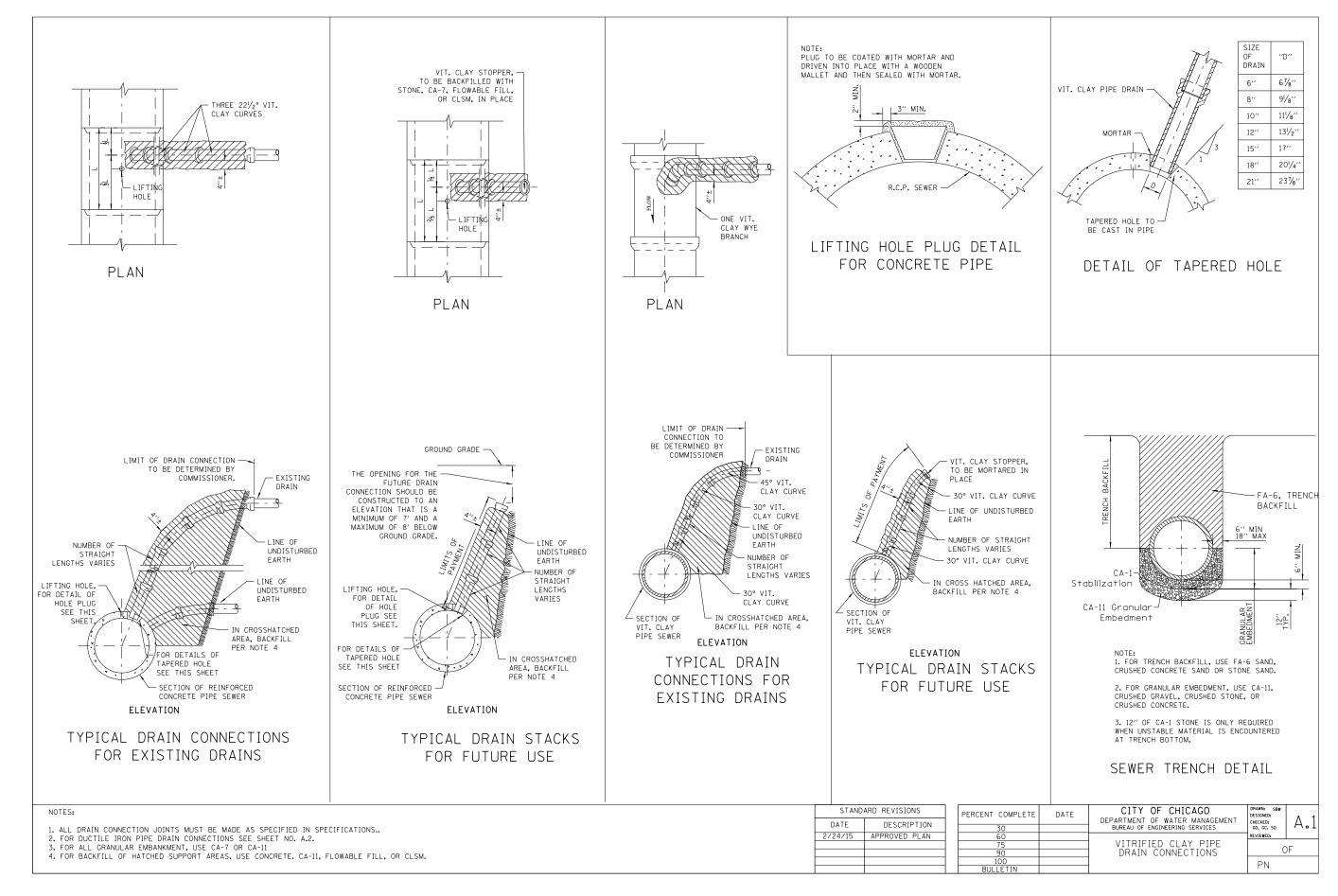
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

SECTION COUNTY SHEET NO. SD-12 11-E1525-00-BR соок 1388 CDOT PROJECT NO. E-1-525 36 of 210

WSP USA Inc.
30 N. LASALLE STREET
SUITE 420.06602
TEL: 1312/ 782-8150
FAX: (312) 782-1684









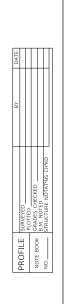
USER NAME = KSD DESIGNED - KSD REVISED CHECKED REVISED DRAWN REVISED PLOT DATE = SDATES CHECKED REVISED

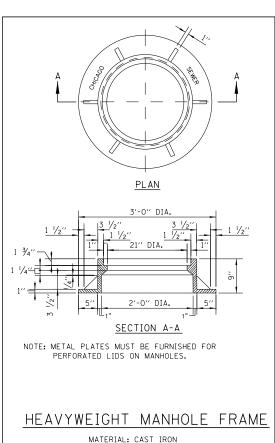
CITY OF CHICAGO DIVISION OF ENGINEERING

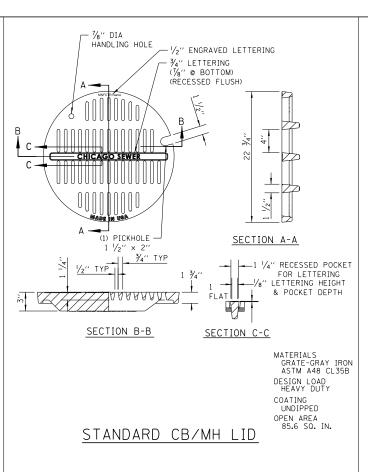
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

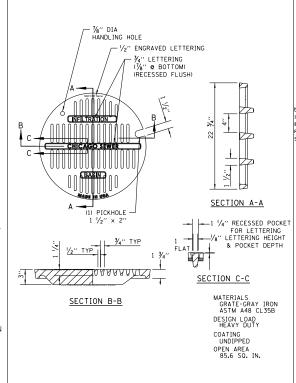
SECTION COUNTY SHEET NO. SD-13 1388 11-E1525-00-BR соок CDOT PROJECT NO. E-1-525 37 of 210



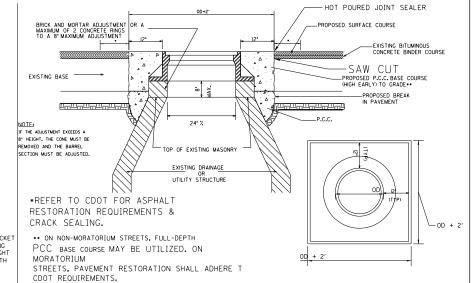








STANDARD LID FOR INFILTRATION SYSTEMS



PLAN VIEW (BASE TO GRADE)

PLAN

SECTION

DETAIL OF FRAME ADJUSTMENT

MUST BE ADJUSTED. THE CONTRACTOR IS REQUIRED TO REPLACE ANY BROKEN THE CONTRACTOR IS REQUIRED TO REPLACE ANY BROKEN
FRAMES AND LIDS OF SEWER STRUCTURES WITH STANDARD
FRAMES AND LIDS OF THE DWM. IN ADJUSTMENT OR RECONSTRUCTION OF SEWER STRUCTURES, ANY
NON-STANDARD FRAMES AND LIDS MUST BE REPLACED WITH STANDARD FRAMES AND LIDS. IN
ADJUSTMENT OR RECONSTRUCTION OF INLETS, ANY NON-STANDARD INLETS GUTTER BOXES) MUST BE

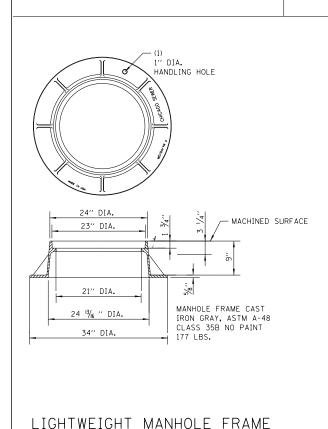
IF THE ADJUSTMENT EXCEEDS AN 8" HEIGHT, THE CONE MUST BE REMOVED AND THE BARREL SECTION

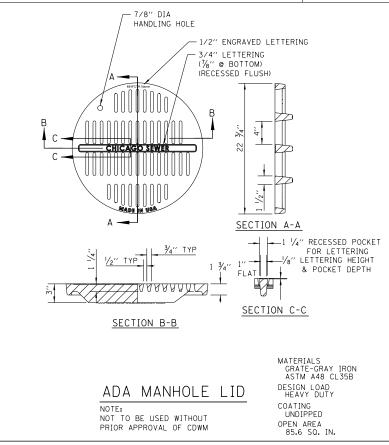
THE FRAMES AND LIDS OF SEWER STRUCTURES TO BE ABANDONED, REMOVED, OR FILLED MUST BE SALVAGED AND THE DWM NOTIFIED FOR PICK UP.

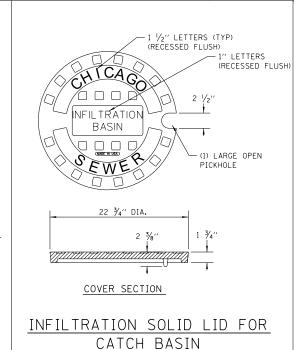
REPLACED WITH DWM STANDARD INLETS.

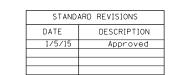
NOTES:

MANHOLES, CATCH BASINS AND INLETS MUST BE PROTECTED FROM THE ENTRY OF ASPHALT/DEBRIS INTO THE SEWER SYSTEM DURING CONSTRUCTION. THE CONTRACTOR MUST MARK LOCATIONS OF ALL SEWER STRUCTURES ON THE SIDEWALK BEFORE STARTING PAVEMENT REMOVAL/REPLACEMENT. ADJUSTMENT OF FRAMES AND LIDS OF SEWER STRUCTURES MUST BE COMPLETED PRIOR TO STREET RESURFACING.









PERCENT COMPLETE	DATE	CITY OF CHICAGO Department of water management	DRAWN: SBW DESIGNED: CHECKED:	Δ3
30		BUREAU OF ENGINEERING SERVICES	GD, GC, SO	^ • J
60			REVIEWED:	
75		MANHOLE	ے ا	_
90		LIDS AND FRAMES) F
100			PN	
BULLETIN			1 14	

SOLID LID FOR MANHOLES

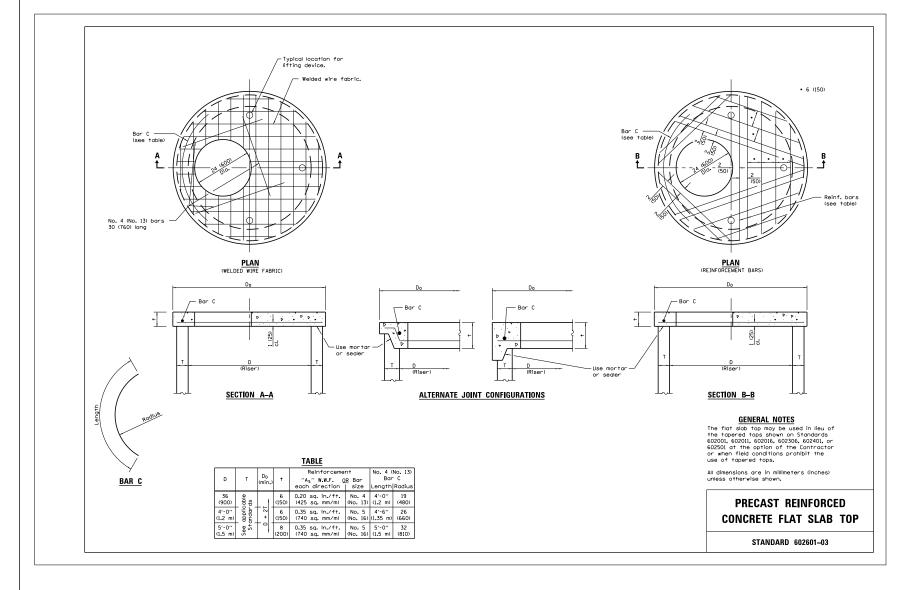
USER NAME = KSD	DESIGNED - KSD	REVISED -
	CHECKED -	REVISED -
PLOT SCALE =	DRAWN - KSD	REVISED -
PLOT DATE = \$DATE\$	CHECKED -	REVISED -

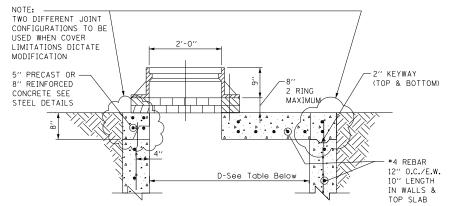
CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

	F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
T STANDARDS	1388	11-E1525-00-BR	соок	SD-14
	CDOT	PROJECT NO. E-1-525		38 of 210

SPECIAL DRAINAGE STRUCTURES FOR PUBLIC STREETS AND ALLEYS





STANDARD FLAT TOP SLAB FOR CATCH BASINS

NOTES:
FLAT TOP SLAB AAPLICATION CAN ONLY BE USED WITH WRITTEN PERMISSION FROM CDWM.
USE LATEST IDOT DETAIL, #602601

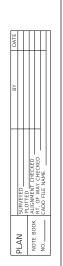
STANDARD REVISIONS			
DATE	DESCRIPTION		
1/5/15	APPROVED PLAN		

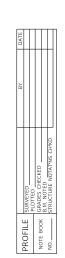
PERCENT COMPLETE	DATE	CITY OF CHICAGO DEPARTMENT OF WATER MANAGEMENT	DRAWN: SBW DESIGNED: CHECKED:	Δ4
30		BUREAU OF ENGINEERING SERVICES		/ \ a
60			REVIEWED:	
75		FLAT TOP SLAB DETAILS	ے ا	_
90)F
100			PN	
BULLETIN			1 14	

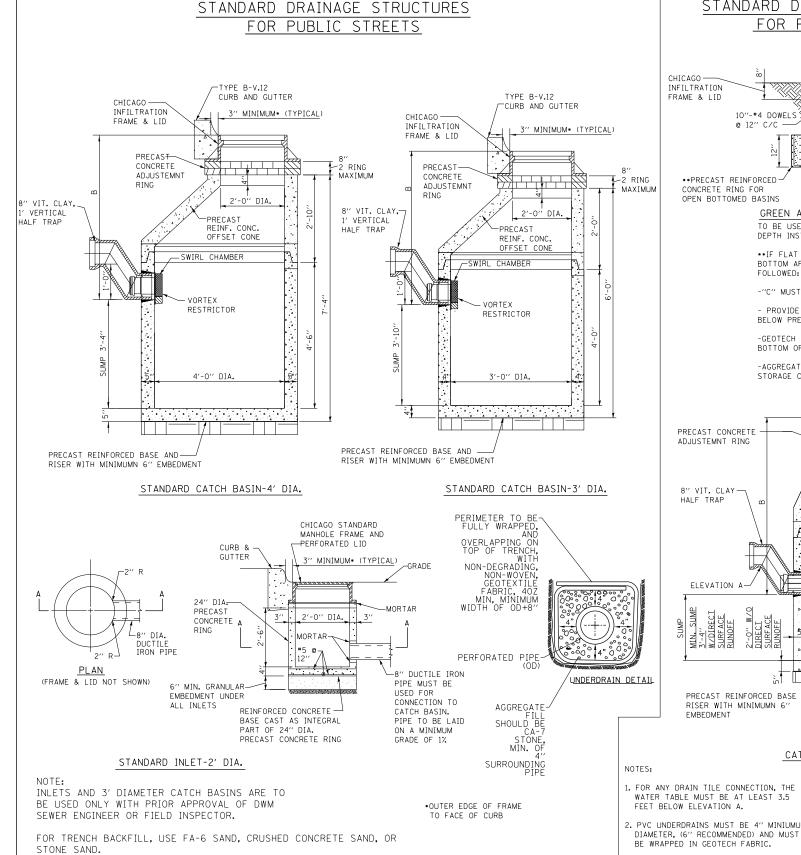


SER NAME = KSD	DESIGNED - KSD	REVISED -
	CHECKED -	REVISED -
OT SCALE =	DRAWN - KSD	REVISED -
OT DATE = \$DATE\$	CHECKED -	REVISED -

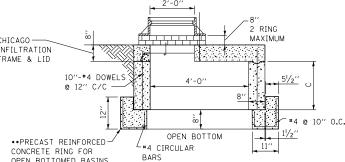
	F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
CDOT STANDARDS	1388	11-E1525-00-BR	соок	SD-15
	CDOT	PROJECT NO. E-1-525		39 of 210







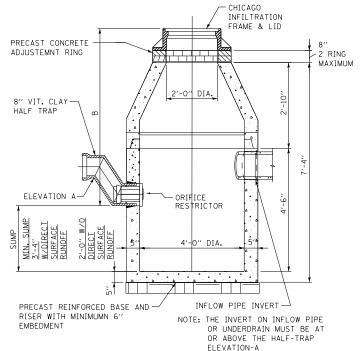
STANDARD DRAINAGE STRUCTURES FOR PUBLIC ALLEYS



GREEN ALLEY OPEN BOTTOM CATCH BASIN

TO BE USED IF STANDARD CONE WITH REQUIRED SUMP DEPTH INSTALLATION IS IMPOSSIBLE.

- ••IF FLAT TOP SLAB CATCH BASIN IS USED IN AN OPEN BOTTOM APPLICATION, THE FOLLOWING CRITERIA MUST BE
- -"C" MUST BE A MINIMUM OF 3 FEET.
- PROVIDE A MINIMUM AGGREGATE BASE OF 1.0 FEET BELOW PRECAST REINFORCED CONCRETE RING.
- -GEOTECH FABRIC MUST BE PLACED ON SIDES AND BOTTOM OF AGGREGATE SURROUNDING BASIN.
- -AGGREGATE BASE MUST PROVIDE ADEQUATE STORMWATER STORAGE CAPACITY PER THE APPROVED/PERMITTED PLANS.



CATCH BASIN-ORIFICE RESTRICTOR

N.T.S.

. FOR ANY DRAIN TILE CONNECTION, THE WATER TABLE MUST BE AT LEAST 3.5

2. PVC UNDERDRAINS MUST BE 4" MINIUMUM DIAMETER, (6" RECOMMENDED) AND MUST

STANDARD REVISIONS DESCRIPTION DATE APPROVED 9/30/16

_	PERCENT COMPLETE	DATE	CITY OF CHICAGO DEPARTMENT OF WATER MANAGEMENT	DESIGNED: CHECKED:
	30		BUREAU OF ENGINEERING SERVICES	
	60			REVIEWED:
	75		DRAINAGE STRUCTURE DETAILS	
	90			
	100			DNI
	BULLETIN			FIN

DRAINAGE STRUCTURES

RESTRICTORS

-ORIFICE RESTRICTOR

TIGHT FIT IS MADE.

VORTEX RESTRICTOR

TIGHT FIT IS MADE.

1. CATCH BASIN TO CATCH BASIN CONNECTIONS ARE

ALLOWED IN PRIVATE SITES & ALLEYS. ONLY THE

DOWNSTREAM CATCH BASIN IS REQUIRED TO HAVE A

2. IF B < 4 FEET, THEN USE A DUCTILE IRON PIPE HALF TRAP AND FLAT TOP SLAB CATCH BASIN AS NECESSARY.

3. INLETS AND 3' DIAMETER CATCH BASINS ARE TO BE

THE DWM'S RAIN BLOCKER RESTRICTOR PROGRAM MUST

CONSIDER LIMITING THE NUMBER OF CATCH BASINS TO THE EXTENT PRACTICAL. THE NUMBER OF EXISTING

FLOW RESTRICTORS MUST BE INSTALLED IN ALL CATCH BASINS OUTSIDE OF THE CENTRAL BUSINESS DISTRICT.
RESTRICTORS MUST NOT BE INSTALLED IN CATCH

BASINS IN CLOSE PROXIMITY TO VIADUCT AREAS, BUS

APPROVE THE NON-INSTALLATION OR REMOVAL OF ANY

STOPS, OR EMERGENCY ENTRANCES. THE DWM MUST

*ARTERIAL STREETS: 3-INCH ORIFICE RESTRICTOR
*BUS ROUTES: 3-INCH ORIFICE RESTRICTOR

*RESIDENTIAL STREETS: 3-INCH VORTEX RESTRICTOR

*ALLEYS: 3-INCH ORIFICE RESTRICTOR IN THE LAST CB. *CLOSED LIDS ARE REQUIRED ON ALL MANHOLES EXCEPT

AT INTERSECTIONS WHERE A PERFORATED LID SHALL BE

WITH PRIOR APPROVAL OF DWM FIELD INSPECTOR.

BE MAINTAINED WITH ANY ROADWAY IMPROVEMENT.

THE DESIGN OF ANY ROADWAY IMPROVEMENT MUST

THE RESTRICTORS CAN BE OBTAINED FROM DWM CENTRAL DISTRICT AT 3901 S. ASHLAND AVE. THE

CONTRACTOR SHOULD ARRANGE FOR PICK UP BY

CONTACTING 312-747-1177 (7AM TO 3PM, M-F)

RESTRICTOR, REQUIREMENTS FOR RESTRICTOR

INSTALLATION ARE AS FOLLOWS:

STRUCTURES SHOULD NOT BE INCREASED.

GENERAL NOTES:

HALE-TRAP.

LISED ONLY

RESTRICTOR NOTES:

INSERT THE RESTRICTOR INTO THE HALF-TRAP.

UPON TIGHTENING OF THE CENTER NUT ON THE

O-RINGS WILL EXPAND INSIDE THE HALF TRAP,

FACE OF THE RESTRICTOR, THE RUBBER

PULL ON RESTRICTOR TO VERIFY THAT A

INSERT THE RESTRICTOR WITH THE OPENING

THE FACE OF THE RESTRICTOR, THE RUBBER

DOWN. UPON TIGHTENING OF THE 2 BOLTS ON

O-RINGS WILL PROVIDE A WATER- TIGHT SEAL.

PROVIDING A WATER- TIGHT SEAL. PULL ON RESTRICTOR TO VERIFY THAT A

USER NAME = KSD	DESIGNED - KSD	REVISED -
	CHECKED -	REVISED -
PLOT SCALE =	DRAWN - KSD	REVISED -
PLOT DATE = \$DATE\$	CHECKED -	REVISED -

FOR GRANULAR EMBEDMENT, USE CA-11, CRUSHED GRAVEL, CRUSHED

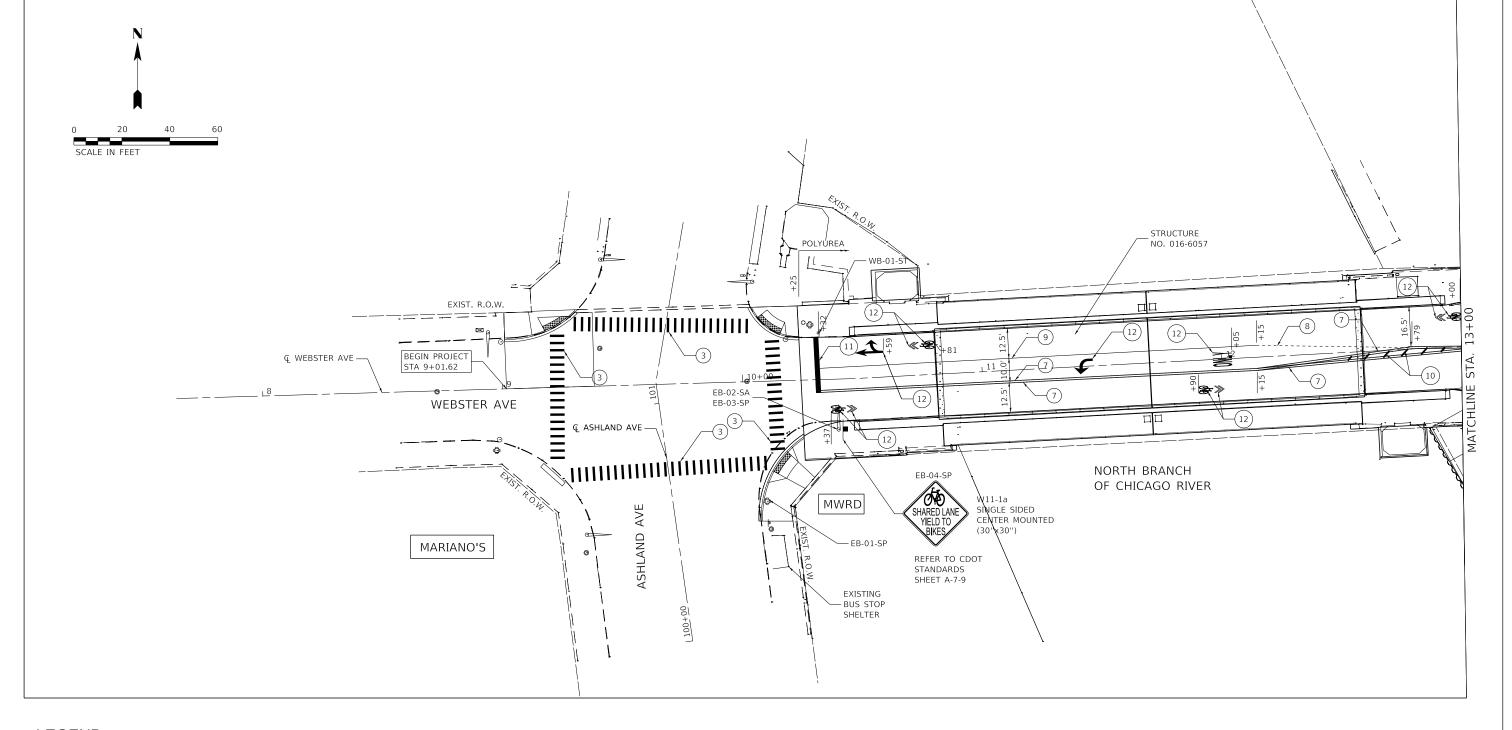
STONE, OR CRUSHED CONCRETE.

	F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
	1388	11-E1525-00-BR	соок	SD-16
	CDOT	PROJECT NO. E-1-525		40 of 210
1				,

A.18







LEGEND

- (1) THERMOPLASTIC PAVEMENT MARKING LINE 4" (WHITE)
- (2) THERMOPLASTIC PAVEMENT MARKING LINE 4" (6' DASH-18' SKIP YELLOW)
- 3 THERMOPLASTIC PAVEMENT MARKING LINE 24" (WHITE)
- 4 THERMOPLASTIC PAVEMENT MARKING LETTERS AND SYMBOLS
- (5) POLYUREA PAVEMENT MARKING TYPE I LINE 4" (WHITE) (6) POLYUREA PAVEMENT MARKING TYPE I - LINE 4" (6' DASH-18' SKIP YELLOW)
- 7 POLYUREA PAVEMENT MARKING TYPE I LINE 4" (YELLOW)
- 8 POLYUREA PAVEMENT MARKING TYPE I LINE 6" (2' DASH-6' SKIP WHITE)
- 9 POLYUREA PAVEMENT MARKING TYPE I LINE 6" (WHITE)
- (10) POLYUREA PAVEMENT MARKING TYPE I LINE 12" (YELLOW)
- 1) POLYUREA PAVEMENT MARKING TYPE I LINE 24" (WHITE)
- 12 POLYUREA PAVEMENT MARKING TYPE I LETTERS AND SYMBOLS

____ SIGN

SIGN NUMBERING CODE **EXAMPLE**

DIRECTION OF TRAFFIC MOUNTING TYPE EB - EASTBOUND WEBSTER AVE ST - STEEL POST SA - SIGNAL POLE MAST ARM WB - WESTBOUND WEBSTER AVE SP - SIGNAL POLE LP - LIGHT POLE SIGN PANEL NUMBER

- 1. STATIONS AND OFFSETS ARE FROM WEBSTER AVE CENTERLINE.
 2. FOR ADDITIONAL DETAILS SEE IDOT DISTRICT 1 STANDARD TC-24
 3. FOR SIGN PANEL DETAILS SEE THE MOST CURRENT EDITION OF
- "SIGN FABRICATION AND INSTALLATIONS" MANUALS.
- 4. PLACE THE SHARED LANE MARKINGS 5 FEET (ON CENTER) FROM FACE OF THE BRIDGE CURB.



USER NAME = MMA	DESIGNED - MMA	REVISED -
	CHECKED - RPH	REVISED -
PLOT SCALE =	DRAWN - MMA	REVISED -
PLOT DATE = SDATES	CHECKED - RPH	REVISED -

CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

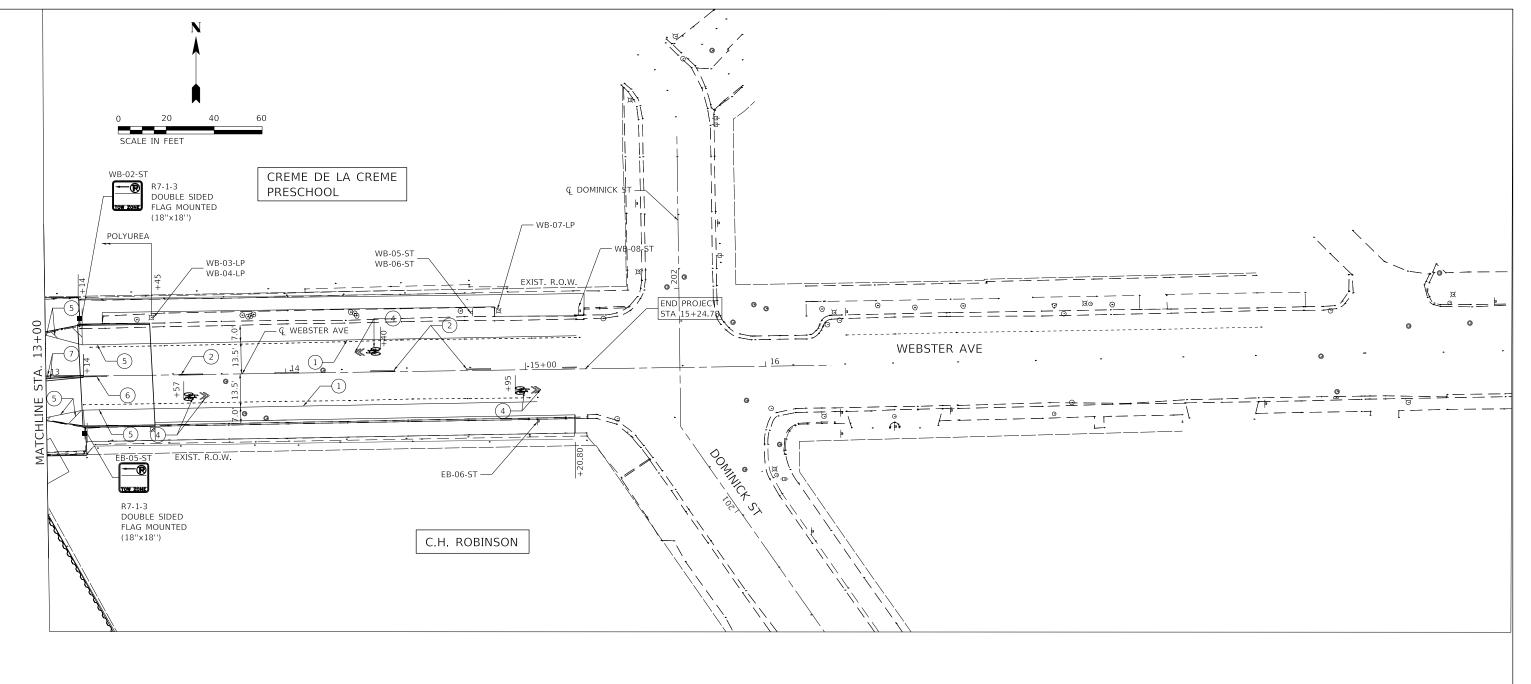
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

PAVEMENT MARKING AND SIGNING

SECTION COUNTY SHEET NO. PMK-1 11-E1525-00-BR СООК CDOT PROJECT NO. E-1-525







LEGEND

- 1) THERMOPLASTIC PAVEMENT MARKING LINE 4" (WHITE)
- (2) THERMOPLASTIC PAVEMENT MARKING LINE 4" (6' DASH-18' SKIP YELLOW)
- (3) THERMOPLASTIC PAVEMENT MARKING LINE 24" (WHITE)
- 4 THERMOPLASTIC PAVEMENT MARKING LETTERS AND SYMBOLS
- (5) POLYUREA PAVEMENT MARKING TYPE I LINE 4" (WHITE)
- (6) POLYUREA PAVEMENT MARKING TYPE I LINE 4" (6' DASH-18' SKIP YELLOW)
- (7) POLYUREA PAVEMENT MARKING TYPE I LINE 4" (YELLOW)
- 8 POLYUREA PAVEMENT MARKING TYPE I LINE 6" (2' DASH-6' SKIP WHITE)
- POLYUREA PAVEMENT MARKING TYPE I LINE 6" (WHITE)
- (10) POLYUREA PAVEMENT MARKING TYPE I LINE 12" (YELLOW)
- (1) POLYUREA PAVEMENT MARKING TYPE I LINE 24" (WHITE)
- 12 POLYUREA PAVEMENT MARKING TYPE I LETTERS AND SYMBOLS

SIGN NUMBERING CODE **EXAMPLE**

DIRECTION OF TRAFFIC MOUNTING TYPE ST - STEEL POST

EB - EASTBOUND WEBSTER AVE WB - WESTBOUND WEBSTER AVE

SA - SIGNAL POLE MAST ARM SP - SIGNAL POLE LP - LIGHT POLE

SIGN PANEL NUMBER

- STATIONS AND OFFSETS ARE FROM WEBSTER AVE CENTERLINE.
 FOR ADDITIONAL DETAILS SEE IDOT DISTRICT 1 STANDARD TC-24
 FOR SIGN PANEL DETAILS SEE THE MOST CURRENT EDITION OF
- "SIGN FABRICATION AND INSTALLATIONS" MANUALS.
- 4. PLACE THE SHARED LANE MARKINGS 5 FEET (ON CENTER) FROM FACE OF THE BRIDGE CURB.



JSER NAME = MMA	DESIGNED - MMA	REVISED -
	CHECKED - RPH	REVISED -
PLOT SCALE =	DRAWN - MMA	REVISED -
PLOT DATE = \$DATE\$	CHECKED - RPH	REVISED -

CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

PAVEMENT MARKING AND SIGNING

SECTION COUNTY SHEET NO. PMK-2 11-E1525-00-BR CDOT PROJECT NO. E-1-525

PAVEMENT MARKING SCHEDULE

LOCATION	CONTRACTOR DESTRICT PAVEMENT MARKING - LETTERS AND CONTRACTOR SYMBOLS	THERMOPLASTIC PAVEMENT O MARKING - LINE 4"	THERMOPLASTIC PAVEMENT O MARKING - LINE 24"	DOLYUREA PAVEMENT MARKING - LETTERS AND SYMBOLS	POLYUREA PAVEMENT OMARKING TY 1 - LINE 4"	POLYUREA PAVEMENT Q MARKING TY 1 - LINE 6"	POLYUREA PAVEMENT OMARKING TY 1 - LINE 1 12"	POLYUREA PAVEMENT OMARKING TY 1 - LINE 1 24"	S PAVEMENT MARKING THE REMOVAL-WATER BLASTING
WEBSTER AVENUE	69	379	516	157	906	201	20	23	516
TOTAL	69	379	516	157	906	201	20	23	516

SIGNING SCHEDULE

LOCATION	SIGN NO.	LEGEND / DESCRIPTION	CODE	MOUNTING TYPE	ACTION	LOCATION (ST	ATION / OI	TION / OFFSET)		N / OFFSET) PROPOSED PANEL DIMENSIONS		POSTS	PANEL TYPE 1	REMOVE, STORE AND RE-ERECT	
						EXISTING	DDC.	DOCED	WIDTH	HE I GHT			SIGN PANEL		
						EXISTING	FNC	PROPOSED		FNOFOSED		(FT)	(EACH)	(SQ FT)	(EACH)
	EB-01-SP	BRIDGE WEIGHT LIMITS-TONS	CUSTOM	TRAFFIC SIGNAL POLE	EXISTING SIGN TO REMAIN	10+08 45.0' RT	-	-							
	WB - 01 - ST	LANE CONTROL SIGN	R3-8	STEEL POST	REMOVE AND REINSTALL EXISTING SIGN	10+32 19.5 LT	10+32	19.5 LT					1		
	EB-02-SA	BRIDGE WEIGHT LIMITS-TONS	CUSTOM	TRAFFIC SIGNAL MAST ARM	REMOVE AND RELOCATE EXISTING SIGN	10+39 16.0' RT	10+43	16.0' RT					1		
	EB-03-SP	N ASHLAND AVE	D3 - 1	TRAFFIC SIGNAL POLE	REMOVE AND RELOCATE EXISTING SIGN	10+39 21.0 RT	10+43	21.0' RT					1		
	EB-04-SP	SHARED LANE-YIELD TO BIKES	W11-1A	TRAFFIC SIGNAL POLE	FURNISH AND INSTALL		10+43	21.0' RT	2.5	2.5		6.25			
	WB - 02 - ST	NO PARKING TOW ZONE	R7-1-3	STEEL POST	FURNISH AND INSTALL		13+15	22.0' RT	1.5	1.5	1	2.25			
WEBSTER	EB-05-ST	NO PARKING TOW ZONE	R7-1-3	STEEL POST	FURNISH AND INSTALL		13+17	22.0' LT	1.5	1.5	1	2.25			
AVENUE	WB-03-LP	ASHLAND	W16-8P	LIGHT POLE POST	REMOVE AND RELOCATE EXISTING SIGN	13+44 24.0 LT	13+22	24.5' LT					1		
	WB-04-LP	LANE CONTROL SIGN	R3-8	LIGHT POLE POST	REMOVE AND RELOCATE EXISTING SIGN	13+44 24.0 LT	13+22	24.5' LT					1		
	WB - 05 - ST	ASHLAND	W16-8P	STEEL POST	EXISTING SIGN TO REMAIN	14+78 24.0 LT	-	-							
	WB-06-ST	LEFT LANE MUST TURN LEFT	R3 - 7	STEEL POST	EXISTING SIGN TO REMAIN	14+78 24.0 LT	-	-							
	WB - 07 - LP	BRIDGE WEIGHT LIMITS-TONS	CUSTOM	LIGHT POLE POST	REMOVE AND RELOCATE EXISTING SIGN	14+89 24.5 LT	15+30	25.0' LT					1		
	EB-06-ST	NO PARKING TOW ZONE	R7-1-3	STEEL POST	REMOVE AND REINSTALL EXISTING SIGN	15+04 21.5 RT	15+04	21.5' RT					1		
	WB - 08 - ST	NO PARKING TOW ZONE	R7-1-3	STEEL POST	REMOVE AND REINSTALL EXISTING SIGN	15+04 21.5 LT	15+23	24.0' LT					1		
			•							TOTAL	2.00	11	8		

SIGN NUMBERING CODE

EXAMPLE

DIRECTION OF TRAFFIC

MOUNTING TYPE

EB - EASTBOUND WEBSTER AVE

WB - WESTBOUND WEBSTER AVE

ST - STEEL POST SA - SIGNAL POLE MAST ARM SP - SIGNAL POLE LP - LIGHT POLE

- NOTES:

 1. STATIONS AND OFFSETS ARE FROM WEBSTER AVE CENTERLINE.

 2. FOR ADDITIONAL DETAILS SEE IDOT DISTRICT 1 STANDARD TC-24

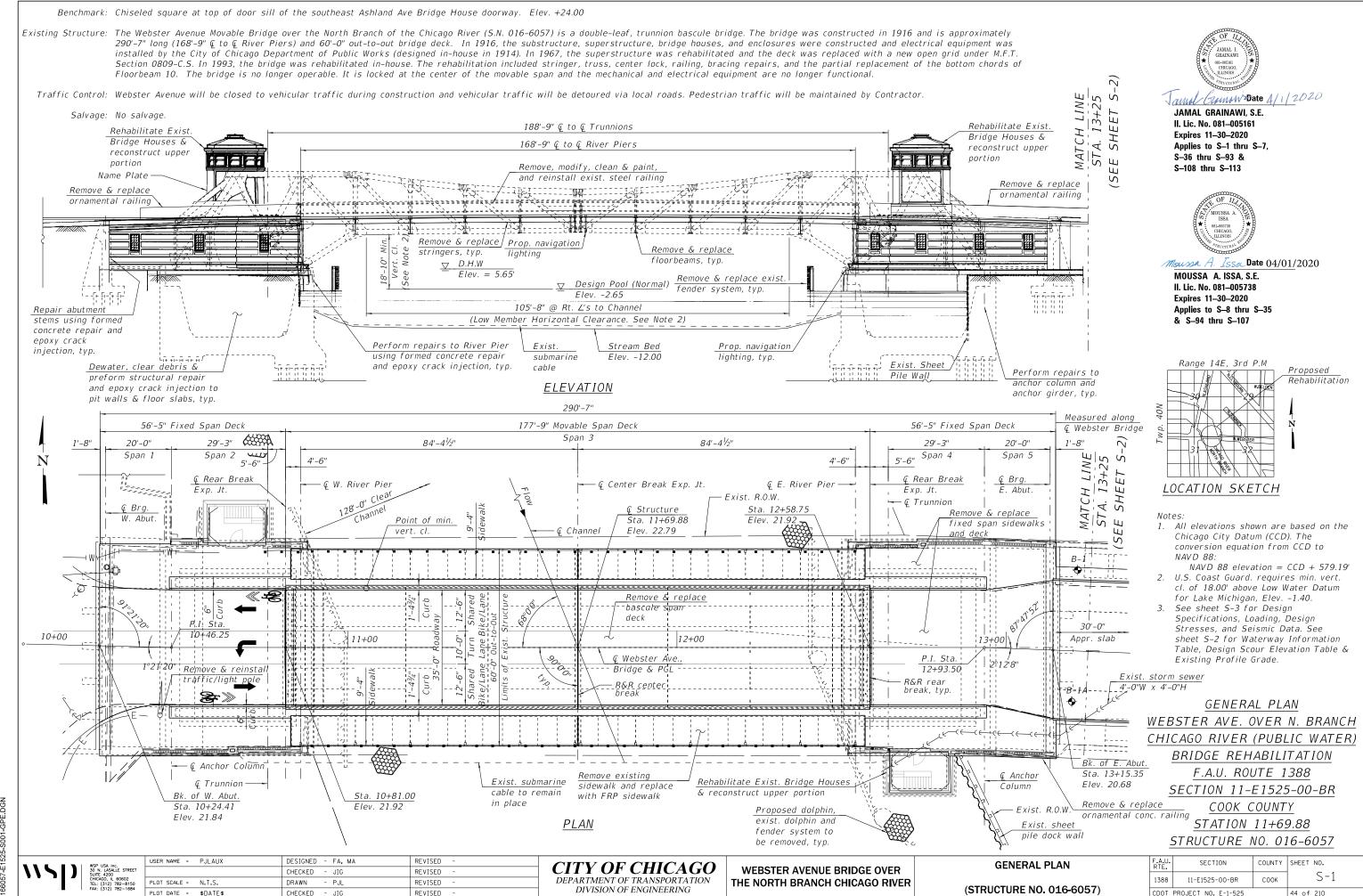
 3. FOR SIGN PANEL DETAILS SEE THE MOST CURRENT EDITION OF
- "SIGN FABRICATION AND INSTALLATIONS" MANUALS.

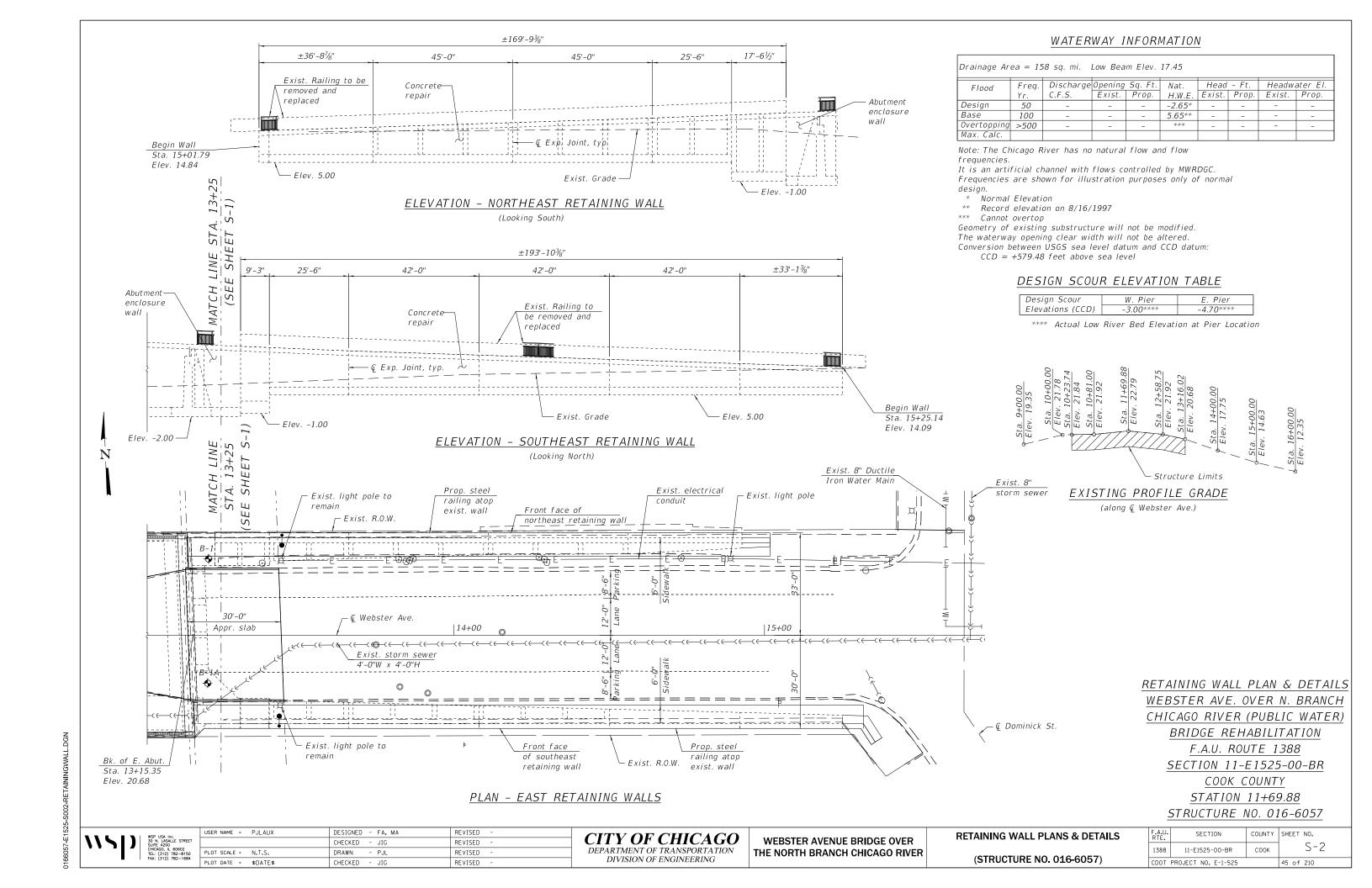
 4. PLACE THE SHARED LANE MARKINGS 5 FEET (ON CENTER) FROM FACE OF THE BRIDGE CURB.

USER NAME = MMA	DESIGNED - MMA	REVISED -
	CHECKED - RPH	REVISED -
PLOT SCALE =	DRAWN - MMA	REVISED -
PLOT DATE = \$DATE\$	CHECKED - RPH	REVISED -

PAVEMENT	MARKING AND
SIGNING	SCHEDULES

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	PMK-3
CDOT	PROJECT NO. E-1-525		43 of 210





GENERAL NOTES

- 1. Calculated weight of Structural Steel = 166,600 lbs. (Fixed Spans) = 372.600 lbs. (Bascule Span) = 6,830 lbs. (Bridge Houses)
- 2. All structural steel shall be AASHTO M270 Grade 50, unless otherwise noted.
- 3. Fasteners shall be ASTM A325 Type 1, mechanically galvanized bolts. Bolts $\frac{7}{6}$ \odot , holes $^{15}/_{16}$ " \bigcirc , unless otherwise noted.
- 4. No field welding is permitted except as specified in the contract documents.
- 5. Reinforcement bars designated (E) shall be epoxy coated.
- 6. Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 7. Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of \(\frac{1}{8} \) in. (0.01 ft.). Adjustments shall be made either by grinding the surface or by shimming the bearings.
- 8. Protective Concrete Sealer shall be applied to the designated areas of the abutments
- 9. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.
- 10. All new and existing steel shall be cleaned and painted utilizing Paint System 1 - Oz/E/U. The color of the final finish coat shall match Sherwin Williams Standard Glossy Color SW2717 (Bordeaux). Cost included in the cost of "Cleaning and Painting Structural Steel."
- 11. All new structural steel shall be shop primed, and faying surfaces shall be cleaned to base metal prior to erection.
- 12. All existing structural steel that is to remain in place in the movable and fixed spans of the bascule bridge shall be cleaned and painted. This includes, but is not limited to, truss members, floorbeams, stringers, lateral bracing, sidewalk support framing, sidewalk railing, anchor columns and girders, machinery framing, counterweight steel, and all connection steel.
- 13. The existing structural steel shall be cleaned per near white blast cleaning, SSPC-SP10, and painted as specified in the Special Provision for "Cleaning and Painting Existing Steel Structures."
- 14. As determined by the Commissioner, inaccessible areas shall be reviewed and possibly omitted from cleaning and painting.
- 15. All motors and machinery shall be properly protected from cleaning and painting work. No painting may be performed until protection has been approved by the Enaineer
- 16. If the Contractor elects to use cantilever forming brackets on the exterior beams of the fixed spans, the brackets shall be placed at the same locations as required for the hardwood blocks in Article 503.06(b) of the Standard Specifications. If additional cantilever forming brackets are required, hardwood blocking shall be wedged between the exterior and first interior beam at each of these additional bracket locations.
- 17. All temporary items within the waterway must be removed.
- 18. All elevations refer to Chicago City Datum.
- 19. The Contractor shall submit calculations and details demonstrations the structural integrity of the bridge is maintained under the additional imposed loads of the containment system. See special provisions.
- 20. A minimum of 4 air monitors will be required to monitor abrasive blasting operations at this site. See special provisions for Containment and Disposal of Lead Paint Cleaning Residues.
- 21. SSPC QP1 and SSPC QP2 Certifications are required for this Contract. removed.

INDEX OF SHEETS

- General Plan
- Retaining Wall Plans & Details 5-2
- 5-3 General Data I (General Notes & Index of Sheets)
- General Data II (Scope of Work & Total Bill of Material) 5-4
- S-5 Cross Sections
- 5-6 Existing Plans and Removal - Fixed Spans
- Existing Plans and Removal Bascule Span 5-7
- Top of Slab Elevations: West Fixed Spans I
- 5-9 Top of Slab Elevations: West Fixed Spans II
- Top of Slab Elevations: East Fixed Spans I 5-10
- Top of Slab Elevations: East Fixed Spans II
- Top of Slab Elevations: East Approach Slab
- Superstructure Plan West Fixed Spans S-13
- Superstructure Plan Geometrics West Fixed Spans 5-14
- S-15 Cross Sections - West Fixed Spans
- S-16 Diaphragm Details - West Fixed Spans
- S-17 Parapet Elevations and Details - West Fixed Spans Superstructure Details - West Fixed Spans
- Superstructure Plan East Fixed Spans 5-19
- 5-20 Superstructure Plan Geometrics - East Fixed Spans
- 5-21 Cross Sections - East Fixed Spans
- Diaphragm Details East Fixed Spans 5-22
- 5-23 Parapet Elevations and Details - East Fixed Spans
- 5-24 Superstructure Details - East Fixed Spans
- S-25 East Approach Slab
- 5-26 East Approach Slab Details
- 5-27 Framing Plan - West Fixed Spans
- Framing Plan East Fixed Spans
- Fixed Spans Stringer Moment and Reaction Table 5-29
- S-30 Fixed Spans Steel Details I 5-31 Fixed Spans Steel Details II
- Fixed Spans Steel Details III 5-32
- Fixed Spans Slot Railing Details
- Fixed Spans Anchor Column Floorbeam Details 5-34
- S-35 Bearing Details
- 5-36 Bascule Span: Deck Plan
- Bascule Span: Deck Details S-37
- Bascule Span: Curb Details 5-38
- Bascule Span: Sidewalk Plan 5-39
- 5-40 Bascule Span: Sidewalk Details 5-41 Bascule Span: Center Break Details
- 5-42 Bascule Span: Rear Break Details
- 5-43 Bascule Span: Framing Plan
- 5-44 Bascule Span: Gussets for Lower Lateral Bracing I
- Bascule Span: Gussets for Lower Lateral Bracing II
- 5-46 Bascule Span: Lower Lateral Bracing I
- 5-47 Bascule Span: Lower Lateral Bracing II
- 5-48 Bascule Span: Floorbeam 0-0
- Bascule Span: Floorbeam 2-2, 4-4, 6-6 & 8-8 5-49
- Bascule Span: Floorbeam Details I S-50
- Bascule Span: Floorbeam Details II 5-51
- Bascule Span: Floorbeam Details III S-52
- 5-53 Bascule Span: Floorbeam 10-10
- Bascule Span: Floorbeam 10-10 Details 5-54 Bascule Span: SW Truss Repairs
- 5-56 Bascule Span: SE Truss Repairs
- S-57 Bascule Span: NE Truss Repairs
- S-58 Bascule Span: NW Truss Repairs
- Bascule Span: Truss PPO to PP2 Repairs S-59
- Bascule Span: Truss PP4 to PP6 Repairs I 5-60
- Bascule Span: Truss PP4 to PP6 Repairs II 5-61
- Bascule Span: Truss PP8 Repairs I
- S-63 Bascule Span: Truss PP8 Repairs II
- Bascule Span: Truss PP10 Repairs I 5-64
- 5-65 Bascule Span: Truss PP10 Repairs II
- Bascule Span: Truss PPT Repairs 5-66
- S-67 Bascule Span: Truss PP9 & PP15 to PP16 Repairs
- 5-68 Bascule Span: Lattice Strut Repair Details
- 5-69 Bascule Span: Moment and Reaction Tables
- 5-70 Bascule Span: Bridge Balancing
- 5-71 Suggested Temporary Support Detail at Counterweight Pit
- Live Load Bearing Refurbishing
- Anchor Column Replacement Details I 5-73
- S-74 Anchor Column Replacement Details II

INDEX OF SHEETS (CONT.)

- S-75 Anchor Column Replacement Details III
- Trunnion Truss Repairs 5-76
- S-77 Trunnion Truss Bracing Repairs
- 5-78 Longitudinal Girder & Machinery Girder Repair Details I
- Longitudinal Girder & Machinery Girder Repair Details II
- Enclosure Walls: Removal Details I
- Enclosure Walls: Removal Details II 5-81
- Enclosure Walls: Plan & Elevation SW 5-82
- S-83 Enclosure Walls: Plan & Elevation - NW
- 5-84 Enclosure Walls: Plan & Elevation - NE
- S-85 Enclosure Walls: Plan & Elevation - SE
- Enclosure Walls: Details I 5-86
- 5-87 Enclosure Walls: Details II
- Enclosure Walls: Details III 5-88
- Enclosure Walls: House Slab Details 5-89 5-90 Enclosure Walls: House Stairwell Details
- Bridge House: Structural Details I 5-91
- Bridge House: Structural Details II
- Bridge House: Structural Details III 5-93
- 5-94 West Abutment Details I 5-95 West Abutment Details II
- East Abutment Details I 5-96
- 5-97 East Abutment Details II 5-98
- West River Pier Repair Details 5-99 East River Pier Repair Details
- S-100 Platform And Ladder Details West And East Fixed Spans
- S-101 West Pit Repair Details
- S-102 East Pit Repair Details
- S-103 Northeast Retaining Wall Plan And Elevation I
- S-104 Northeast Retaining Wall Plan And Elevation II
- S-105 Southeast Retaining Wall Plan And Elevation I
- S-106 Southeast Retaining Wall Plan And Elevation II Southeast Retaining Wall Plan And Elevation III
- S-108 East Retaining Walls Railing Details
- S-109 Steel Railing Details
- S-110 Dolphins And Pier Protection I
- S-111 Dolphins And Pier Protection II
- S-112 Boring Logs I
- S-113 Boring Logs II

DESIGN SPECIFICATIONS

- 2017 AASHTO LRFD Bridge Design Specifications 8th Ed. (Prop. Rdwy. Stringers & Floorbeams) 2007 AASHTO LRFD Movable Highway Bridge
- Design Specifications 2nd Ed. with 2008, 2010,
- 2011, 2012, and 2014 Interim Revisions 2002 AASHTO 17th Ed. (Exist. Structure
- Rehabilitation) 2009 AASHTO LRFD Guide Specifications for the Design of Pedestrian Bridges, 2nd Ed. with

2015 Interim Revisions (Prop. Sdwk. Stringers)

LOADING HS20-44 (ROADWAY)

No allowance for future wearing surface

LOADING (SIDEWALK)

Uniform live load for 100psf (pedestrian) H-5 (not in concurrence with pedestrian)

DESIGN STRESSES

FIELD UNITS (New Construction)

- f'c = 3,500 psi
- f'c = 4,000 psi (superstructure concrete) fy = 60,000 psi (reinforcement)fy = 50,000 psi (AASHTO M270, Gr. 50)
- FIELD UNITS (Existing Construction)
- f'c = 3.500 psi
- fy = 40,000 psi (Reinforcement)
- fy = 30,000 psi (Structural Steel)

LEGEND

- R&R Remove & replace
- Truss Panel Point (PP) designation
- Exist. fastener to remain
- Hole to match existing location
- Northwest NW
- Southeast Boring Location

Site Coefficient (S) = 1.0

New hole

SEISMIC DATA

F.A.U.

Seismic Performance Category (SPC) = A Horizontal Bedrock Acceleration Coefficient (A) = 0.025g

nc.	USER	NAME	=	1
LLE STREET				
782-8150	PLOT	SCALE	=	٨
782-1684	PLOT	DATE	=	1

RIE.			
1388	11-E1525-00-BR	соок	S-3
CDOT	PROJECT NO. E-1-525		46 of 210

COUNTY SHEET NO.

SECTION

SCOPE OF WORK

- Remove the existing open steel grid deck and replace with steel grid half-filled with concrete deck.
- Remove existing 2" concrete filled steel grid sidewalk and replace with fiberglass sidewalk.
- 3. Remove and replace the existing fixed span decks and sidewalks.
- Install expansion joint between bascule and fixed spans between bridge leaves at the center break.
- Remove and replace steel stringers in the bascule span and remove and replace steel stringers in the fixed spans.
- 6. Remove all the jackbeams of the bascule span.
- 7. Remove and replace all floorbeams in the bascule span.
- 8. Remove and replace all curb and roadway stringers adjacent to the trusses
- Remove and replace the bottom lateral bracing and horizontal bracing in the movable spans and over the counterweight pits. Repair deteriorated members of the lattice truss.
- 10. Perform repairs to the anchor columns and the anchor column girder
- 11. Repair and strengthen the main trusses in areas with documented section loss.
- 12. Clean and paint the entire steel superstructure and substructure (anchor columns) with containment and disposal of any existing lead based paint.
- 13. Remove, modify, clean and paint, and reinstall the steel railing of the bascule span.
- 14. Remove and replace the ornamental concrete railing on the fixed spans.
- 15. Repair and adjust the live load bearings.
- 16. Perform bridge balancing or counterweight adjustments to account for additional dead load on the bridge.
- 17. Dewater and clear debris from East and West counterweight pits followed by structural repair of concrete and epoxy crack injection to the pit walls and floor slab.
- 18. Repair damaged concrete at East and West River Piers using formed concrete repair and epoxy crack injection as required.
- 19. Remove and reconstruct broken machine room walls above the East River Pier.
- 20. Remove the existing damaged dolphins and fender system and replace with new dolphins and approved pier protection system.
- 21. Repair all damaged areas to the Northeast and Southeast Retaining walls using formed concrete repair and epoxy crack injections. Remove all graffiti from the faces of the walls. Remove and replace railing on top of the retaining walls with an ornamental steel railing.
- 22. Repair concrete abutment stems using formed concrete repair and epoxy crack injections.
- 23. Remove existing bearing pedestals, repair surrounding concrete and clean bearing area to accommodate new bearings.
- 24. Remove and replace the existing abutment backwalls to be replaced with semi-integral backwall. Construct approach slabs at east end.
- 25. Repair any holes or missing damaged portions of machine room enclosure walls. Tuck point where required and remove all graffiti from faces of walls.
- 26. For the rehabilitation of the bridge houses, remove the existing roof, reconstruct upper portion of third level and roof, repair existing, and remove hazardous materials.
- 27. Clean and paint existing mechanical equipment with exception of center lock assemblies. Remove existing auxiliary and mechanical center locks. Install new actuator-type locks with hand operation capability.
- 28. Stripe bridge deck for one 12'-6" wide shared bike lane in each direction (Eastbound and Westbound) and one 10'-0" wide left turn lane from Eastbound Webster Avenue to Southbound Ashland Avenue.
- 29. Reconstruct the sidewalk at the Northeast and Southeast corners of the Ashland Avenue and Webster Avenue intersection, all crosswalks at the intersection shall be constructed to current ADA standards.
- 30. Install electrical connection to the bridge.
- 31. Remove and install new navigational/obstruction lights.
- 32. Install two arterial street light poles and luminaires at the eastern end of the bridge. Install one davit arm light on each truss (four total).

TOTAL BILL OF MATERIAL

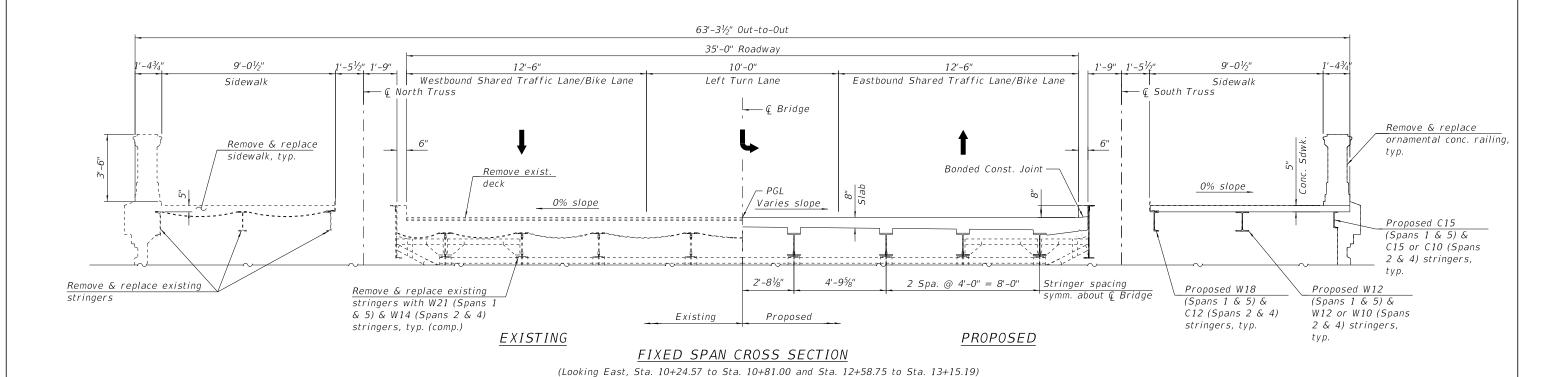
CODE NO.	ITEM	UNIT	TOTAL
20900110	POROUS GRANULAR BACKFILL	CU YD	375.0
50157300	PROTECTIVE SHIELD	SQ YD	1,188
50200100	STRUCTURE EXCAVATION	CU YD	86.0
50300260	BRIDGE DECK GROOVING	SQ YD	588
50300285	FORM LINER TEXTURED SURFACE	SQ FT	2,732
50500505	STUD SHEAR CONNECTORS	EACH	6,492
51500100	NAME PLATES	EACH	1
52100010	ELASTOMERIC BEARING ASSEMBLY, TYPE I	EACH	32
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	108,630
52000110	PREFORMED JOINT STRIP SEAL	FOOT	108
52100520	ANCHOR BOLTS, 1"	EACH	64
59000200	EPOXY CRACK INJECTION	FOOT	176
CD0T5010030	CONCRETE REMOVAL	CU YD	253.8
CD0T5030020	HIGH PERFORMANCE CONCRETE STRUCTURES	CU YD	258.0
CD0T5030030	HIGH PERFORMANCE CONCRETE SUPERSTRUCTURES	CU YD	256.3
CD0T5030050	CLASS SI CONCRETE MISC	CU YD	32.6
CD0T5870010	PROTECTIVE CONCRETE SEALER	SQ YD	1,100
X0323444	DECORATIVE STEEL RAILING	FOOT	210
X0326519	STEEL RAILING REMOVAL	FOOT	206
Z0001903	STRUCTURAL STEEL REMOVAL	POUND	486,420
Z0007101	CONTAINMENT AND DISPOSAL OF LEAD PAINT CLEANING RESIDUES NO. 1	L SUM	1
Z0012754	STRUCTURAL REPAIR OF CONCRETE (DEPTH EQUAL TO OR LESS THAN 5 INCHES)	SQ FT	589
Z0012755	STRUCTURAL REPAIR OF CONCRETE (DEPTH GREATER THAN 5 INCHES)	SQ FT	99
******	BALANCING OF BRIDGE AND ALTERATION OF COUNTERWEIGHTS	L SUM	1
******	BRIDGE OPERATION AND MAINTENANCE	L SUM	1
******	CLEANING AND PAINTING EXISTING STEEL STRUCTURES	L SUM	1
******	COUNTERWEIGHT PIT CLEANING	EACH	2
******	DOLPHINS	EACH	4
******	DRAINAGE SYSTEM	L SUM	1
******	FURNISHING AND ERECTING 5" GRATING, HALF CONCRETE FILLED	SQ FT	6,114
******	FURNISHING AND ERECTING FRP GRATING	SQ FT	3,227
******	FURNISHING AND ERECTING STRUCTURAL STEEL	L SUM	1
******	FURNISHING AND ERECTING STRUCTURAL STEEL, FIELD DISCOVERED	POUND	20,000
	CONDITIONS REPAIRED AS DIRECTED BY THE COMMISSIONER		
******	FLOOR ACCESS HATCH	EACH	4
******	METAL LADDERS	EACH	4
******	PIER PROTECTION REPLACEMENT	FOOT	301
******	REMOVAL OF DETERIORATED CONNECTORS AND REPLACEMENT WITH HIGH	EACH	51
	STRENGTH BOLTS		
******	REFURBISHING OF LIVE LOAD BEARINGS	EACH	4
******	REMOVAL OF EXISTING SUPERSTRUCTURES	EACH	1
******	REMOVAL OF EXISTING GRID DECK	L SUM	1
******	STEEL RAILING (SPECIAL)	FOOT	506
******	STRUCTURAL STEEL REPAIRS	POUND	61,620
******	TEMPORARY SUPPORT	L SUM	1

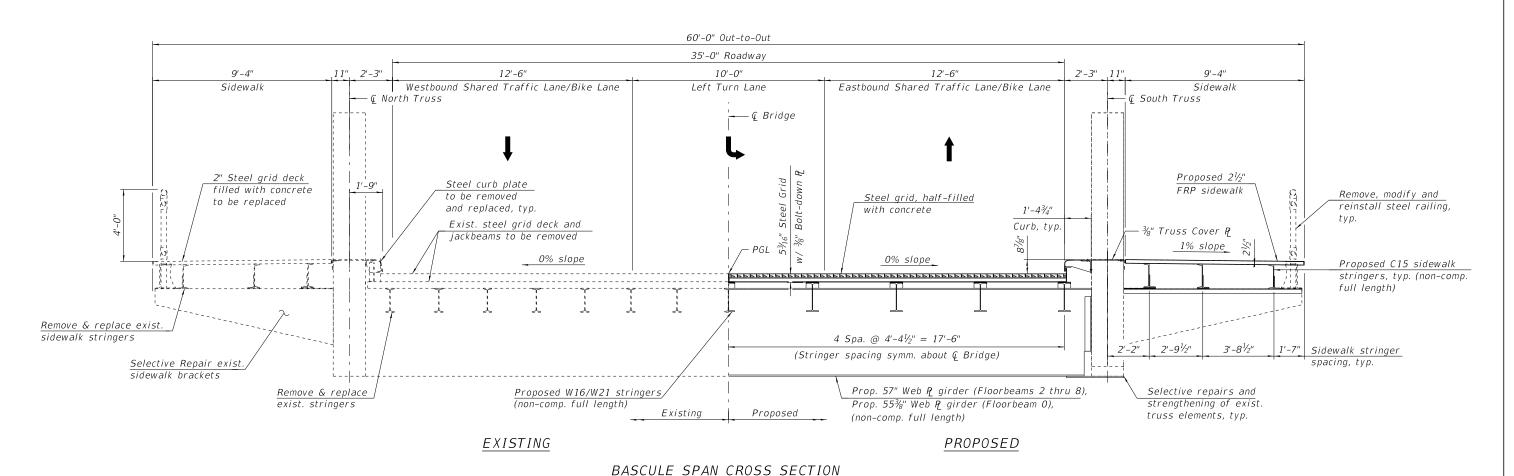
WEBSTER AVENUE BRIDGE
OVER
NORTH BRANCH CHICAGO RIVER
RE-BUILT 20-- BY
CITY OF CHICAGO
LOADING HS-20
STRUCTURE NO. 016-6057

NAME PLATE
See Std. 515001

USER NAME = IJLOPEZ	DESIGNED - IJL	REVISED -	
	CHECKED - JIG	REVISED -	
PLOT SCALE = N.T.S.	DRAWN - IJL	REVISED -	
PLOT DATE = 10/5/2020	CHECKED - JIG	REVISED -	

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-4
CDOT	PROJECT NO. E-1-525		47 of 210





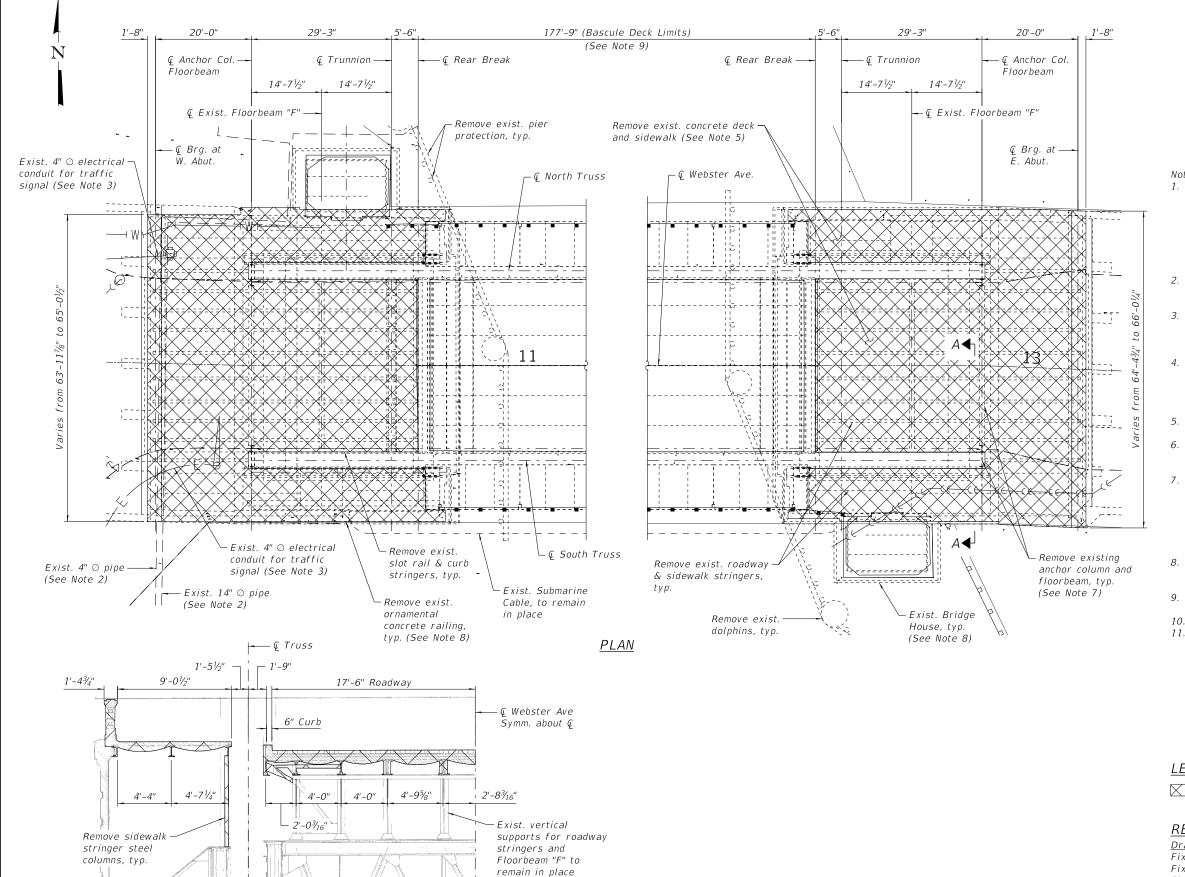
USER NAME = PJLAUX DESIGNED - NJP

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

(Looking East, Sta. 10+81.00 to Sta. 12+58.75)

THE NORTH BRANCH CHICAGO RIVER

SECTION COUNTY SHEET NO. **CROSS SECTIONS** S-5 11-E1525-00-BR COOK CDOT PROJECT NO. E-1-525 48 of 210



- 1. Existing fixed span roadway and sidewalk decks are comprised of a concrete deck and steel buckle plates. Additionally, the roadway deck includes an asphalt wearing surface. Removal of existing roadway asphalt, concrete, and steel fixed deck and its supporting existing steel framing including bearings shall be included in the cost of "Removal of Existing Superstructures".
- Existing pipes attached to existing fixed deck and steel framing shall be temporarily supported during construction. See Special Provisions.
- Existing electrical conduits on West Approach connecting to existing traffic signals shall remain in place. Conduits shall be protected during removal operations.
- The Contractor shall exercise extreme care during removal of abutment backwall and concrete deck to prevent damage to conduits. Any damage to the existing conduits to remain in place shall be repaired at the Contractor's expense.
- Removal of existing dolphins is included in the cost of "Dolphins".
- Removal of existing pier protection including all timber piles, wales, and anchors is included in the cost of "Pier Protection Replacement".
- Removal of existing floorbeam between anchor columns is included in the cost of "Removal of Existing Superstructures" and the removal of existing anchor column is included in the cost of "Structural Steel Removal". See sheet S-73 for additional removal details of the anchor column and floorbeam.
- 8. For details of removal of existing railings, enclosure walls, and bridge houses, see sheets S-80 & S-81 and architectural special provisions.
- 9. See sheet S-7 for existing structure removal for bascule span.
- 10. See sheet S-5 for fixed span cross section.
- 11. Removal of existing steel railing on approach retaining walls is included in "Steel Railing Removal"

BILL OF MATERIAL

Item	Unit	Quantity
Removal of Existing	Each	1
Superstructures		

LEGEND:

Removal of Existing Superstructures

REFERENCE DRAWINGS

<u>Drawing</u> Fixed Part General Plan Fixed Part Roadway Stringers Fixed Part Sidewalk Stringers Sheet No. 1660570015 1660570022 1660570023

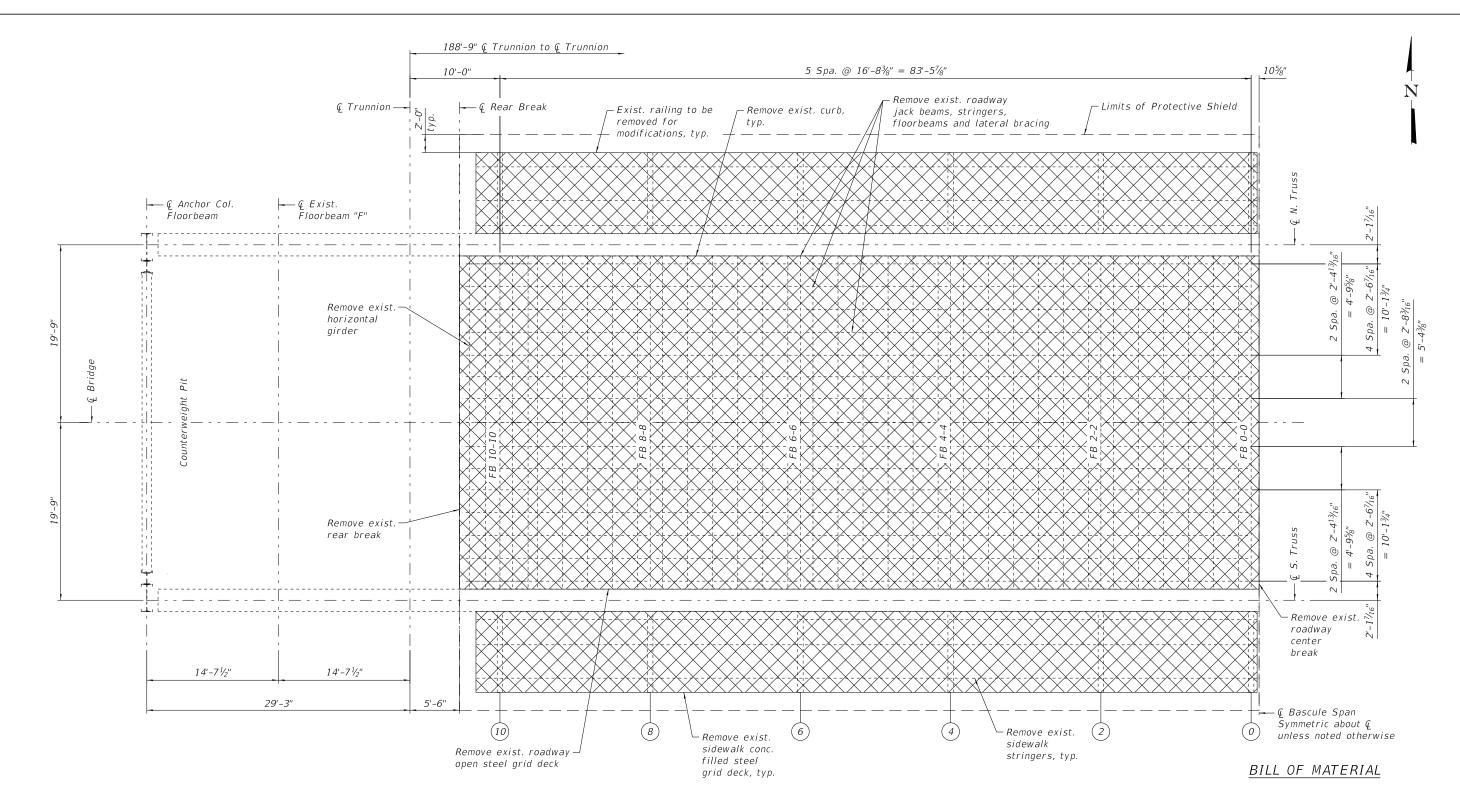
USER NAME = PJLAUX DESIGNED - PJL REVISED CHECKED - IJL REVISED REVISED PLOT DATE = \$DATF\$ CHECKED - JIG REVISED

SECTION A-A - TYPICAL FIXED SPAN HALF SECTION

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER **EXISTING PLANS AND REMOVAL** FIXED SPANS (STRUCTURE NO. 016-6057)

SECTION COUNTY SHEET NO. S-6 11-E1525-00-BR COOK CDOT PROJECT NO. E-1-525 49 of 210



Notes

 Removal of existing roadway and sidewalk open 4. steel and concrete filled grid shall be included in the cost of "Removal of Existing Grid Deck".

- 2. Removal of existing roadway rear breaks, center break, jack beams, stringers, floorbeams, lateral bracing, horizontal girders, and sidewalk stringers shall be included in the cost of "Structural Steel Removal".
- 3. See sheet S-109 for details of modifications to existing steel railings.
- The existing lateral bracing has been modified from the original and is comprised of a cable system. The details of the existing cable system are not reflected in the existing drawings but can be found in the 2016 emergency repair plans. The Contractor shall take care to document and weigh the material removed in accordance with the Bridge Balancing special provision.
- 5. See sheet S-5 for bascule span cross section.
- See sheet S-6 for existing structure removal for fixed spans.

EXISTING STRUCTURE REMOVAL PLAN - BASCULE SPAN

(West Leaf shown, East Leaf opposite hand)

Item	Unit	Quantity
Structural Steel Removal	Pound	452,820
Removal of Existing Grid Deck	L. Sum	1
Protective Shield	Sq. Yd.	1,188

LEGEND:

Removal of Existing Grid Deck

REFERENCE DRAWINGS

<u>Drawing</u> Movable Part- Stringers & Sub-Planking Stringers and Sidewalk Brackets

<u>Sheet No.</u> 1660570015 1660570209

115)	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684
-----	---	---

	USER NAME = PJLAUX	DESIGNED - PJL	REVISED -
т		CHECKED - IJL	REVISED -
)	PLOT SCALE = N.T.S.	DRAWN - PJL	REVISED -
•	PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -

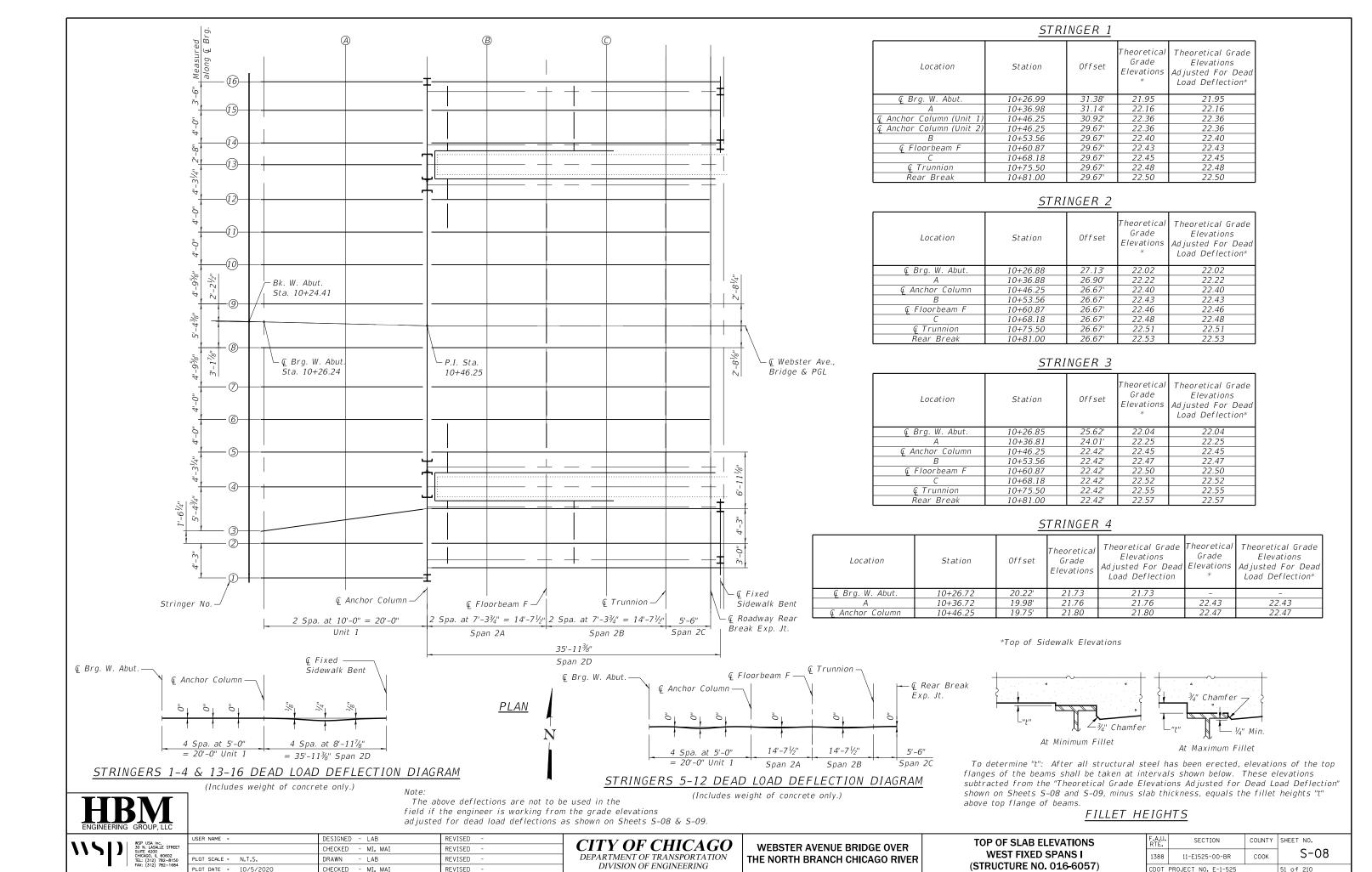
CITY OF CHICAGO

DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER
THE NORTH BRANCH CHICAGO RIVER

EXISTING PLANS AND REMOVAL BASCULE SPAN (STRUCTURE NO. 016-6057)

F.A.U. RTE.	SECTION	COUNTY	SHEET	NO.
1388	11-E1525-00-BR	соок		S-7
CDOT	PROJECT NO. E-1-525		50 of	210



STRINGER 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
♀ Brg. W. Abut.	10+26.62	15.95'	21.75	21.75
A	10+36.62	15.72'	21.78	21.78
ℚ Anchor Column	10+46.25	15.48'	21.82	21.82
В	10+53.56	15.48'	21.84	21.84
⊊ Floorbeam F	10+60.87	15.48'	21.86	21.86
С	10+68.18	15.48'	21.88	21.88
⊈ Trunnion	10+75.50	15.48'	21.90	21.90
Rear Break	10+81.00	15.48'	21.92	21.92

STRINGER 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
♀ Brg. W. Abut.	10+26.53	11.95'	21.78	21.78
Α	10+36.52	11.72'	21.80	21.80
⊈ Anchor Column	10+46.25	11.48'	21.83	21.83
В	10+53.56	11.48'	21.85	21.85
⊊ Floorbeam F	10+60.87	11.48'	21.87	21.87
С	10+68.18	11.48'	21.89	21.89
← Trunnion	10+75.50	11.48'	21.91	21.91
Rear Break	10+81.00	11.48'	21.92	21.92

STRINGER 7

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
♀ Brg. W. Abut.	10+26.43	7.96'	21.80	21.80
Α	10+36.43	7.72'	21.82	21.82
ℚ Anchor Column	10+46.25	7.48'	21.84	21.84
В	10+53.56	7.48'	21.86	21.86
⊈ Floorbeam F	10+60.87	7.48'	21.88	21.88
С	10+68.18	7.48'	21.89	21.89
⊈ Trunnion	10+75.50	7.48'	21.91	21.91
Rear Break	10+81.00	7.48'	21.92	21.92

STRINGER 8

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
♀ Brg. W. Abut.	10+26.32	3.15'	21.83	21.83
Α	10+36.31	2.92'	21.84	21.84
€ Anchor Column	10+46.25	2.68'	21.86	21.86
В	10+53.56	2.68'	21.87	21.87
⊈ Floorbeam F	10+60.87	2.68'	21.89	21.89
С	10+68.18	2.68'	21.90	21.90
← Trunnion	10+75.50	2.68'	21.91	21.91
Rear Break	10+81.00	2.68'	21.92	21.92

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
ℚ Brg. W. Abut.	10+26.24	0.00'	21.84	21.84
Α	10+36.25	0.00'	21.86	21.86
€ Anchor Column	10+46.25	0.00'	21.87	21.87
В	10+53.56	0.00'	21.88	21.88
⊊ Floorbeam F	10+60.87	0.00'	21.89	21.89
С	10+68.18	0.00'	21.90	21.90
⊈ Trunnion	10+75.50	0.00'	21.91	21.91
Rear Break	10+81.00	0.00'	21.92	21.92

STRINGER 12

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection		
⊈ Brg. W. Abut.	10+25.89	-15.01'	21.76	21.76		
Α	10+35.88	-15.24'	21.79	21.79		
⊈ Anchor Column	10+46.25	-15.48'	21.82	21.82		
В	10+53.56	-15.48'	21.84	21.84		
⊊ Floorbeam F	10+60.87	-15.48'	21.86	21.86		
С	10+68.18	-15.48'	21.88	21.88		
€ Trunnion	10+75.50	-15.48'	21.90	21.90		
Rear Break	10+81.00	-15.48'	21.92	21.92		

STRINGER 13

Location	Station	Offset	Theoretical Grade Elevations	Elevations	Grade	Theoretical Grade Elevations Adjusted For Dead Load Deflection*	
€ Brg. W. Abut.	10+25.79	-19.27'	21.73	21.73	22.34	22.34	
Α	10+35.78	-19.51'	21.77	21.77	22.43	22.43	
← Anchor Column	10+46.25	-19.75'	21.80	21.80	22.47	22.47	

STRINGER 9

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
⊈ Brg. W. Abut.	10+26.19	-2.21'	21.83	21.83
Α	10+36.19	-2.44'	21.85	21.85
	10+46.25	-2.68'	21.86	21.86
В	10+53.56	-2.68'	21.87	21.87
⊈ Floorbeam F	10+60.87	-2.68'	21.89	21.89
С	10+68.18	-2.68'	21.90	21.90
	10+75.50	-2.68'	21.91	21.91
Rear Break	10+81.00	-2.68'	21.92	21.92

STRINGER 10

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
♀ Brg. W. Abut.	10+26.08	-7.01'	21.80	21.80
Α	10+36.07	-7.25'	21.82	21.82
∉ Anchor Column	10+46.25	-7.48'	21.84	21.84
В	10+53.56	-7.48'	21.86	21.86
⊈ Floorbeam F	10+60.87	-7.48'	21.88	21.88
С	10+68.18	-7.48'	21.89	21.89
⊈ Trunnion	10+75.50	-7.48'	21.91	21.91
Rear Break	10+81.00	-7.48'	21.92	21.92

STRINGER 11

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
♀ Brg. W. Abut.	10+25.98	-11.01'	21.78	21.78
Α	10+35.98	-11.24'	21.81	21.81
← Anchor Column	10+46.25	-11.48'	21.83	21.83
В	10+53.56	-11.48'	21.85	21.85
⊊ Floorbeam F	10+60.87	-11.48'	21.87	21.87
С	10+68.18	-11.48'	21.89	21.89
⊈ Trunnion	10+75.50	-11.48'	21.91	21.91
Rear Break	10+81.00	-11.48'	21.92	21.92

STRINGER 14

Location	Station	Offset	Theoretical Grade Elevations *	Theoretical Grade Elevations Adjusted For Dead Load Deflection*
⊈ Brg. W. Abut.	10+25.72	-21.94'	22.34	22.34
Α	10+35.72	-22.17'	22.39	22.39
⊈ Anchor Column	10+46.25	-22.42'	22.44	22.44
В	10+53.56	-22.42'	22.48	22.48
⊊ Floorbeam F	10+60.87	-22.42'	22.52	22.52
С	10+68.18	-22.42'	22.55	22.55
⊈ Trunnion	10+75.50	-22.42'	22.59	22.59
Rear Break	10+81.00	-22.42'	22.62	22.62

STRINGER 15

Location	Station	Offset	Theoretical Grade Elevations *	Theoretical Grade Elevations Adjusted For Dead Load Deflection*
⊊ Brg. W. Abut.	10+25.63	-25.94'	22.34	22.34
Α	10+35.63	-26.17'	22.38	22.38
ℚ Anchor Column	10+46.25	-26.42'	22.43	22.43
В	10+53.56	-26.42'	22.46	22.46
⊊ Floorbeam F	10+60.87	-26.42'	22.49	22.49
С	10+68.18	-26.42'	22.52	22.52
⊈ Trunnion	10+75.50	-26.42'	22.55	22.55
Rear Break	10+81.00	-26.42'	22.58	22.58

STRINGER 16

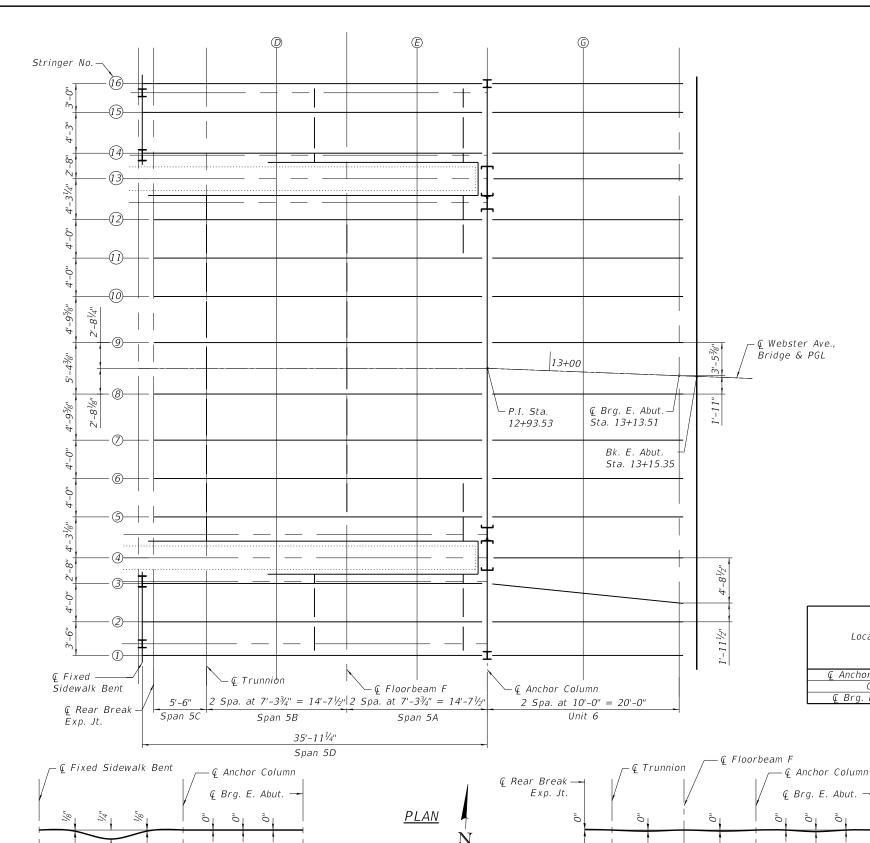
Location	Station	Offset	Theoretical Grade Elevations *	Theoretical Grade Elevations Adjusted For Dead Load Deflection*
⊊ Brg. W. Abut.	10+25.55	-29.43'	22.34	22.34
Α	10+35.54	-29.67'	22.38	22.38
⊈ Anchor Column	10+46.25	-29.92'	22.42	22.42
В	10+53.56	-29.92'	22.44	22.44
⊊ Floorbeam F	10+60.87	-29.92'	22.47	22.47
С	10+68.18	-29.92'	22.50	22.50
⊈ Trunnion	10+75.50	-29.92'	22.52	22.52
Rear Break	10+81.00	-29.92'	22.54	22.54

*Top of Sidewalk Elevations



USER NAME =	DESIGNED - LAB	REVISED -
	CHECKED - MI, MAI	REVISED -
PLOT SCALE = N.T.S.	DRAWN - LAB	REVISED -
PLOT DATE = 10/5/2020	CHECKED - MI, MAI	REVISED -

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	COOK	S-09
CDOT	PROJECT NO. E-1-525		52 of 210



STRINGER 1

Location	Station	Offset	Theoretical Grade Elevations*	Theoretical Grade Elevations Adjusted For Dead Load Deflection*
Rear Break	12+58.75	29.92'	22.54	22.54
⊈ Trunnion	12+64.25	29.92'	22.46	22.46
D	12+71.56	29.92'	22.42	22.42
⊈ Floorbeam F	12+78.88	29.92'	22.38	22.38
Ε	12+86.19	29.92'	22.16	22.16
← Anchor Column	12+93.50	29.92'	21.94	21.94
G	13+04.64	29.51'	21.64	21.64
⊈ Brg. E. Abut.	13+14.63	29.13'	21.34	21.34

STRINGER 2

Location	Station	Offset	Theoretical Grade Elevations*	Theoretical Grade Elevations Adjusted For Dead Load Deflection*
Rear Break	12+58.75	26.42'	22.57	22.57
€ Trunnion	12+64.25	26.42'	22.50	22.50
D	12+71.56	26.42'	22.46	22.46
⊊ Floorbeam F	12+78.88	26.42'	22.42	22.42
E	12+86.19	26.42'	22.20	22.20
€ Anchor Column	12+93.50	26.42'	21.97	21.97
G	13+04.50	26.01'	21.67	21.67
⊈ Brg. E. Abut.	13+14.50	25.63'	21.37	21.37

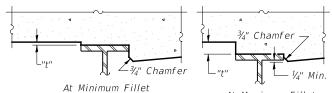
STRINGER 3

Location	Station	Offset	Theoretical Grade Elevations*	Elevations
Rear Break	12+58.75	22.42'	22.61	22.61
€ Trunnion	12+64.25	22.42'	22.54	22.54
D	12+71.56	22.42'	22.50	22.50
⊊ Floorbeam F	12+78.88	22.42'	22.46	22.46
E	12+86.19	22.42'	22.24	22.24
	12+93.50	22.42'	22.01	22.01
G	13+04.39	23.03'	21.70	21.70
⊈ Brg. E. Abut.	13+14.42	23.67'	21.39	21.39

STRINGER 4

Location	Station	Offset	Grado	Elevations	Grado	Theoretical Grade Elevations Adjusted For Dead Load Deflection*
← Anchor Column	12+93.50	19.75'	21.37	21.37	22.03	22.03
Ğ	13+04.25	19.35'	21.06	21.06	21.73	21.73
⊊ Brg. E. Abut.	13+14.24	18.97'	20.76	20.76	_	-

*Top of Sidewalk Elevations



At Maximum Fillet

To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown below. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown on Sheets S-10 and S-11, minus slab thickness, equals the fillet heights "t" above top flange of beams.

FILLET HEIGHTS

The above deflections are not to be used in the field if the engineer is working from the grade elevations

adjusted for dead load deflections as shown on Sheets S-10 & S-11. REVISED

TOP OF SLAB ELEVATIONS EAST FIXED SPANS I (STRUCTURE NO. 016-6057)

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-10
CDOT	PROJECT NO. E-1-525		53 of 210

USER NAME = PLOT SCALE = N.T.S. PLOT DATE = 10/5/2020

4 Spa. at 8'-111%"

= 35'-11¾" Span 5D

STRINGERS 1-4 & 13-16 DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete only.)

DESIGNED - LAB CHECKED - MI, MAI REVISED DRAWN REVISED CHECKED - MI, MAI REVISED

4 Spa. at 5'-0"

= 20'-0" Unit 6

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

5'-6"

14'-71/2"

Span 5C Span 5B

14'-71/2"

Span 5A

STRINGERS 5-12 DEAD LOAD DEFLECTION DIAGRAM

(Includes weight of concrete only.)

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

4 Spa. at 5'-0"

= 20'-0" Unit 6

STRINGER 5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection		
Rear Break	12+58.75	15.48'	21.92	21.92		
⊈ Trunnion	12+64.25	15.48'	21.89	21.89		
D	12+71.56	15.48'	21.85	21.85		
⊊ Floorbeam F	12+78.88	15.48'	21.81	21.81		
Е	12+86.19	15.48'	21.59	21.59		
← Anchor Column	12+93.50	15.48'	21.37	21.37		
G	13+04.08	15.09'	21.06	21.06		
Ç Brg. E. Abut.	13+14.08	14.70'	20.76	20.76		

STRINGER 6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Rear Break	12+58.75	11.48'	21.92	21.92
← Trunnion	12+64.25	11.48'	21.89	21.89
D	12+71.56	11.48'	21.85	21.85
⊊ Floorbeam F	12+78.88	11.48'	21.81	21.81
Ε	12+86.19	11.48'	21.59	21.59
⊈ Anchor Column	12+93.50	11.48'	21.37	21.37
G	13+03.93	11.09'	21.06	21.06
⊊ Brg. E. Abut.	13+13.92	10.71'	20.76	20.76

STRINGER 7

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Rear Break	12+58.75	7.48'	21.92	21.92
⊈ Trunnion	12+64.25	7.48'	21.89	21.89
D	12+71.56	7.48'	21.85	21.85
⊈ Floorbeam F	12+78.88	7.48'	21.81	21.81
Ε	12+86.19	7.48'	21.59	21.59
⊈ Anchor Column	12+93.50	7.48'	21.37	21.37
G	13+03.78	7.09'	21.06	21.06
⊊ Brg. E. Abut.	13+13.77	6.71'	20.76	20.76

STRINGER 8

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Rear Break	12+58.75	2.68'	21.92	21.92
⊈ Trunnion	12+64.25	2.68'	21.89	21.89
D	12+71.56	2.68'	21.85	21.85
⊊ Floorbeam F	12+78.88	2.68'	21.81	21.81
Ε	12+86.19	2.68'	21.59	21.59
← Anchor Column	12+93.50	2.68'	21.37	21.37
G	13+03.59	2.30'	21.06	21.06
♀ Brg. E. Abut.	13+13.59	1.91'	20.76	20.76

Location	Station	0ffset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Rear Break	12+58.75	0.00'	21.92	21.92
€ Trunnion	12+64.25	0.00'	21.89	21.89
D	12+71.56	0.00'	21.85	21.85
⊊ Floorbeam F	12+78.88	0.00'	21.81	21.81
Ε	12+86.19	0.00'	21.59	21.59
← Anchor Column	12+93.50	0.00'	21.37	21.37
G	13+03.50	0.00'	21.06	21.06
⊈ Brg. E. Abut.	13+13.51	0.00'	20.76	20.76
•	•		-	

STRINGER 12

<u> </u>					
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection	
Rear Break	12+58.75	-15.48'	21.92	21.92	
⊈ Trunnion	12+64.25	-15.48'	21.89	21.89	
D	12+71.56	-15.48'	21.85	21.85	
⊈ Floorbeam F	12+78.88	-15.48'	21.81	21.81	
E	12+86.19	-15.48'	21.59	21.59	
⊈ Anchor Column	12+93.50	-15.48'	21.37	21.37	
G	13+02.89	-15.86'	21.06	21.06	
⊈ Brg. E. Abut.	13+12.89	-16.24'	20.76	20.76	

STRINGER 13

Location	Station	Offset	Grado	Theoretical Grade Elevations Adjusted For Dead Load Deflection	Grado	Elevations
€ Anchor Column	12+93.50	-19.75'	21.37	21.37	22.03	22.03
G	13+02.73	-20.12'	21.06	21.06	21.73	21.73
⊈ Brg. E. Abut.	13+12.72	-20.50'	20.76	20.76	-	=

STRINGER 9

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Rear Break	12+58.75	-2.68'	21.92	21.92
⊈ Trunnion	12+64.25	-2.68'	21.89	21.89
D	12+71.56	-2.68'	21.85	21.85
⊈ Floorbeam F	12+78.88	-2.68'	21.81	21.81
E	12+86.19	-2.68'	21.59	21.59
← Anchor Column	12+93.50	-2.68'	21.37	21.37
G	13+03.39	-3.06'	21.06	21.06
⊊ Brg. E. Abut.	13+13.38	-3.45'	20.76	20.76

STRINGER 10

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Rear Break	12+58.75	-7.48'	21.92	21.92
⊈ Trunnion	12+64.25	-7.48'	21.89	21.89
D	12+71.56	-7.48'	21.85	21.85
⊊ Floorbeam F	12+78.88	-7.48'	21.81	21.81
E	12+86.19	-7.48'	21.59	21.59
	12+93.50	-7.48'	21.37	21.37
G	13+03.20	-7.86′	21.06	21.06
⊊ Brg. E. Abut.	13+13.19	-8.25'	20.76	20.76

STRINGER 11

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Rear Break	12+58.75	-11.48'	21.92	21.92
€ Trunnion	12+64.25	-11.48'	21.89	21.89
D	12+71.56	-11.48'	21.85	21.85
⊊ Floorbeam F	12+78.88	-11.48'	21.81	21.81
E	12+86.19	-11.48'	21.59	21.59
	12+93.50	-11.48'	21.37	21.37
G	13+03.05	-11.86'	21.06	21.06
⊈ Brg. E. Abut.	13+13.04	-12.24'	20.76	20.76

STRINGER 14

Location	Station	Offset	Theoretical Grade Elevations*	Theoretical Grade Elevations Adjusted For Dead Load Deflection*
Rear Break	12+58.75	-22.42'	22.61	22.61
€ Trunnion	12+64.25	-22.42'	22.54	22.54
D	12+71.56	-22.42'	22.50	22.50
⊊ Floorbeam F	12+78.88	-22.42'	22.46	22.46
Ε	12+86.19	-22.42'	22.24	22.24
€ Anchor Column	12+93.50	-22.42'	22.01	22.01
G	13+02.63	-22.78'	21.71	21.71
ℚ Brg. E. Abut.	13+12.62	-23.17'	21.40	21.40

STRINGER 15

Location	Station	Offset	Theoretical Grade Elevations*	Theoretical Grade Elevations Adjusted For Dead Load Deflection*
Rear Break	12+58.75	-26.67'	22.57	22.57
€ Trunnion	12+64.25	-26.67'	22.50	22.50
D	12+71.56	-26.67'	22.46	22.46
⊊ Floorbeam F	12+78.88	-26.67'	22.42	22.42
E	12+86.19	-26.67'	22.19	22.19
← Anchor Column	12+93.50	-26.67'	21.97	21.97
G	13+02.47	-27.03'	21.66	21.66
⊈ Brg. E. Abut.	13+12.46	-27.42'	21.36	21.36

STRINGER 16

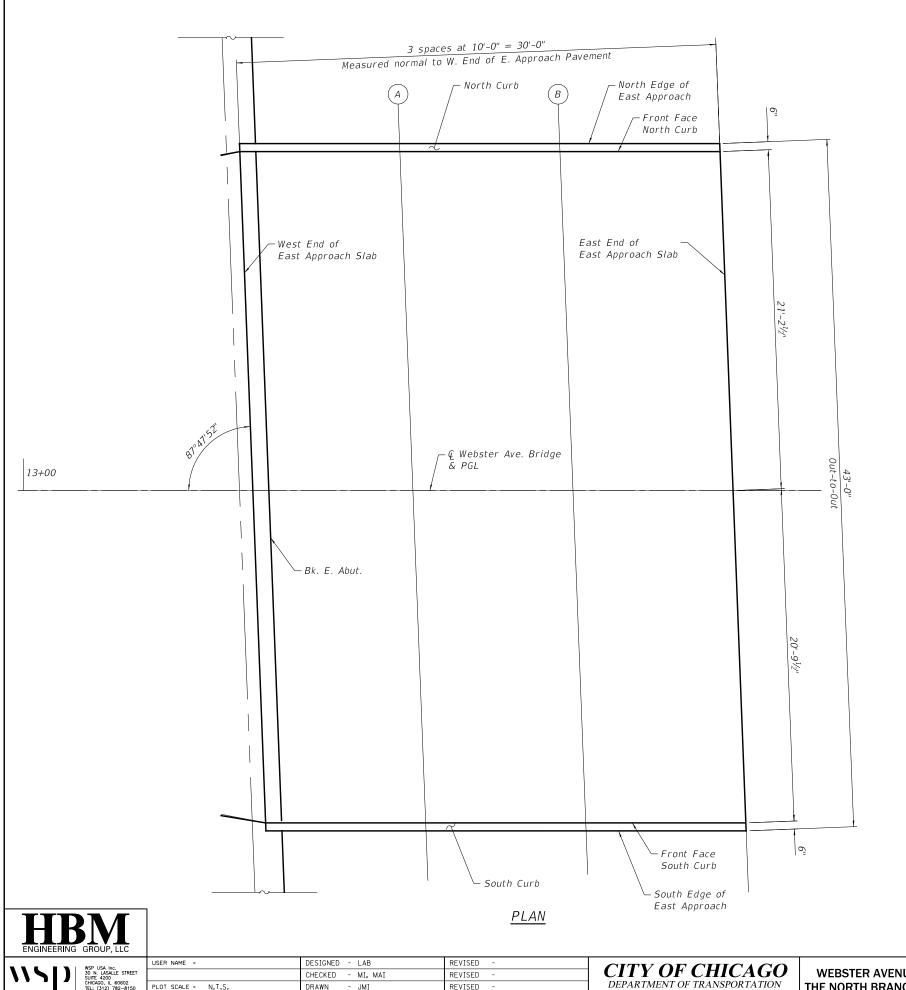
Location	Station	0ffset	Theoretical Grade Elevations*	Theoretical Grade Elevations Adjusted For Dead Load Deflection*
Rear Break	12+58.75	-29.67'	22.54	22.54
∉ Trunnion	12+64.25	-29.67'	22.47	22.47
D	12+71.56	-29.67'	22.43	22.43
⊊ Floorbeam F	12+78.88	-29.67'	22.39	22.39
Е	12+86.19	-29.67'	22.16	22.16
∉ Anchor Column	12+93.50	-29.67'	21.94	21.94
G	13+02.35	-30.03'	21.63	21.63
ℚ Brg. E. Abut.	13+12.34	-30.41'	21.33	21.33

*Top of Sidewalk Elevation



USER NAME =	DESIGNED - LAB	REVISED -
	CHECKED - MI, MAI	REVISED -
PLOT SCALE = N.T.S.	DRAWN - LAB	REVISED -
PLOT DATE = 10/5/2020	CHECKED - MI, MAI	REVISED -

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.	
1388	11-E1525-00-BR	соок	S-11	
CDOT	PROJECT NO. E-1-525		54 of 210	



FRONT FACE OF NORTH CURB

Location	Station	Offset	Theoretical
			Grade
			Elevations
W. End of E. Appr. Slab	13+13.53	-21.19'	20.76
Α	13+23.54	-21.19'	20.31
В	13+33.55	-21.19'	19.86
E. End of E. Appr. Slab	13+43.55	-21.19'	19.42

Location	Station	Offset	Theoretical
			Grade
			Elevations
W. End of E. Appr. Slab	13+14.35	0.00'	20.73
A	13+24.35	0.00'	20.43
В	13+34.36	0.00'	20.12
E. End of E. Appr. Slab	13+44.37	0.00'	19.82

FRONT FACE OF SOUTH CURB

Location	Station	0ffset	Theoretical
			Grade
			Elevations
W. End of E. Appr. Slab	13+15.14	20.77'	20.71
Α	13+25.15	20.77'	20.26
В	13+35.16	20.77'	19.82
E. End of E. Appr. Slab	13+45.17	20.77'	19.38

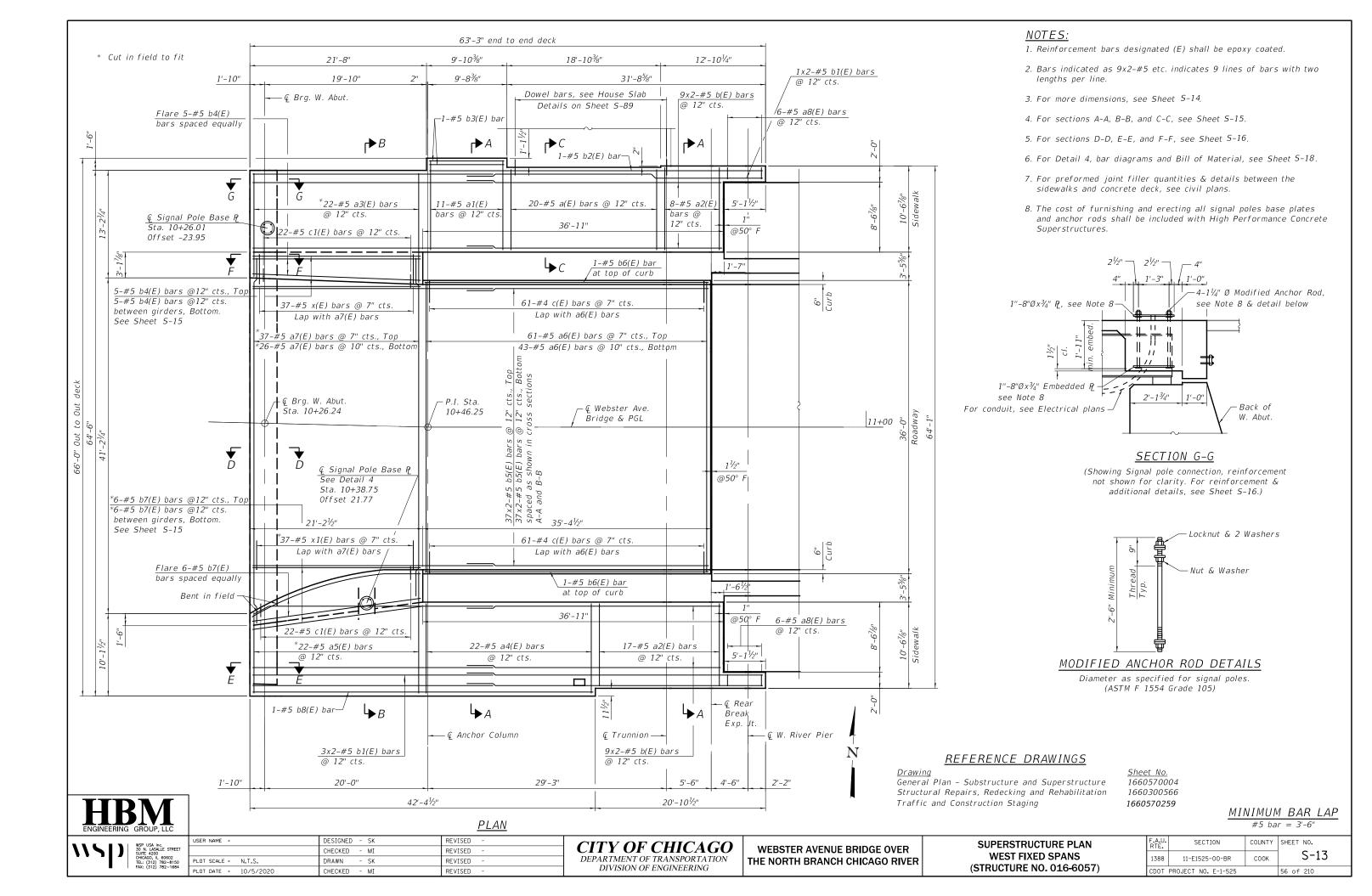
DRAWN REVISED PLOT DATE = 10/5/2020 CHECKED - MI, MAI REVISED

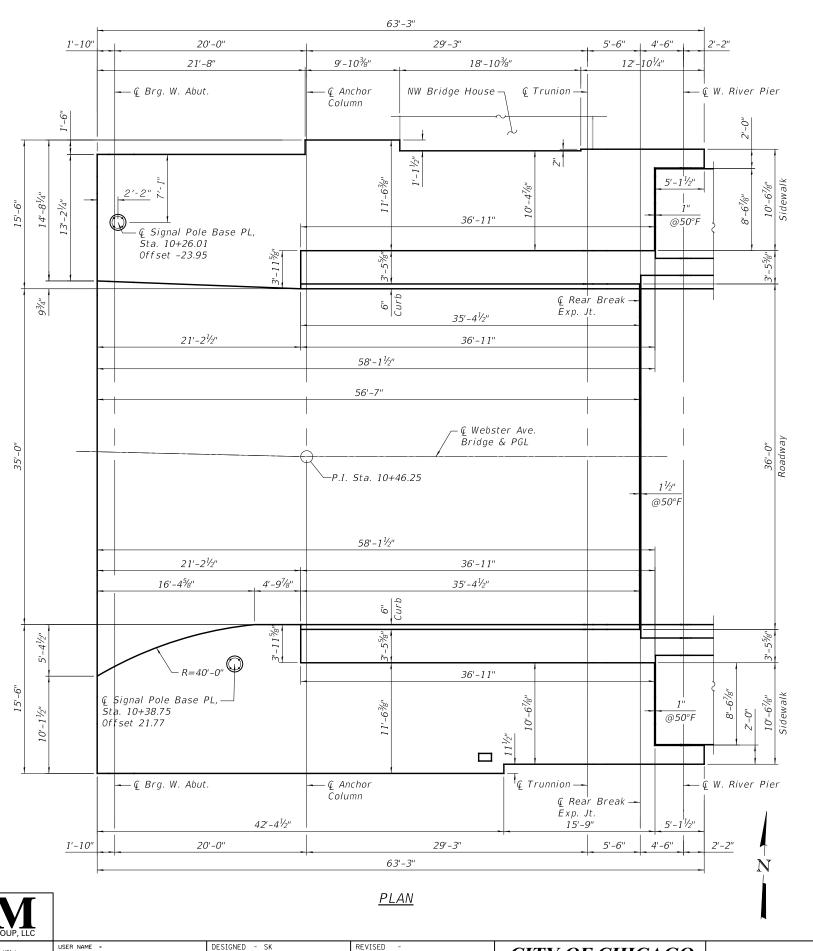
CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

TOP OF SLAB ELEVATIONS EAST APPROACH SLAB (STRUCTURE NO. 016-6057)

COUNTY SHEET NO. SECTION S-12 11-E1525-00-BR COOK CDOT PROJECT NO. E-1-525 55 of 210





WSP USA Inc.
30 N. LASALLE STREET
SUITE 4200
CHICAGO, II. 60602
TEL: (312) 782-8150
FAX: (312) 782-1684

 USER NAME =
 DESIGNED - SK
 REVISED

 CHECKED - MI
 REVISED

 PLOT SCALE = N.T.S.
 DRAWN - SK
 REVISED

 PLOT DATE = 10/5/2020
 CHECKED - MI
 REVISED

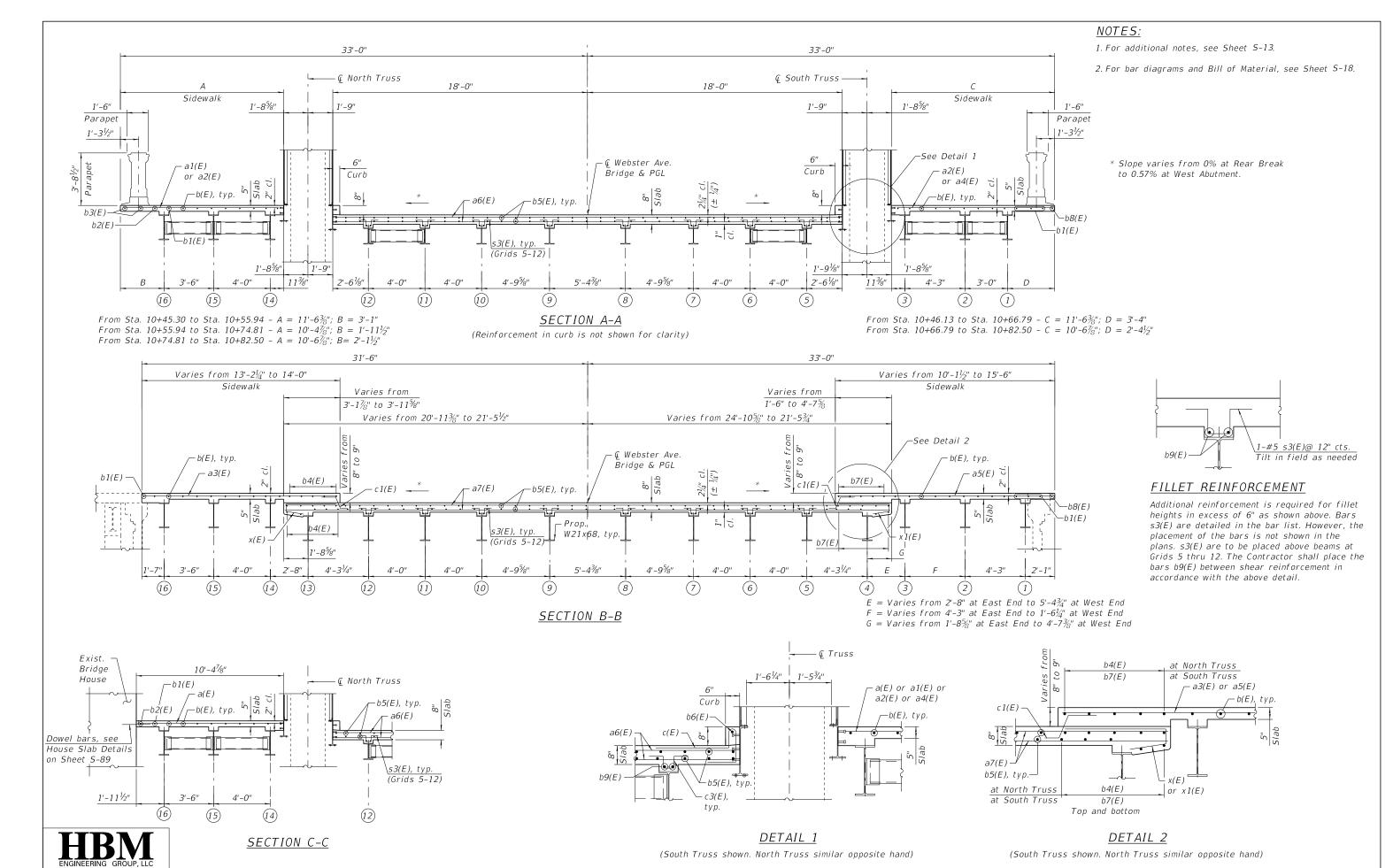
CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

SUPERSTRUCTURE PLAN GEOMETRICS WEST FIXED SPANS (STRUCTURE NO. 016-6057) F.A.U. SECTION COUNTY SHEET NO.

1388 11-E1525-00-BR COOK S-14

CDOT PROJECT NO. E-1-525 57 of 210



CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

USER NAME

PLOT SCALE = N.T.S.

PLOT DATE = \$DATF\$

DESIGNED - SK

CHECKED - MI

CHECKED - MI

DRAWN

REVISED

REVISED

REVISED

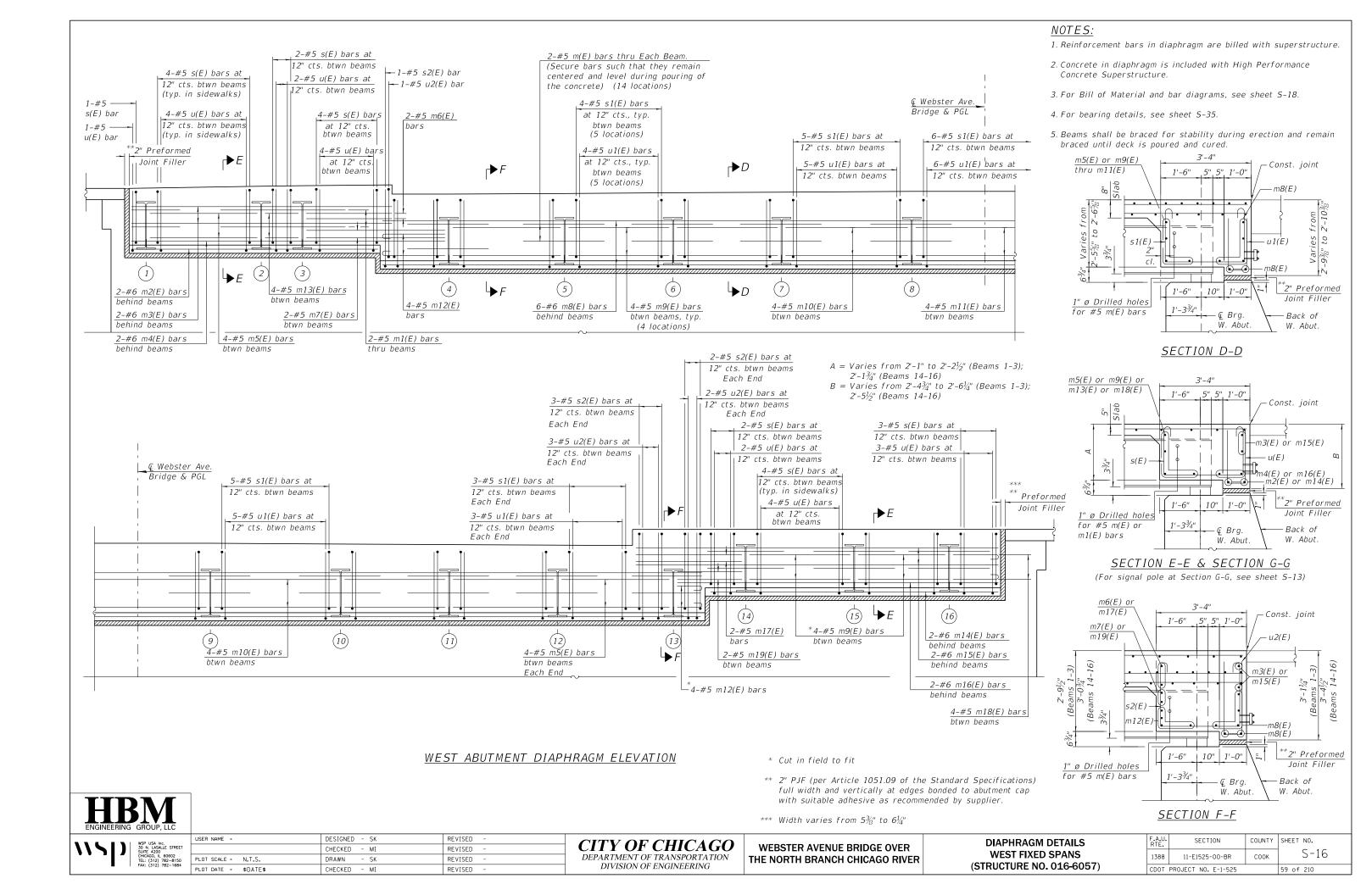
REVISED

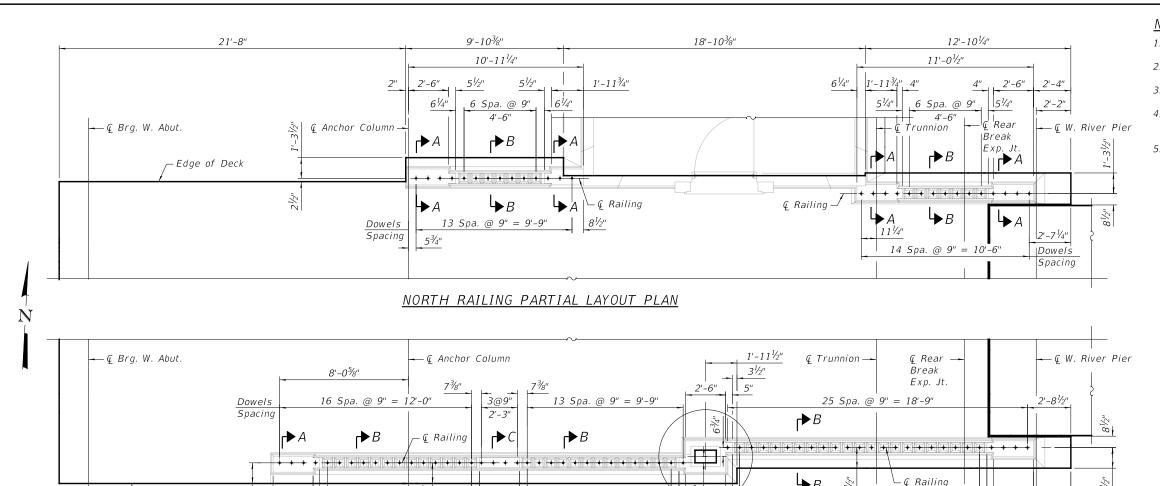
CROSS SECTIONS
WEST FIXED SPANS
(STRUCTURE NO. 016-6057)

F.A.U. SECTION COUNTY SHEET NO.

1388 11-E1525-00-BR COOK S-15

CDOT PROJECT NO. E-1-525 58 of 210





 $\blacktriangleright B$

12 Spa. @ 9" = 9'-0"

Detail 3-

NOTES:

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. See sheet S-15 for deck cross sections.
- 3. For bar diagrams and Bill of material, see sheet S-18
- 4. See architectural (A-series) sheets for precast railing details.
- 5. The Contractor must use approved single straight coil loop inserts when pendant mounting threaded rods to a sidewalk. The single straight loop inserts must be cast into the concrete sidewalk.

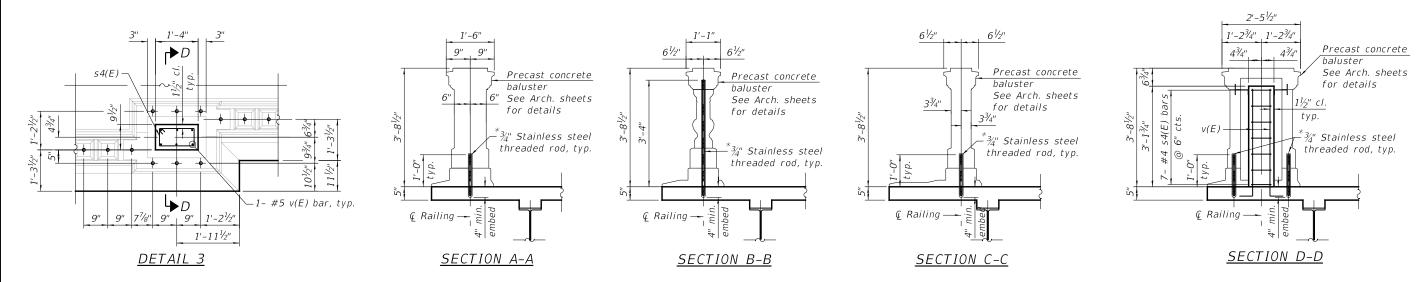
SOUTH RAILING PARTIAL LAYOUT PLAN

5⁷/8"

2'-6"

12 Spa. @ 9" = 9'-0"

42'-41/2"



2'-6"



► Edge of Deck

2'-6"

USER NAME = DESIGNED - SK REVISED
CHECKED - MI REVISED
PLOT SCALE = N.T.S. DRAWN - SK REVISED
PLOT DATE = 10/5/2020 CHECKED - MI REVISED
DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

20 Spa. @ 9" = 15'-0"

20'-101/2"

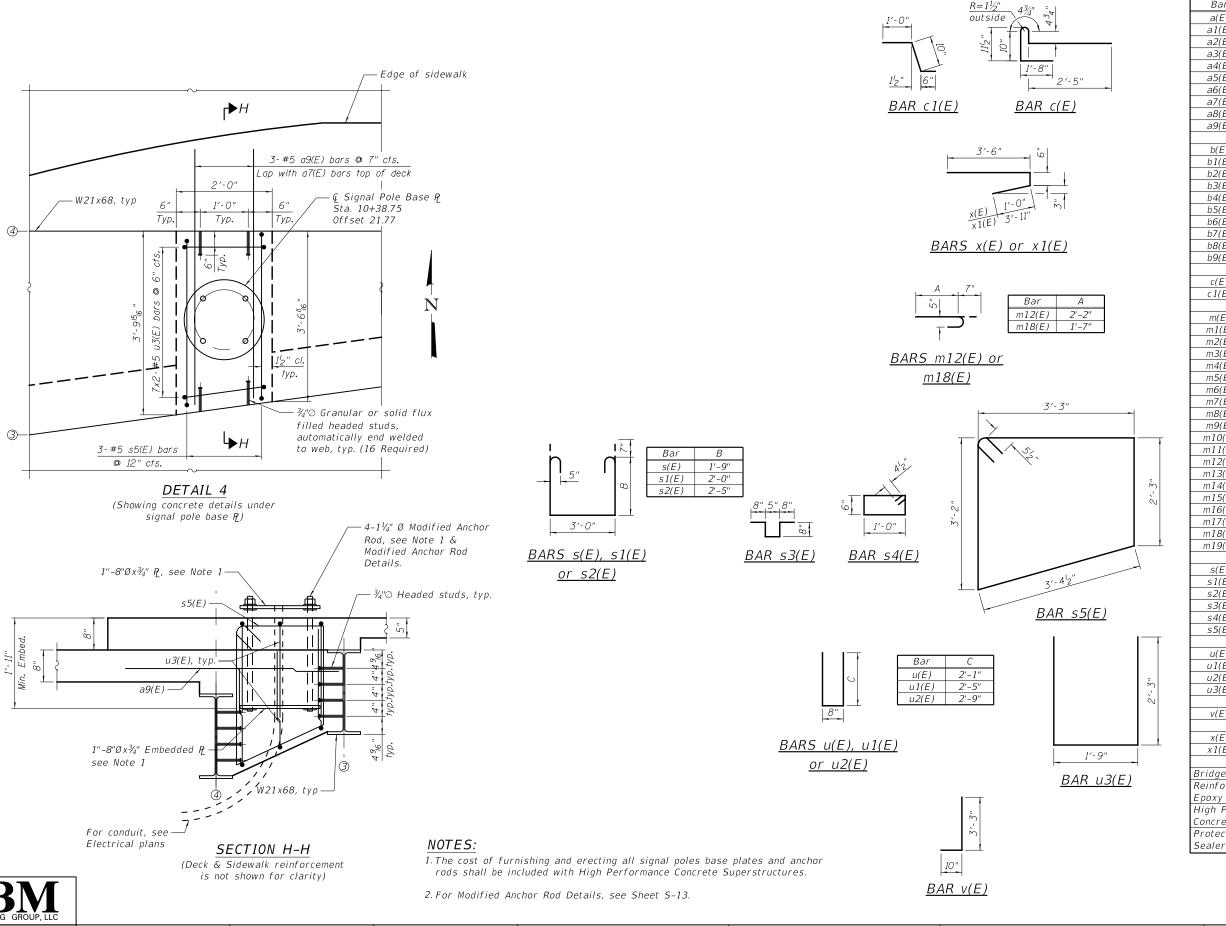
PARAPET ELEVATIONS AND DETAILS WEST FIXED SPANS (STRUCTURE NO. 016-6057)

51/2"

2'-6"

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-17
CDOT	PROJECT NO. E-1-525		60 of 210

Included with the cost of "High Performance Concrete Superstructure".



BILL OF MATERIAL

Dar	Ma	Cino	Longth	Chana
Bar	No.	Size	Length	Shape
a(E)	20	#5	10'-1"	
a1(E)	11	#5	11'-2"	
a2(E)	25	#5	10'-3"	
a3(E)		#5	13'-8"	
	22			
a4(E)	22	#5	11'-2"	
a5(E)	22	#5	15'-2"	
a6(E)	104	#5	35'-8"	
a7(E)	63	#5	45'-6"	
a8(E)	12	#5	1'-8"	
a9(E)	3	#5	7'-0"	
45(2)		,, 5	, ,	
b(E)	36	#5	30'-8"	
	8	#5		
b1(E)			33'-3"	
b2(E)	1	#5	41'-3"	
b3(E)	2	#5	9'-6"	
b4(E)	15	#5	20'-11"	
b5(E)	148	#5	29'-11"	
b6(E)	2	#5	35'-0"	
b7(E)	18	#5	21'-1"	
				_
b8(E)	1	#5	42'-0"	
b9(E)	32	#5	25'-0"	
/				l
c(E)	122	#4	5'-10"	<u> </u>
			2'-4"	_ =
c1(E)	44	#5	2'-4"	L L
m/E1	28	#5	3'-0"	
m(E)				
m1(E)	2	#5	4'-6"	
m2(E)	2	#6	8'-11"	
. ,				
m3(E)	2	#6	9'-4"	
m4(E)	2	#6	11'-5"	
m5(E)	12	#5	3'-11"	
m6(E)	2	#5	3'-0"	
m7(E)	2	#5	5'-0"	
m8(E)	6	#6	42'-10"	
m9(E)	24	#5	3'-8"	
m10(E)	8	#5	4'-6"	
m11(E)	4	#5	5'-0"	
m12(E)	8	#5	2'-9"	
				_
m13(E)	4	#5	1'-2"	
m14(E)	2	#6	10'-7"	
		#6	13'-3"	
m15(E)	2			
m16(E)	2	#6	11'-9"	
m17(E)	2	#5	3'-10"	
. ,				
m18(E)	4	#5	2'-2"	
m19(E)	2	#5	2'-4"	
/	_	" "	- ' -	I
s(E)	24	#5	7'-8"	[1
s1(E)	42	#5	8'-2"	П
s2(E)	6	#5	9'-0"	
s3(E)	416	#5	3'-1"	귝
s4(E)	7	#4	3'-9"	
s5(E)	3	#5	13'-0"	
JJ(L)		", "	 	
u(E)	24	#5	4'-10"	
			5'-6"	
u1(E)	42	#5		
u2(E)	6	#5	6'-2"	
u3(E)	14	#5	6'-3"	
uJ(E)	14	#)	0-5	
v(E)	6	#5	4'-1"	
* (L /		" >	r 1	
x(E)	37	#5	5'-0"	
			7'-11"	
x1(E)	37	#5	/ -11"	7
Bridge Dec	k Groot	/ina	Sq. Yd.	225
Reinforcem		5,	Pound	20,610
Epoxy Coat				
Link Dir			C. V.I	100 5
High Perfo			Cu. Yd.	100.5
Concrete S	uperstr	ucture		
Protective			Sq. Yd.	464
	CONCLE	C	Jy. 14.	1404
Sealer				i .

MINIMUM BAR LAP

#5 bar = 3'-6"

CITY OF CHICAGO

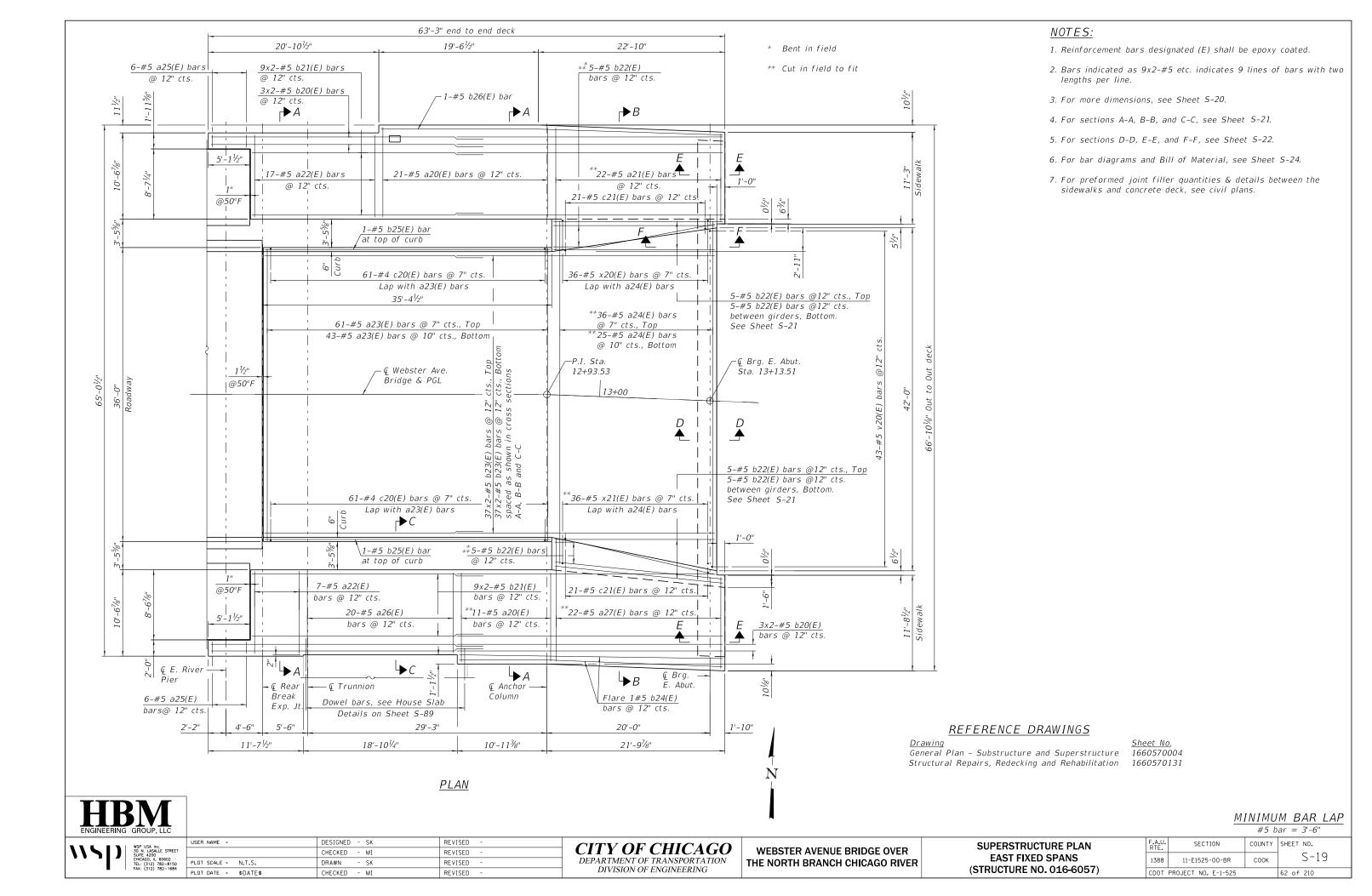
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

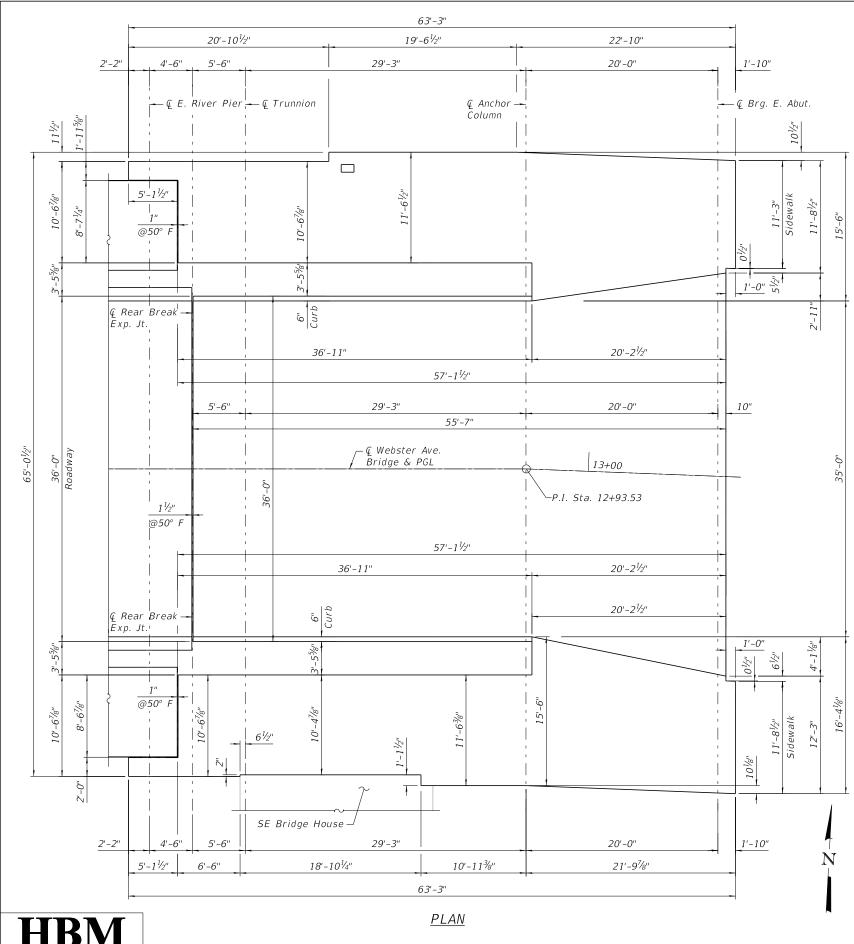
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

SUPERSTRUCTURE DETAILS WEST FIXED SPANS (STRUCTURE NO. 016-6057) F.A.U. SECTION COUNTY SHEET NO.

1388 11-E1525-00-BR COOK S-18

CDOT PROJECT NO. E-1-525 61 of 210

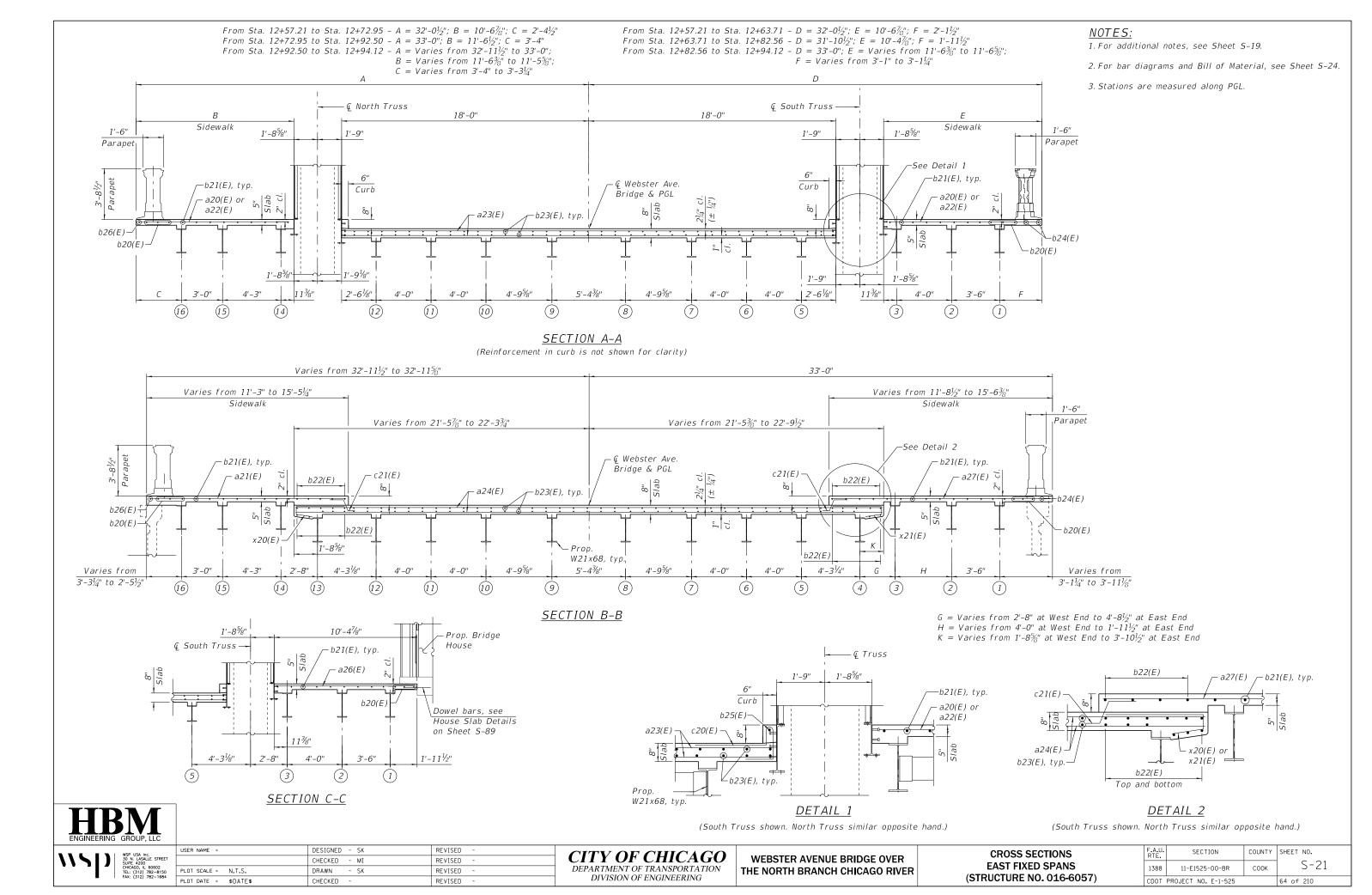


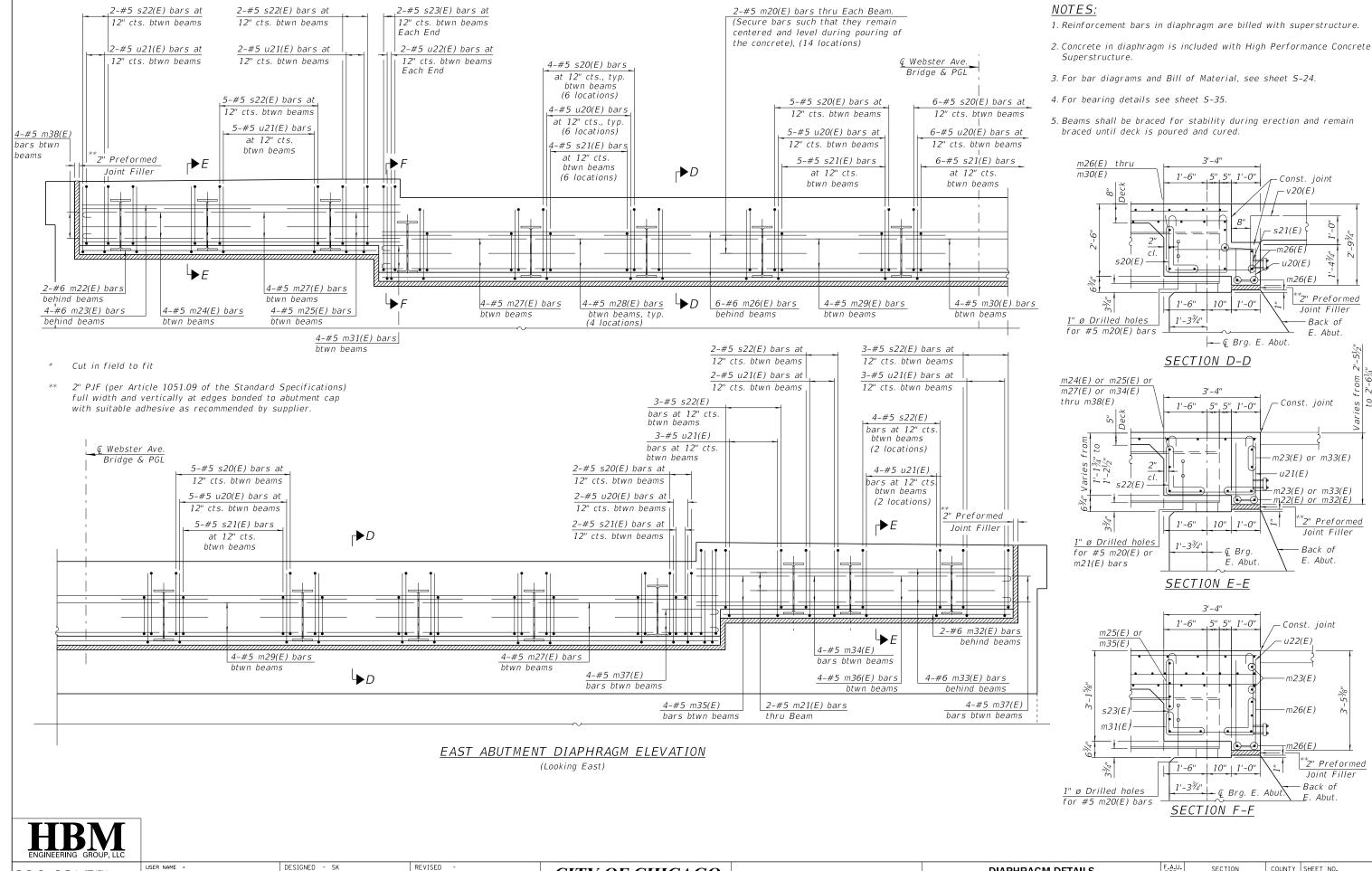


	USER NAME =	DESIGNED - SK	REVISED -
ET		CHECKED - MI	REVISED -
0	PLOT SCALE = N.T.S.	DRAWN - SK	REVISED -
4	PLOT DATE = \$DATE\$	CHECKED - MI	REVISED -

SUPERSTRUCTURE PLAN GEOMETRICS
EAST FIXED SPANS
(STRUCTURE NO. 016-6057)

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-20
CDOT	PROJECT NO. E-1-525		63 of 210



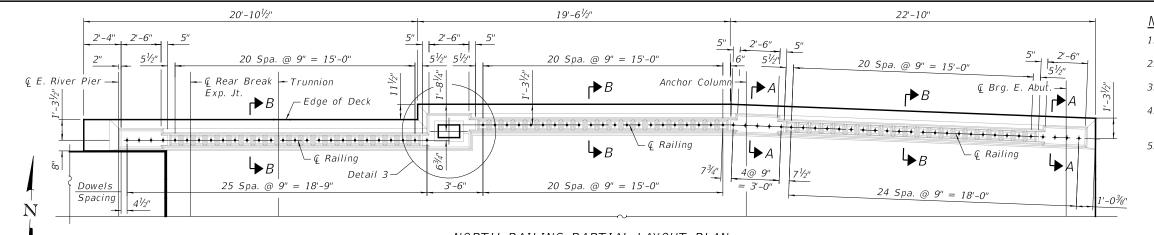


WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684

CITY OF CHICAGO

DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

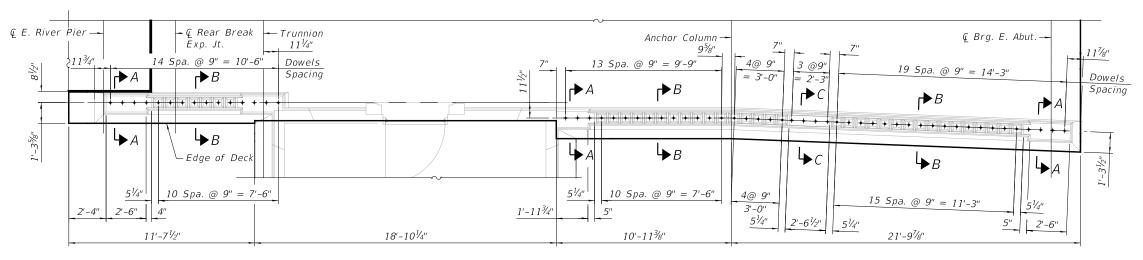
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER DIAPHRAGM DETAILS EAST FIXED SPANS (STRUCTURE NO. 016-6057)



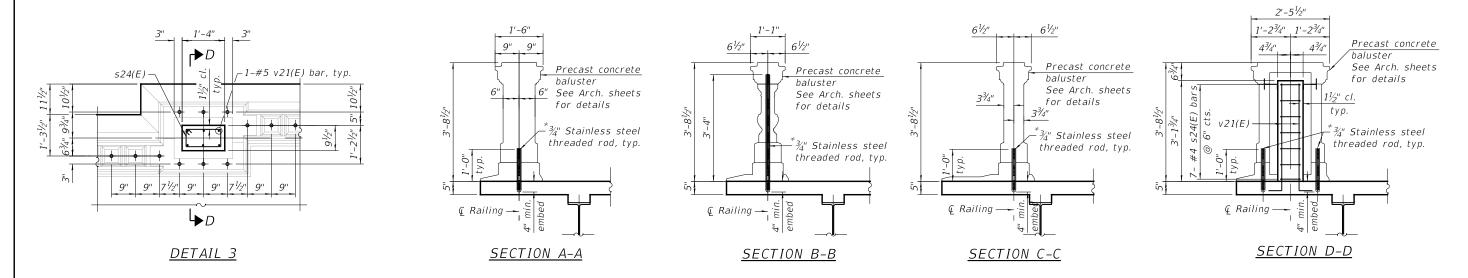
NOTES:

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. See sheet S-21 for deck cross sections.
- 3. For bar diagrams and Bill of material, see sheet S-24
- 4. See architectural (A-series) sheets for precast railing details.
- 5. The Contractor must use approved single straight coil loop inserts when pendant mounting threaded rods to a sidewalk. The single straight loop inserts must be cast into the concrete sidewalk.

NORTH RAILING PARTIAL LAYOUT PLAN



SOUTH RAILING PARTIAL LAYOUT PLAN

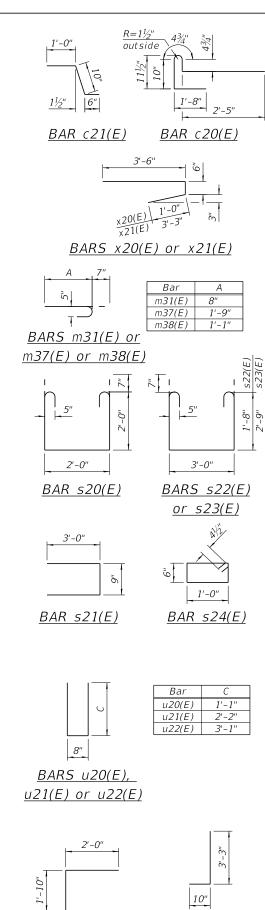


^{*} Included with the cost of "High Performance Concrete Superstructure".



VSP USA Inc. NO N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 EL: (312) 782-8150 CAX: (312) 782-1684	USER NAME =	DESIGNED - SK	REVISED -
		CHECKED - MI	REVISED -
	PLOT SCALE = N.T.S.	DRAWN - SK	REVISED -
	PLOT DATE = 10/5/2020	CHECKED - MI	REVISED -

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	COOK	S-23
CDOT	PROJECT NO. E-1-525		66 of 210



BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a20(E)	32	#5	11'-2"	
a21(E)	22	#5	15'-2"	
a22(E)	24	#5	10'-3"	
a23(E)	104	#5	35'-8"	
	61	#5	44'-8"	
a24(E)				
a25(E)	12	#5	1'-8"	
a26(E)	20	#5	10'-1"	
a27(E)	22	#5	15'-2"	
b20(E)	12	#5	33'-3"	
b21(E)	36	#5	30'-8"	
b22(E)	30	#5	21'-0"	
b23(E)	148	#5	29'-4"	
b24(E)	2	#5	32'-5"	
b25(E)	2	#5	35'-0"	
b26(E)	1	#5	42'-1"	
DZO(L)	1	# 5	72 1	
c20(E)	122	41	5'-10"	•
c20(E)	122	#4		
c21(E)	42	#5	2'-4"	
m20(E)	28	#5	3'-0"	
m21(E)	2	#5	5'-0"	
m22(E)	2	#6	10'-0"	
m23(E)	4	#6	10'-9"	
m24(E)	4	#5	2'-8"	
m25(E)	4	#5	2'-2"	
m26(E)	6	#6	42'-4"	
m27(E)	12	#5	3'-11"	
m28(E)	16	#5	3'-8"	
			4'-6"	
m29(E)	8	#5		
m30(E)	4	#5	5'-0"	
m31(E)	4	#5	1'-3"	
m32(E)	2	#6	9'-9"	
m33(E)	4	#5	10'-7"	
m34(E)	4	#5	1'-8"	
m35(E)	4	#5	3'-0"	
m36(E)	4	#5	3'-2"	
m37(E)	8	#5	2'-4"	
m38(E)	4	#5	1'-8"	
	,			
s20(E)	42	#5	7'-2"	Γſ
521(E)	42	#5	6'-9"	
			7'-6"	
522(E)	25	#5		
s23(E)	4	#5	9'-8"	
524(E)	7	#4	3'-9"	
u20(E)	42	#5	2'-10"	U
u21(E)	25	#5	5'-0"	U
u22(E)	4	#5	6'-10"	U
v20(E)	43	#5	3'-10"	
v21(E)	6	#5	4'-1"	
x20(E)	36	#5	5'-0"	
x21(E)	36	#5	7'-3"	
~~ 1(L/	50	"," >	, ,	
Bridge Dec	k Croo	ina	Ca Vd	222
			Sq. Yd.	223
Reinforcem		5,	Pound	18,500
Epoxy Coat			0	05.6
High Perfo			Cu. Yd.	95.6
Concrete S				
Protective	Concret	te .	Sq. Yd.	457
Sealer				
				1





USER NAME =	DESIGNED - SK	REVISED -
	CHECKED - MI	REVISED -
PLOT SCALE = N.T.S.	DRAWN - SK	REVISED -
PLOT DATE = \$DATE\$	CHECKED - MI	REVISED -

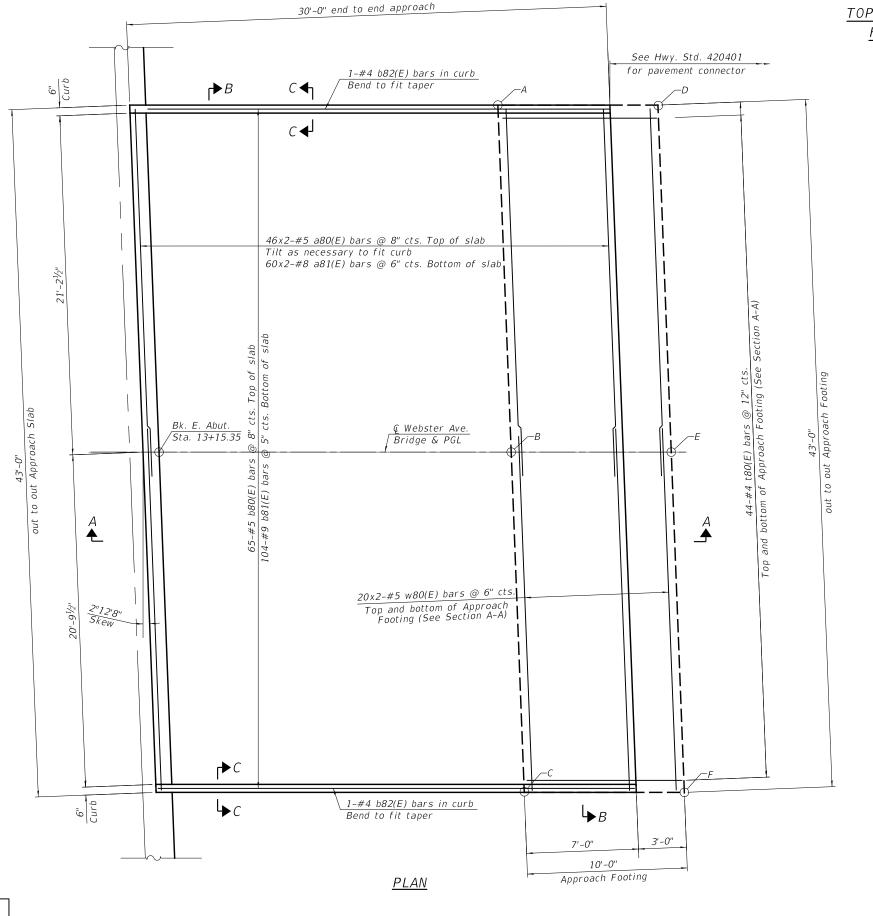
SUPERSTRUCTURE DETAILS
EAST FIXED SPANS
(STRUCTURE NO. 016-6057)

BAR v21(E)

BAR v20(E)

		# 3 0	ai – 3-0
F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-24
CDOT	PROJECT NO. E-1-525		67 of 210

MINIMUM BAR LAP



TOP AND BOTTOM ELEVATIONS

FOR APPROACH FOOTING

	Approach		
Point	Тор	Bottom	
Α	18.48	17.65	l
В	18.78	17.95	
С	18.44	17.61	
D	18.07	17.23	
Ε	18.48	17.64	
F	18.03	17.19	l

NOTES:

- 1. For Sections A-A, B-B, and C-C, see Sheet S-26.
- 2. a80(E) and a81(E) bar spacings measured along © Rdwy.
- 3. Bars indicated thus 46x2 -#5 indicates 46 lines of bars with 2 length per line.

REFERENCE DRAWINGS

<u>Drawing</u> East Approach - Retaining Wall Details and Reinforcement Bars 1660570051 General Plan of Approaches 1660570052 Paving on Approaches

Sheet No. 1660570060



JSER NAME =	DESIGNED - JMI	REVISED -
	CHECKED - MI	REVISED -
PLOT SCALE = N.T.S.	DRAWN - JMI	REVISED -
PLOT DATE = 10/5/2020	CHECKED - MI	REVISED -

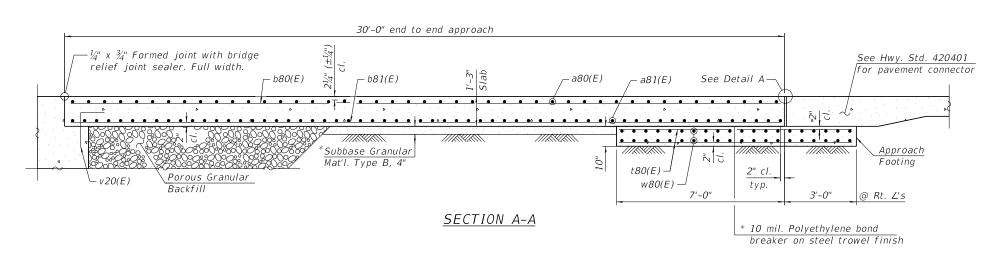
CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

EAST APPROACH SLAB (STRUCTURE NO. 016-6057)

N

COUNTY SHEET NO. SECTION S-25 11-E1525-00-BR соок CDOT PROJECT NO. E-1-525 68 of 210



NOTES:

- 1. The joint opening shall be adjusted for temperature per Article 520.04 of the Standard Specifications. However, since this detail is for jointless structures, the length of bridge used to calculate the adjustment shall be equal to half the total bridge length plus the length of the bridge approach slab.
- 2. Approach slab shall be paid for as High Performance Concrete Superstructures.
- 3. Approach footing concrete shall be paid for as High Performance Concrete
- 4. Reinforcement shall be paid for as Reinforcement Bars, Epoxy Coated.
- 5. For v20(E) bar details, see Sheet S-24.
- 6. The approach footing maximum applied service bearing pressure (Qmax) = 2.0 ksf.
- 7. Cost of excavation for approach footing included with High Performance Concrete Structures.
- 8. For Porous Granular Backfill and drainage treatment details, see Sheet S-97.

Bar a80(E)

a81(E)

b80(E)

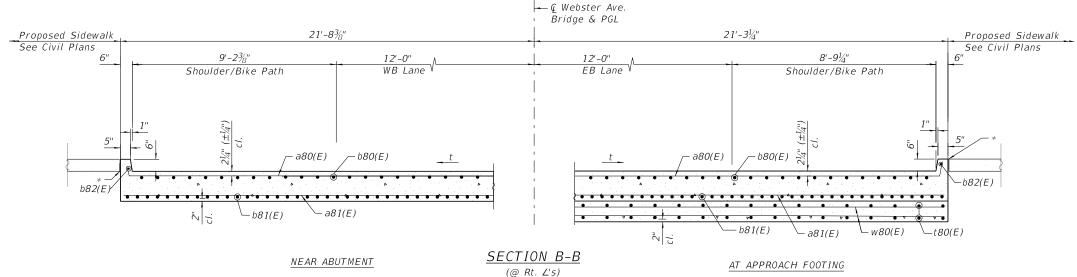
b81(E)

b82(E)

t80(E)

w80(E)

Sealer



*Preformed Expansion Joint Filler according to Article 1051.09 of the Standard Specifications; full depth of sidewalk, full length of Approach Slab.

^TVaries from 0% at East Abutment to 2% at East end of Approach Slab

Proposed Sidewalk See Civil Plans



BILL OF MATERIAL

#8

#9

#4

#5

Size Length Shape

29'-8"

9'-8"

23'-1"

Sq. Yd.

Pound

Cu. Yd.

Cu. Yd.

Sq. Yd.

140

24,930

13.3

60.2

149

#5 23'-9"

#5 29'-8"

#4 29'-8"

No.

92

120

65

104

88

80

Bridge Deck Grooving

Reinforcement Bars,

Concrete Structures High Performance

Protective Concrete

Concrete Superstructures

High Performance

Epoxy Coated

East Approach - Retaining Wall Details and Reinforcement Bars

REFERENCE DRAWINGS

Sheet No. 1660570051 1660570052 1660570060

(Detail A shown, applies to Highway Standard 420401 only. Detail A for pavement connector (HMA) may be found on Highway Standard 420406.)

50° F

— € Joint

**Expansion joint. See Special Provision "Preformed Pavement

Pavement Connector

(PCC)

Joint Seal". Recess 1/4" minimum. Run out to out of curb

DETAIL A

(@ Rt. L's)

** Cost included with High Performance Concrete Superstructures

2¾" at 50° F

See Notes.

End of Appr. slab

MINIMUM BAR LAP

 $#4 \ bar = 2'-5''$ $#5 \ bar = 3'-6''$

 $#8 \ bar = 4'-9''$



USER NAME =	DESIGNED - JMI	REVISED -
	CHECKED - MI	REVISED -
PLOT SCALE = N.T.S.	DRAWN - JMI	REVISED -
PLOT DATE = \$DATE\$	CHECKED - MI	REVISED -

Paving on Approaches

- b82(E)

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-26
CDOT	PROJECT NO. E-1-525		69 of 210

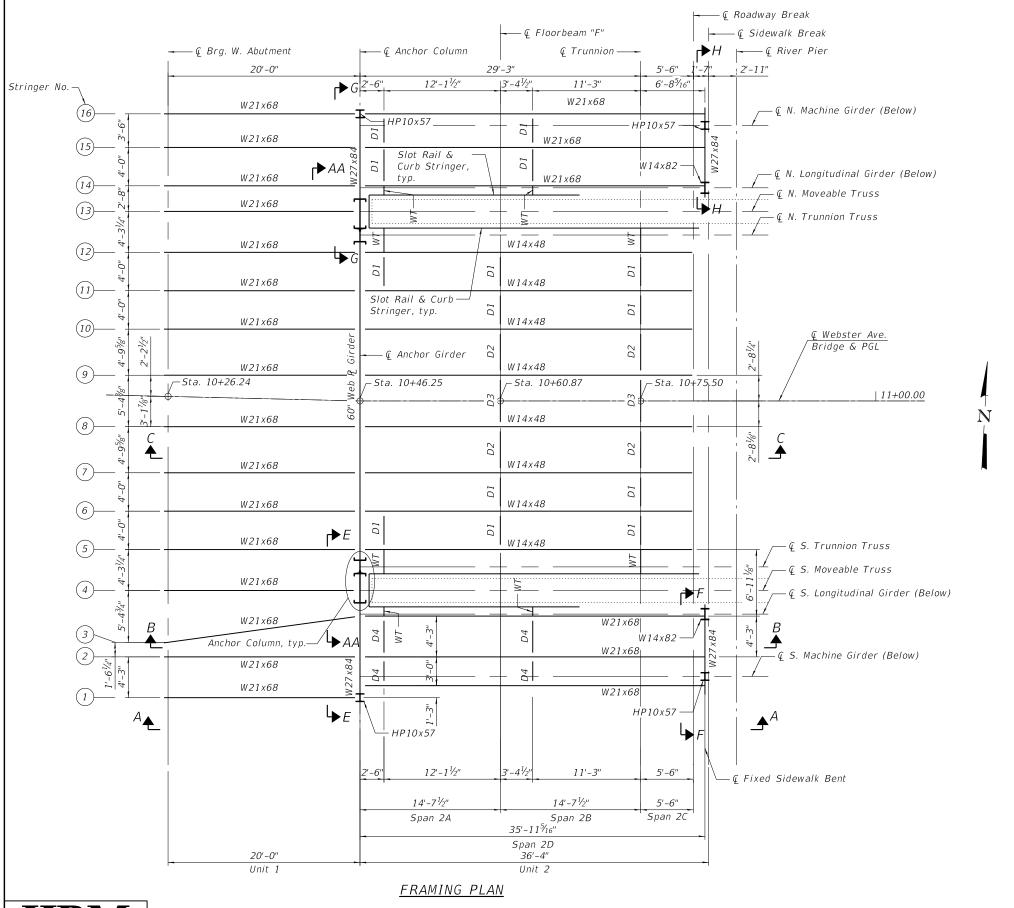
SECTION C-C

General Plan of Approaches

a81(E) - b81(E)

a80(E) \(\frac{b80(E)}{\tau} \)

^{***} Per manufacturer recommendations



REFERENCE DRAWINGS

<u>Drawing</u> Sheet No. Fixed Part - General Plan 1660570016 1660570110 Stress Diagrams Structural Repairs, Redecking and Rehabilitation 1660570131 Structural Repairs - Fixed Part 1660570134 General Diagram of Fixed Part 1660570165

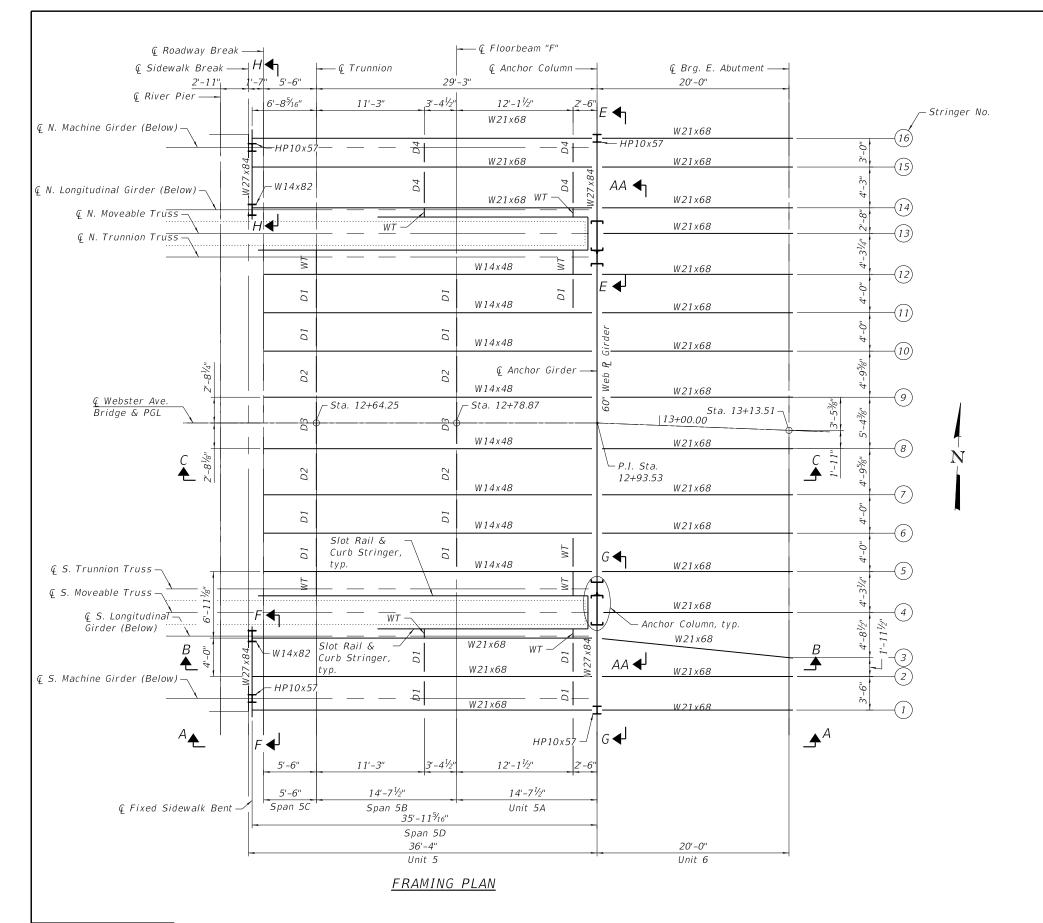
NOTES:

- 1. See Sheet S-03 for general structural notes.
- 2. For Sections A-A thru C-C see Sheets S-30.
- 3. For Sections E-E thru H-H see Sheet S-32.
- 4. For Slot Rail & Curb Stringer detailing, see Sheet S-33.
- 5. For Section AA-AA see Sheet S-34.
- 6. For Interior Diaphragm Details see Sheet S-32.



USER NAME =	DESIGNED - MA	REVISED -
	CHECKED - WM	REVISED -
PLOT SCALE = N.T.S.	DRAWN - MA	REVISED -
PLOT DATE = 10/5/2020	CHECKED - WM	REVISED -

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-27
CDOT	PROJECT NO. E-1-525		70 of 210



REFERENCE DRAWINGS

DrawingSheet No.Fixed Part - General Plan1660570016Stress Diagrams1660570110Structural Repairs, Redecking and Rehabilitation1660570131Structural Repairs - Fixed Part1660570134General Diagram of Fixed Part1660570165

NOTES:

- 1. See Sheet S-03 for general structural notes.
- 2. For Sections A-A thru C-C, see Sheets S-30.
- 3. For Section E-E thru H-H, see Sheet S-32.
- 4. For Slot Rail & Curb Stringer detailing, see Sheet S-33.
- 5. For Section AA-AA, see Sheet S-34.
- 6. For Interior Diaphragm Details, see Sheet S-32.

HBM ENGINEERING GROUP, LLC

WSP USA Inc.	USER NAME =	DESIGNED - MA	REVISED -
30 N. LASALLE STREET SUITE 4200		CHECKED - WM	REVISED -
CHICAGO, IL 60602 TEL: (312) 782-8150	PLOT SCALE = N.T.S.	DRAWN - MA	REVISED -
FAX: (312) 782-1684	PLOT DATE = 10/5/2020	CHECKED - WM	REVISED -



F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	COOK	S-28
CDOT	PROJECT NO. E-1-525		71 of 210

UNIT 1 OR 5	STRING	FR MOMENT	TARIF
01411 1 011 9	5.711740		
		0.4 Span 1 or 0.4 Span 5	
		W21x68	W21x68
		(Roadway)	(Sidewalk)
Is	(in ⁴)	1,480	1,480
Ic(n)	(in ⁴)	7,689	5,551
Ic(3n)	(in ⁴)	5,402	3,659
Ic(cr)	(in ⁴)	-	-
Ss	(in³)	140	140
Sc(n)	(in³)	305	261
Sc(3n)	(in³)	270	224
Sc(cr)	(in³)	-	-
DC1	(k/')	0.573	0.522
MDC1	('k)	58	44
DC2	(k/')	0.000	0.000
MDC2	('k)	0	0
DW	(k/')	0.220	0.000
MDW	('k)	11	0
LLDF		ı	ı
MŁ + IM	('k)	146	21 *
Mu (Strength I)	('k)	345	92
Øf Mn	('k)	1,132	729
fs DC1	(ksi)	4.96	3.76
fs DC2	(ksi)	0.00	0.00
fs DW	(ksi)	0.49	0.00
fs (½+IM)	(ksi)	5.75	0.98
fs (Service II)	(ksi)		5.03
0.95Rh Fyf	(ksi)		47.50
fs (Total)(Strength		17.00	6.41
Øf Fn	(ksi)	-	-
Vf	(k)	27.46	-

UNIT 1 OR 5 STRINGER REACTION TABLE				
	East or We	East or West Abut.		
	W21x68	W21x68		
	(Roadway)	(Sidewalk)		
LLDF	-	_		
0CF	-	-		
RDC1 (k	12.1	8.12		
RDC2 (k) 0	0		
RDW (k	3.3	0		
RŁ (k	54.65	4.25 *		
Rim (k	18.04	0		
RTotal (k	88.09	12.37		

UNIT 2 OR 4 STRINGER MOMENT TABLE				
		0.4 Span 2A or	Floorbeam	0.4 Span
		0.6 Span 2B	"F"	2D
		W14x48	W14x48	W21x68
		(Roadway)	(Roadway)	(Sidewalk)
Is	(in⁴)	484	484	1,480
Ic(n)	(in⁴)	3,591	_	7,689
Ic(3n)	(in⁴)	2,575	_	5,402
Ic(cr)	(in⁴)	-	1,322	-
Ss	(in³)	70	70	140
Sc(n)	(in³)	184	-	305
Sc(3n)	(in³)	163	-	270
Sc(cr)	(in³)	-	403	-
DC1	(k/')	0.616	0.616	0.390
MDC1	('k)	8	15	59
DC2	(k/')	0.000	0.000	0.000
MDC2	('k)	0	0	0
DW	(k/')	0.254	0.254	0.000
MDW	('k)	3	6	0
LLDF		-	-	-
M& + IM	('k)	82	79	57 *
Mu (Strength I)	('k)	158	166	174
Øf Mn	('k)	702	464	583
fs DC1	(ksi)	1.37	2.56	5.05
fs DC2	(ksi)	0.00	0.00	0.00
fs DW	(ksi)	0.22	0.18	0.00
fs (4+IM)	(ksi)	5.35	2.35	2.25
fs (Service II)	(ksi)	8.55	5.80	7.97
0.95Rh Fyf	(ksi)	47.50	47.50	47.50
fs (Total)(Strength	I)(ksi)	11.41	7.59	10.25
Øf Fn	(ksi)	-	-	-
Vf	(k)	27.58	27.27	-

UNIT 2 OR 4 STRINGER REACTION TABLE				
		Anchor Col. FB		Floorbeam "F"
		W14x48 W21x68		W14x48
		(Roadway)	(Sidewalk)	(Roadway)
LLDF		_	-	-
0CF		-	_	_
RDC1	(k)	4.45	10.72	12.47
RDC2	(k)	0	0	0
RDW	(k)	1.81	0	6.03
R Ł	(k)	51.15	6.525*	59.8
RIM	(k)	16.88	0	19.74
RTotal	(k)	74.29	17.245	98.04

- Is, Ss: Non-composite moment of inertia and section modulus of the steel section used for computing fs(Total-Strength I, and Service II) due to non-composite dead loads (in.4 and in.3).
- Ic(n), Sc(n): Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing fs(Total-Strength I, and Service II) in uncracked sections due to short-term composite live loads (in.4 and in.3).
- Ic(3n), Sc(3n): Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing fs(Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in.4 and in.3).
- Ic(cr), Sc(cr): Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing fs (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in.4 and in.3).
 - DC1:Un-factored non-composite dead load (kips/ft.).
 - MDC1: Un-factored moment due to non-composite dead load (kip-ft.).
 - DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
 - MDC2: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
 - DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
 - MDW: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
 - $M_{\text{L}}+\text{IM}$: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).
- Mu (Strength I): Factored design moment (kip-ft.).
 - 1.25 (MDC1 + MDC2) + 1.5 MDW + 1.75 M± + IM
 - Øf Mn: Compact composite positive moment capacity computed according to Article 6.10.7.1 or non-slender negative moment capacity according to Article A6.1.1 or A6.1.2 (kip-ft).
 - fs DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi). MDC1/ Snc
 - fs DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated below (ksi).
 - MDC2/ Sc(3n) or MDC2/ Sc(cr) as applicable.
 - fs DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi).

 MDW/ Sc(3n) or MDW/ Sc(cr) as applicable.
 - fs (½+IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).
 - M4+IM / Sc(n) or M4+IM / Sc(cr) as applicable.
- fs (Service II): Sum of stresses as computed below (ksi). fsDC1 + fsDC2 + fsDW + 1.3 fs(+ + 1M)
 - 0.95RhFyf: Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).
- fs (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi).
 - 1.25 (fsDC1 + fsDC2) + 1.5 fsDW + 1.75 fs(+ + IM)
 - Øf Fn: Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7 or 6.10.8 (ksi).
 - Vf: Maximum factored shear range in span computed according to Article 6.10.10.

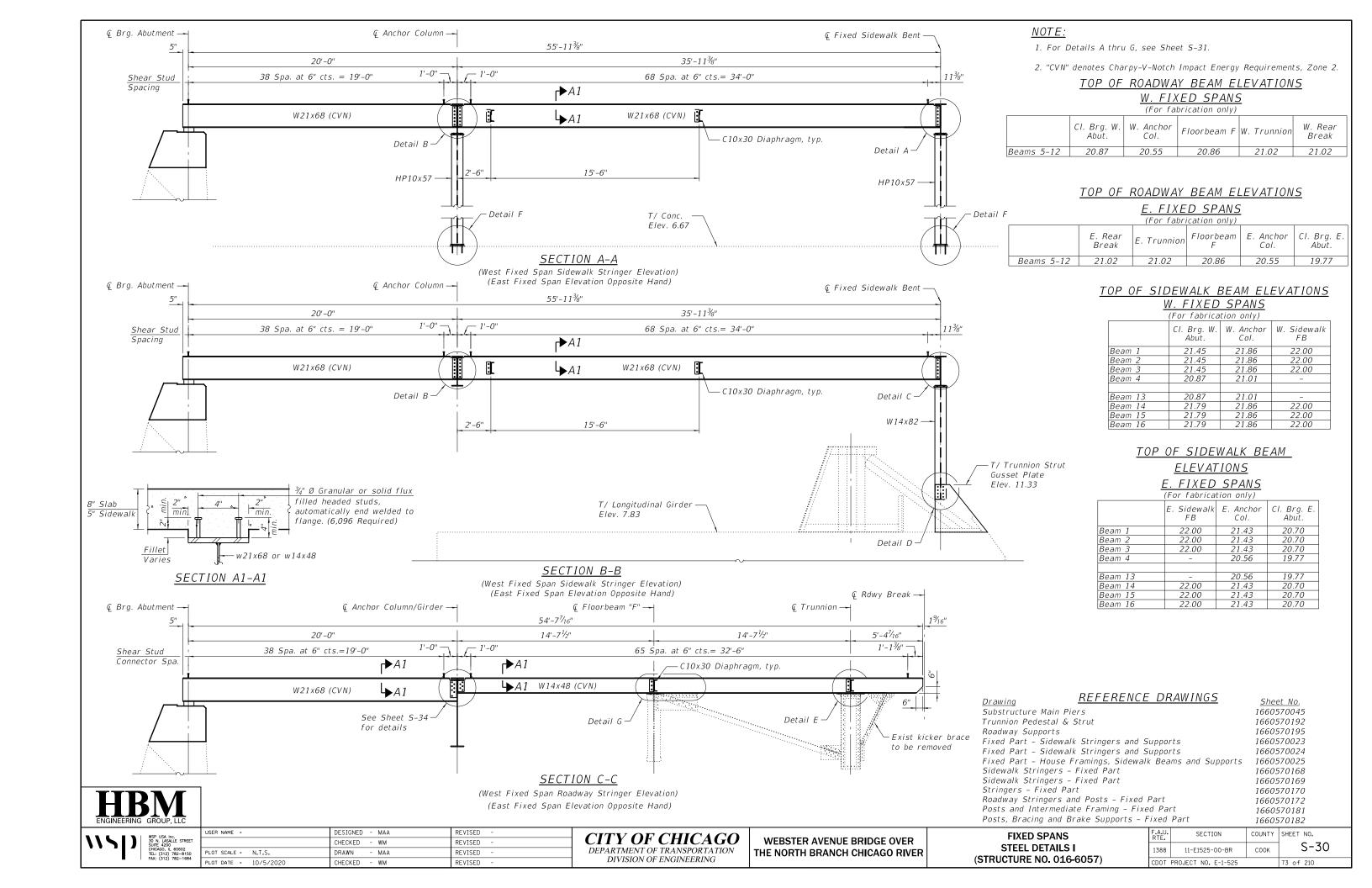
Note:

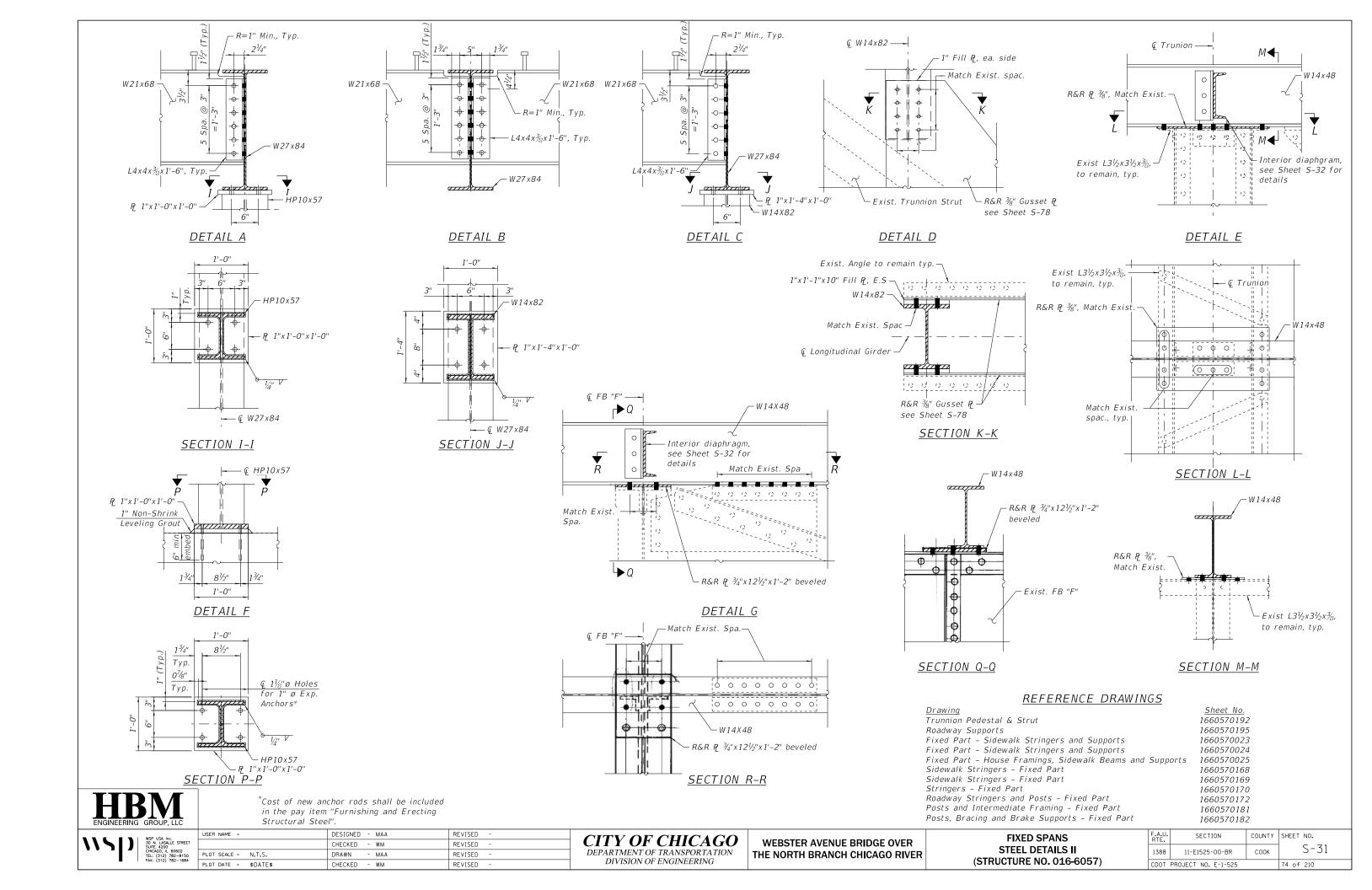
 $\mbox{\rm ML}$ and $\mbox{\rm RL}$ include the effects of centrifugal force and superelevation.

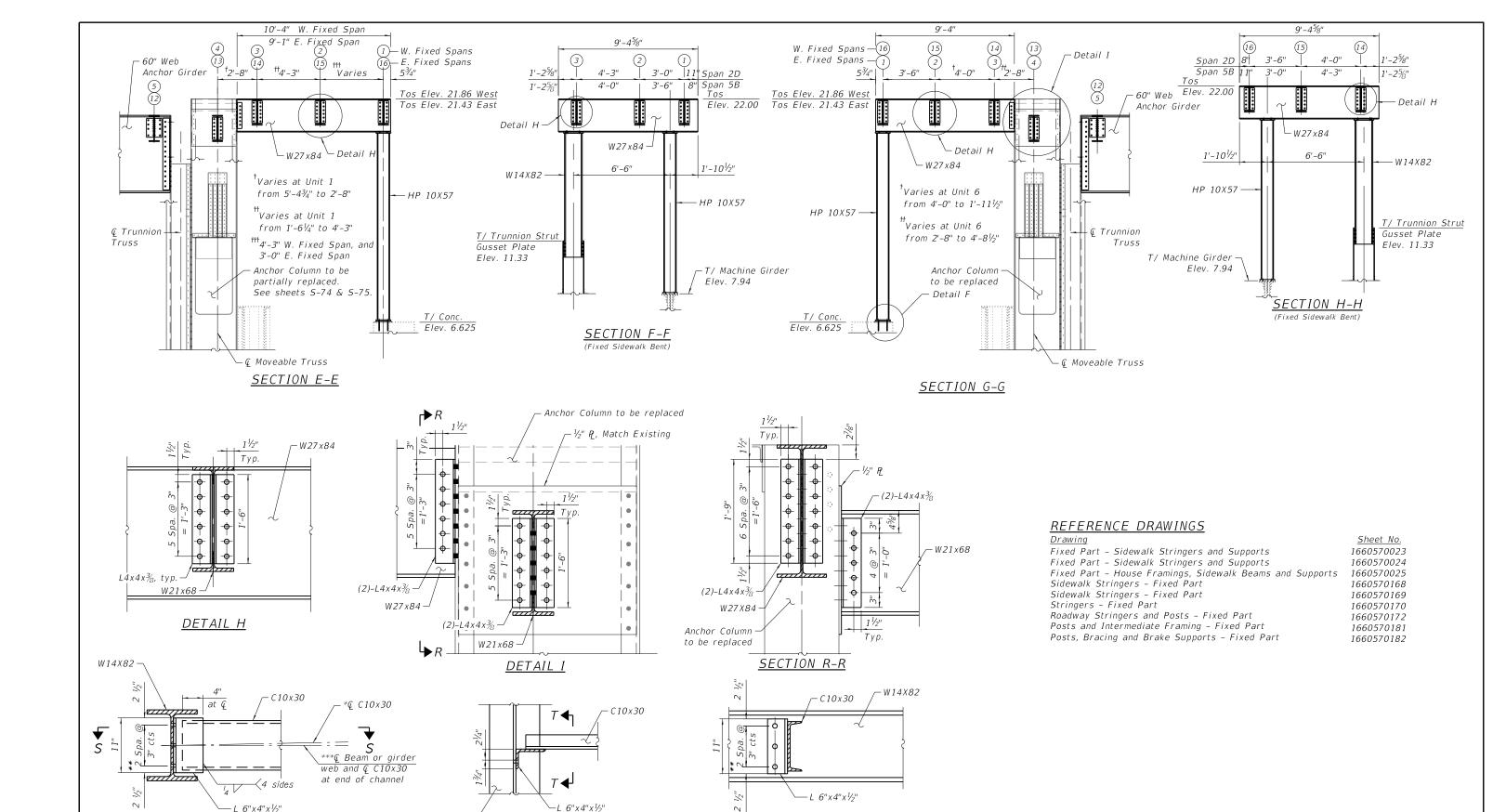
* The live load moment and reaction values for sidewalk stringers are based on uniform pedestrian live load of 100psf without impact.



USER NAME =	DESIGNED - MA	REVISED -
	CHECKED - MI	REVISED -
PLOT SCALE = N.T.S.	DRAWN - MA	REVISED -
PLOT DATE = \$DATE\$	CHECKED - WM	REVISED -







<u>INTERIOR DIAPHRAGM-D1, D2, D3</u> (48 Required) SECTION S-S

SECTION T-T

HBM ENGINEERING GROUP, LLC

** 3/4" \oslash HS bolts, $^{15}\!\!/_{16}$ " \oslash holes.

W14X82

*** Alternate channels are permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section. The alternate, if utilized, shall be provided at no additional cost to the Department.



 USER NAME =
 DESIGNED - MAA
 REVISED

 CHECKED - WM
 REVISED

 PLOT SCALE = N.T.S.
 DRAWN - MAA
 REVISED

 PLOT DATE = 10/5/2020
 CHECKED - WM
 REVISED

 DIVISION OF

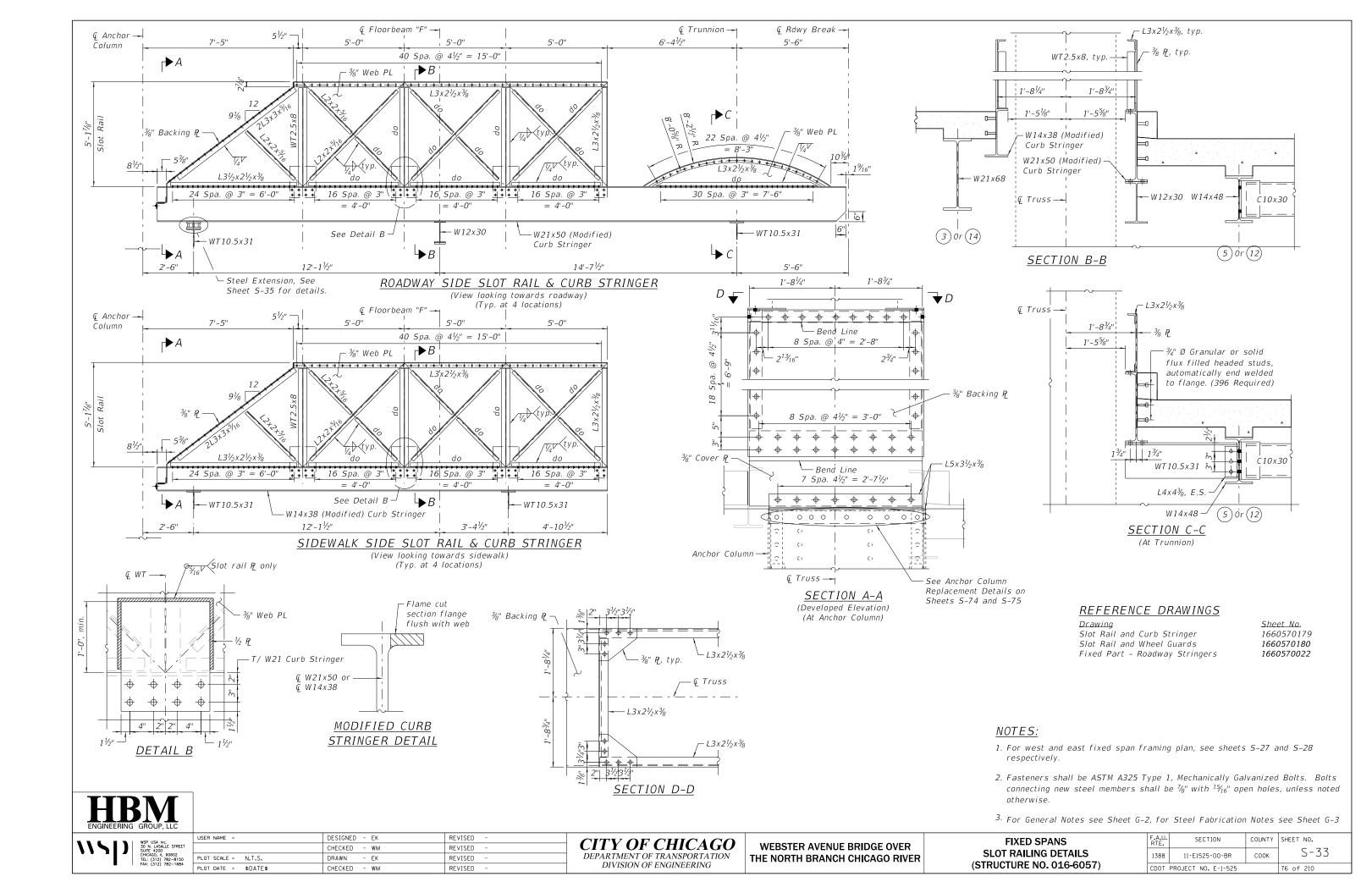
CITY OF CHICAGO

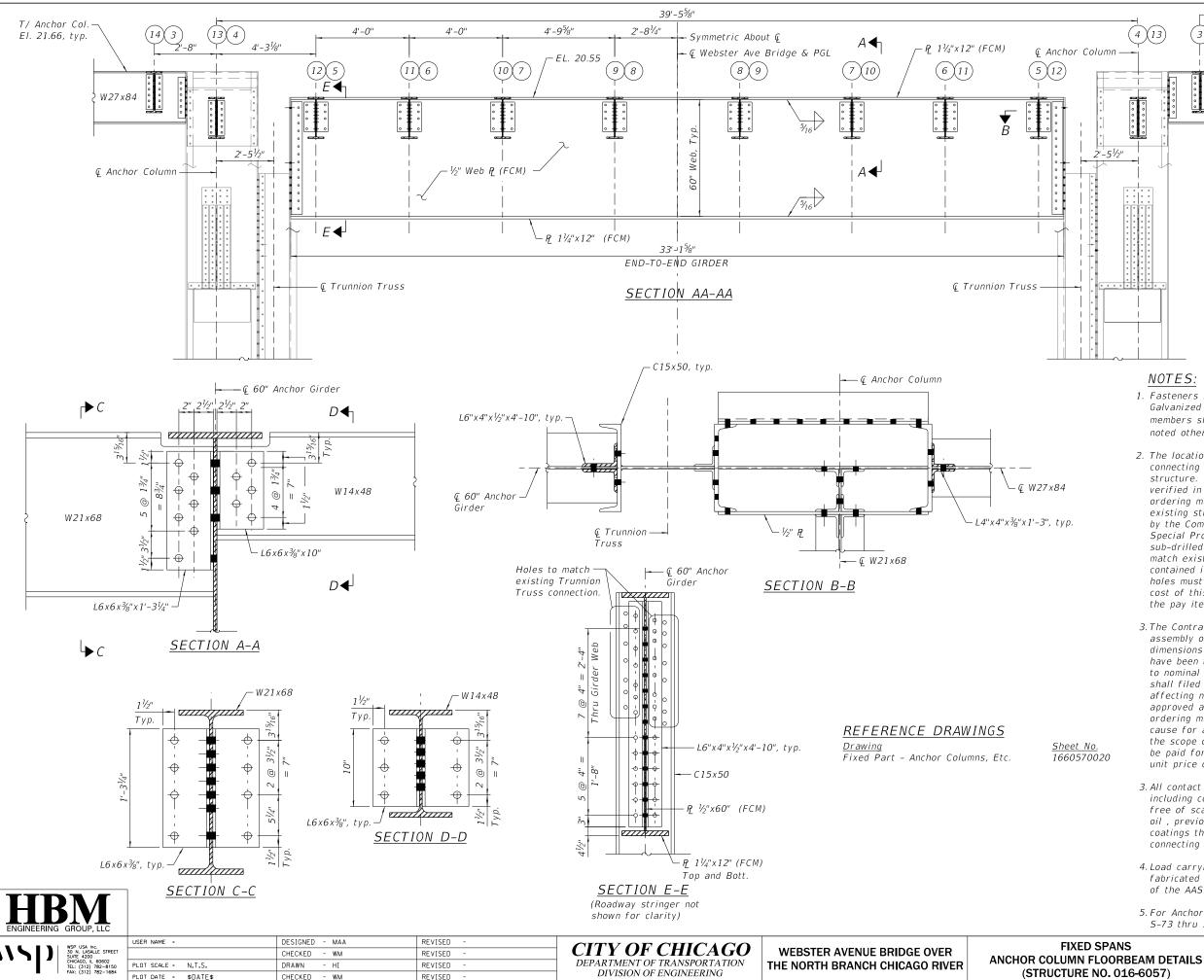
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER
THE NORTH BRANCH CHICAGO RIVER

FIXED SPANS STEEL DETAILS III (STRUCTURE NO. 016-6057)

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	COOK	S-32
CDOT	PROJECT NO. E-1-525	-	75 of 210





CHECKED - WM

REVISED

NOTES:

1. Fasteners shall be ASTM A325 Type 1, Mechanically Galvanized Bolts. Bolts connecting new steel members shall be $\frac{7}{8}$ " with $\frac{15}{16}$ " open holes, unless noted otherwise.

-West Fixed Spans

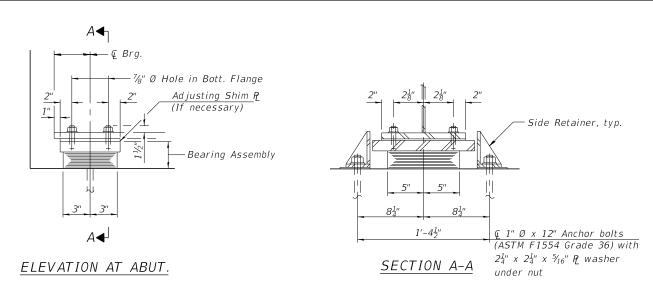
- East Fixed Spans

B

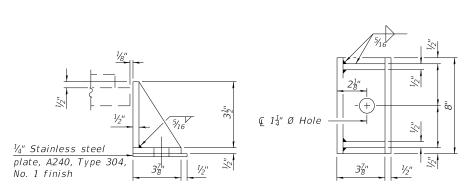
W27x84

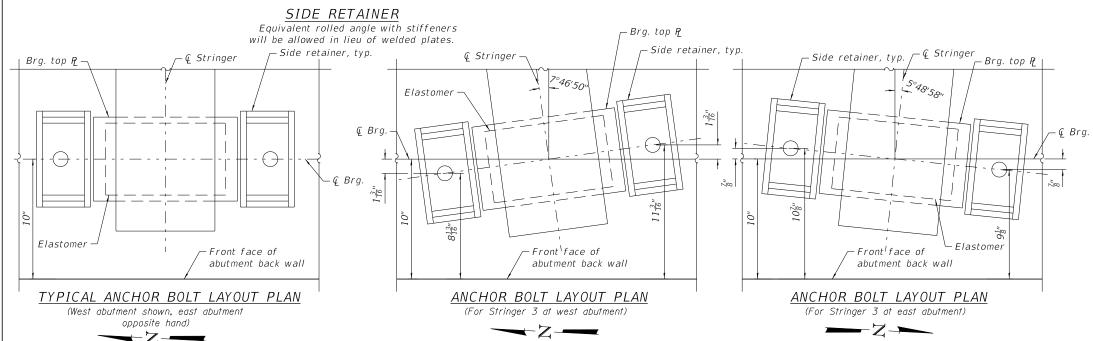
- 2. The location and diameter of the holes in new connecting material must match holes in the existing structure. Bolt and rivet spacings and size must be verified in the filed by the Contractor prior to ordering material for fabrication. Holes in the existing structure may be enlarged only as approved by the Commissioner and in accordance with the Special Provisions. Holes may be sub-punched or sub-drilled in the new material and field reamed to match existing holes provided the sub-hole is fully contained in the outline of the reamed hole. Final holes must be round and may not be oversized. The cost of this work shall be considered incidental to the pay item which it pertains.
- 3. The Contractor is responsible for proper fitting and assembly of all parts of the proposed work. Plan dimensions and details relative to existing structure have been taken from existing plans and are subject to nominal construction variations. The Contractor shall filed verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering material. Such variations shall not be cause for additional compensation for a change in the scope of the work. However, the Contractor will be paid for the quantity actually furnished at the unit price of the work.
- 3. All contact surfaces on the new and existing steel, including connection bolts, nuts and washers, are free of scale, burrs, dirt and other foreign material, oil , previously applied pain, lacquer or other coatings that would prevent solid seating of the connecting parts.
- 4. Load carrying components designated "FCM" shall be fabricated according to the provisions of Clause 12 of the AASHTO/AWS D1.5 Bridge Welding Code.
- 5. For Anchor Column Repairs and Details, see Sheets S-73 thru S-75.

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-34
CDOT	PROJECT NO. E-1-525		77 of 210



TYPE I ELASTOMERIC EXP. BRG.





Bonded 7" 3''' 9 Threaded Stud with flat washer & hex nut. (4-Regd.) P. $1\frac{1}{2}"$ \times 7" \times 12"Bonded $\frac{1}{2}$ $\frac{1}{2$

BEARING ASSEMBLY

Note:

Shim plates shall not be placed under Bearing Assembly.

NOTES:

- 1. Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.
- 2. Side retainers and other steel members required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type I.
- 3. Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.
- 4. For Slot Railing and Curb Stringer sections and details see Sheet S-33.
- 5. Prior to ordering any material, the Contractor shall field verify all bearing heights and shim thickness dimensions.

Item

Elastomeric Bearing

Assembly Type I

BILL OF MATERIAL

Unit

Each

Total

32

HBM ENGINEERING GROUP, LLC

wsp	WSP USA Inc. 30 N. LASALLE STREE SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684
-----	--

USER NAME =	DESIGNED - JJS	REVISED -
	CHECKED - MA	REVISED -
PLOT SCALE = N.T.S.	DRAWN - JJS	REVISED -
PLOT DATE = \$DATE\$	CHECKED - MA	REVISED -

CITY OF CHICAGO

DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER
THE NORTH BRANCH CHICAGO RIVER

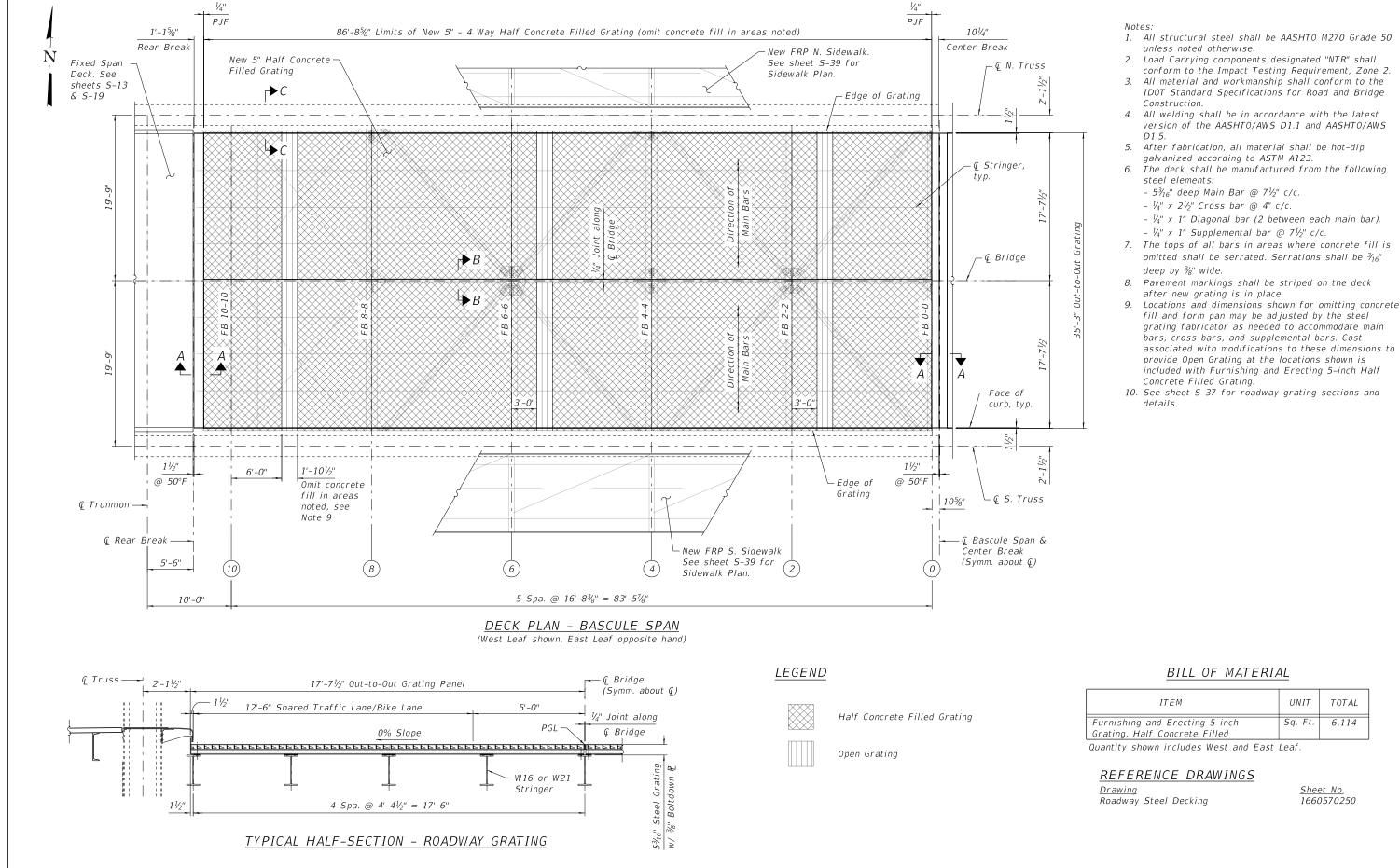
BEARING DETAILS
(STRUCTURE NO. 016-6057)

 Anchor Bolts, 1"
 Each
 64

 F.A.U. RTE.
 SECTION
 COUNTY
 SHEET NO.

 1388
 11-E1525-00-BR
 COOK
 S - 35

 CDOT PROJECT NO. E-1-525
 78 of 210



USER NAME = IJLOPEZ

PLOT SCALE = N.T.S.

PLOT DATE = \$DATF\$

DESIGNED - IJL

CHECKED - NBR

CHECKED - JIG

- IJL

REVISED

REVISED

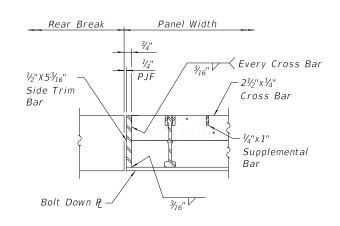
REVISED

REVISED

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-36
CDOT	PROJECT NO. E-1-525		79 of 210

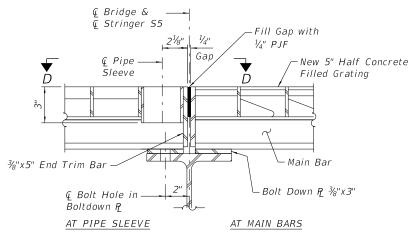
TOTAL

6,114



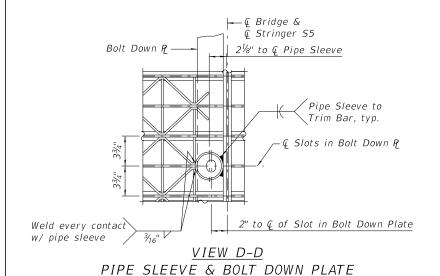
SECTION A-A END PANEL AT REAR BREAK

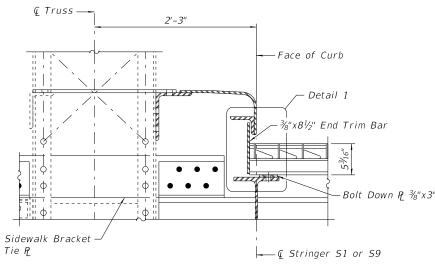
(Similar at Center Break)

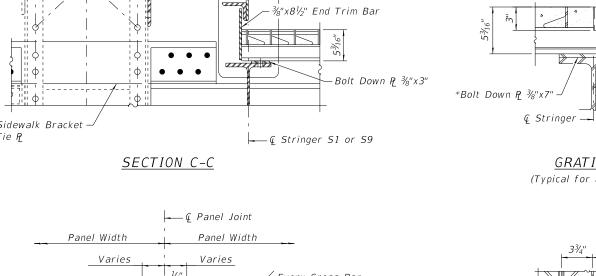


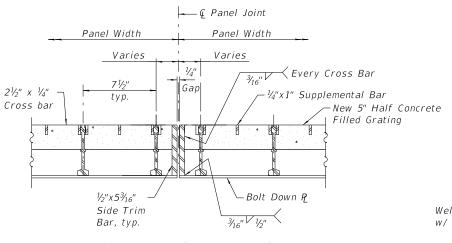
SECTION B-B

(Concrete fill not shown for clarity)

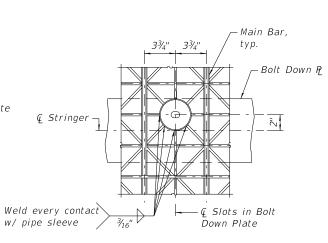








TYPICAL INTERIOR PANEL JOINT



* Bolt Down 12 3/8" x 3"

at Stringers S1, S5, and S9

Weld every contact \

w/ pipe sleeve

PLANPIPE SLEEVE & BOLT DOWN PLATE (Typical for all stringers unless noted otherwise)

hold-down bolts. min. thickness of sleeve 0.2" 20 Ga. Galv. Form Pans, typ. Set here for concrete filled panels. Tack weld on 8" ctrs. and staggered.

 $-3\frac{1}{2}$ " 0.D. Sch. 40 pipe sleeve

to permit installation of

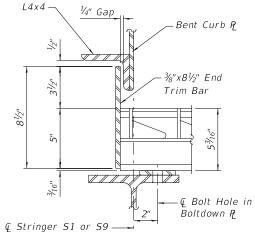
-3/8"x7" Bolt Down ₽

¾" ⊘ Galv. A325 bolts w/lock washer in std. holes in flange of stringer & $^{13}/_{16}$ " x $1^{7}/_{8}$ " transverse slots in the grating bolt down plate spaced at $\pm 1'-3''$ cts. on every stringer.

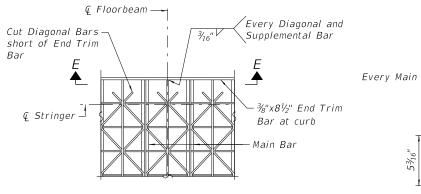
GRATING BOLT DOWN DETAIL

New stringer

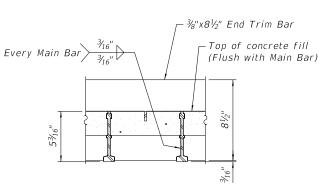
(Typical for all stringers unless noted otherwise)



DETAIL 1



PLANGRATING AT CURB



SECTION E-E

(Diagonal bars not shown for clarity)

Notes:

- 1. See sheet S-36 for additional notes and location of Section A-A, B-B, and C-C.
- 2. See sheet S-38 for curb details.

REFERENCE DRAWINGS

Drawing

Roadway Steel Decking

Sheet No. 1660570250



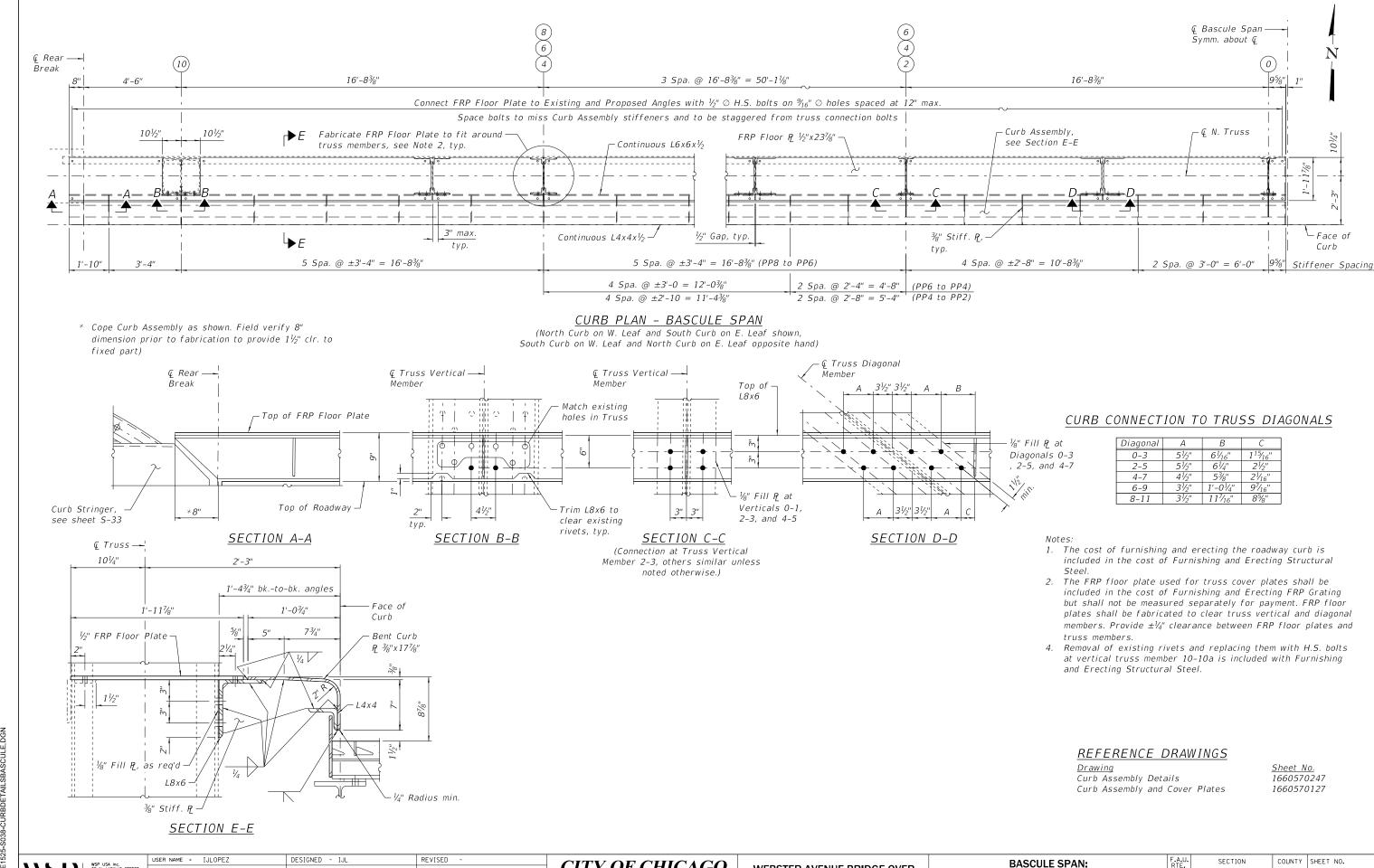
USER NAME = IJLOPEZ DESIGNED - IJL REVISED CHECKED - NBR REVISED DRAWN - IJL REVISED PLOT DATE = \$DATE\$ CHECKED - JIG REVISED

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

BASCULE SPAN: DECK DETAILS (STRUCTURE NO. 016-6057)

SECTION COUNTY SHEET NO. S-37 11-E1525-00-BR COOK CDOT PROJECT NO. E-1-525 80 of 210



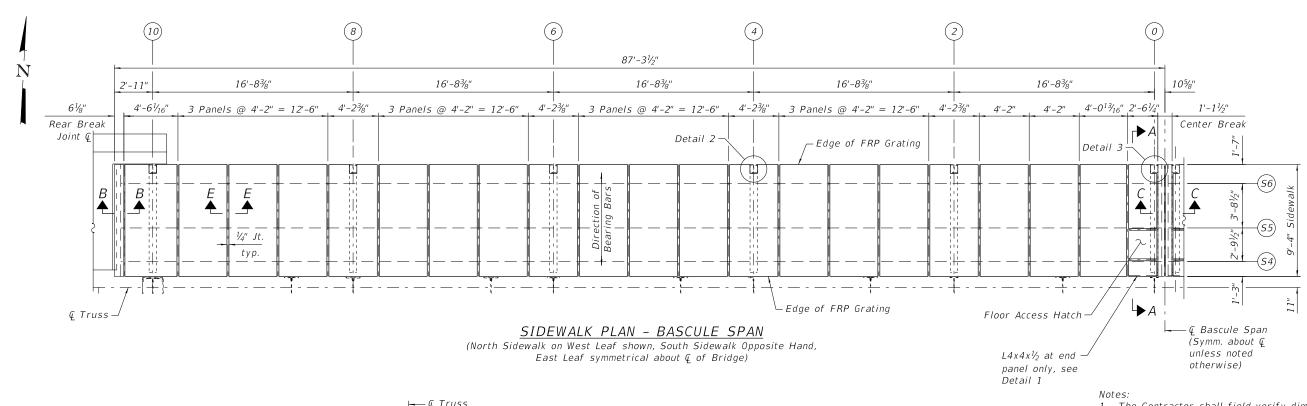
CHECKED - NBR REVISED PLOT SCALE = N.T.S. - IJL REVISED PLOT DATE = \$DATF\$ CHECKED - JIG REVISED

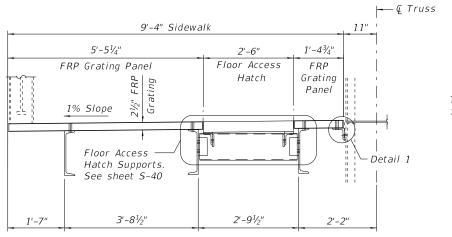
CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

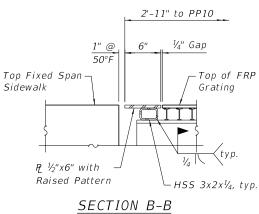
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

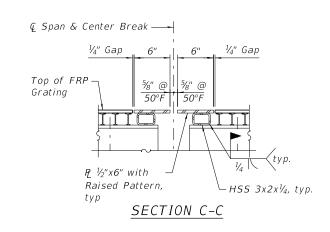
CURB DETAILS (STRUCTURE NO. 016-6057)

S-38 11-E1525-00-BR COOK CDOT PROJECT NO. E-1-525 81 of 210



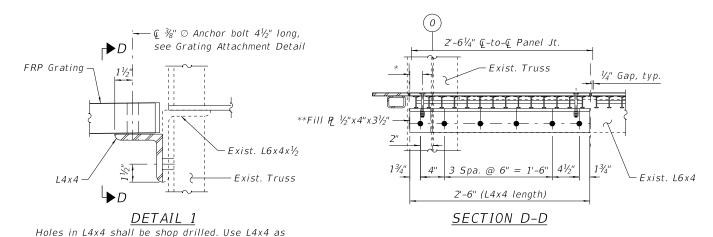






SECTION A-A SIDEWALK GRATING AT FLOOR ACCESS HATCH

template to field drill holes on existing L6x4 and Truss.



- * Per FRP Grating Manufacturer. Coordinate anchor bolt location with structural steel fabricator. Cost included with Furnishing and Erecting FRP Grating.
- ** Length of Fill P to be field verified.

MINIMUM SECTION PROPERTIES

(Properties shown are per foot width of FRP Grating)

I = 1.68 in 4

St = 1.96 in 3

Sb = 1.47 in3

 $Avg. EI = 7,600,000 \ lb-in2 \ (Span >= 24")$

- 1. The Contractor shall field verify dimensions of existing sidewalk posts prior to fabrication of sidewalk grating. A gap of $\frac{1}{2}$ " ($\pm\frac{1}{16}$ ") shall be provided around the existing sidewalk posts.
- Holes in FRP panels shall be shop drilled. Coordinate fabrication of FRP with Sidewalk Stringer fabrication. Cost for coordinating this effort is included with Furnishing and Erecting FRP Grating.
- 3. Raised pattern steel plates shall conform to the requirements of ASTM A786.
- 4. Place panels on supporting members and adjust into final position with proper bearing and alignment at joints and supports and fasten immediately. End panel at the breaks shall be the last one to be installed. Adjust height, pitch of the end panels to meet fixed span rear breaks after live load shimming has been completed. Verify required gap at the center and rear breaks. Make corrections to panel widths as necessary to meet required clearances and gaps.
- 5. HSS tubes for sidewalk rear and center breaks shall be hot-dip galvanized in accordance with AASHTO M111. See Special Provisions. Cost included with Furnishing and Erecting Structural
- 6. Cost of furnishing and installation of the sidewalk rear and center breaks shall included in the cost of Furnishing and Erecting Structural Steel.
- 7. Anchor bolts, nuts and washers for FRP Grating connections to sidewalk stringers shall be Stainless Steel Type 316. Cost included with Furnishing and Erecting FRP Grating.
- 8. See sheet S-40 for Details 2 and 3, Section E-E, Floor Door Supports, and for additional FRP Grating and connection details.

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Furnishing and Erecting FRP Grating	Sq. Ft.	3,227
Floor Access Hatch	Each	4

Quantity shown includes West and East Leaf

REFERENCE DRAWINGS

<u>Drawing</u> Sidewalk Decking Sheet No. 1660570251

USER NAME = IJLOPEZ DESIGNED - IJL REVISED CHECKED - NBR REVISED PLOT SCALE = N.T.S. - IJL REVISED PLOT DATE = \$DATF\$ CHECKED - JIG REVISED

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

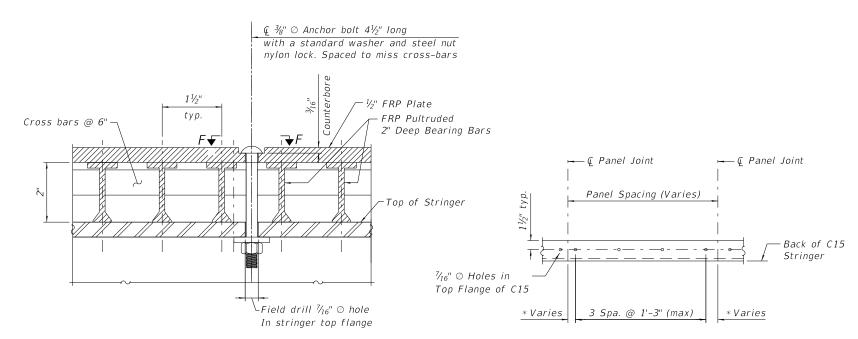
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

BASCULE SPAN: SIDEWALK PLAN (STRUCTURE NO. 016-6057)

SECTION COUNTY SHEET NO. S-39 11-E1525-00-BR COOK CDOT PROJECT NO. E-1-525 82 of 210

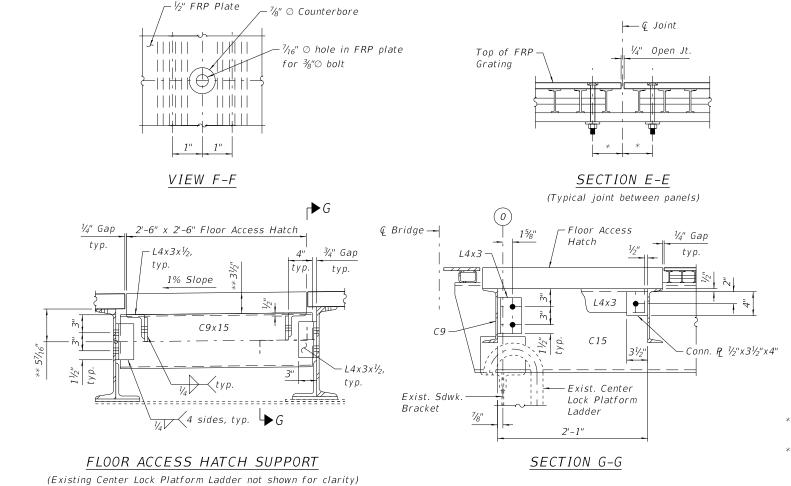


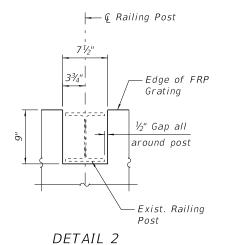
1. See sheet S-39 for additional notes and locations of Section E-E and Details 2 and 3.



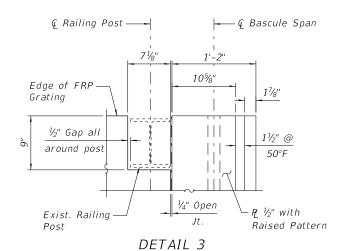
GRATING ATTACHMENT DETAIL

CONNECTION LAYOUT FOR FRP PANELS





(Typical cutout detail around railing posts at PP2 thru PP10)



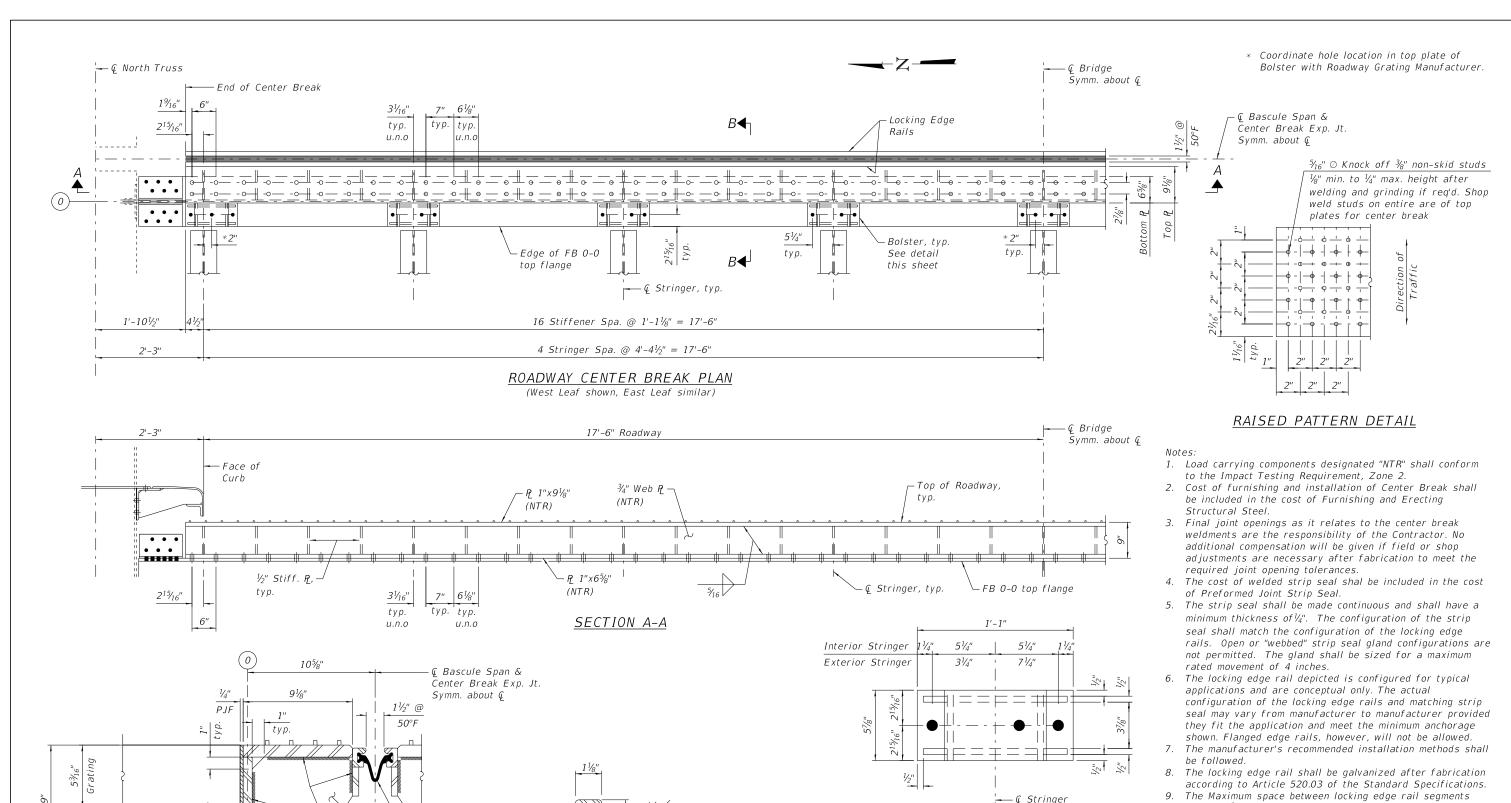
(Cutout detail around railing posts at PPO)

- Minimum edge distance per FRP Grating Manufacturer Specifications.
- ** Floor Access Hatch Support details based on overall depth of $3\frac{1}{2}$ ". Dimensions may vary depending on Floor Access Hatch manufacturer's design.



USER NAME = IJLOPEZ	DESIGNED - IJL	REVISED -
	CHECKED - NBR	REVISED -
PLOT SCALE = N.T.S.	DRAWN - IJL	REVISED -
PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -
TEST DATE - WDATEW	CHECKED 010	WE A LOED

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-40
CDOT	PROJECT NO. E-1-525		83 of 210



17/8" 27/8" 11/2"

W21

LOCKING EDGE RAIL

Strip Seal

½" Stiff. ₽

Mill to bear

REVISED

REVISED

REVISED

REVISED

** Back gouge not required if complete joint penetration is verified by mock-up.

3/8"

typ. TYPICAL BOLSTER DETAIL AT FB 0-0

(Bolster shown at interior stringer, similar at exterior stringers except as shown)

- The Maximum space between locking edge rail segments shall be $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail on sheet S-42.
- 10. Center break weldment, bolster, locking edge rails, bolts, nuts, and washers shall be galvanized according to AASHTO M111 or M232, as applicable.

BILL OF MATERIAL

[Item	Unit	Quantity
ĺ	Preformed Joint Strip Seal	Foot	36

Bolster

USER NAME = IJLOPEZ DESIGNED - IJL CHECKED - NBR DRAWN - IJL PLOT DATE = \$DATF\$ CHECKED - JIG

SECTION B-B

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

 $P_{6}^{7}/_{16}$ " $\times 5^{7}/_{8}$ " $\times 1'-1'$

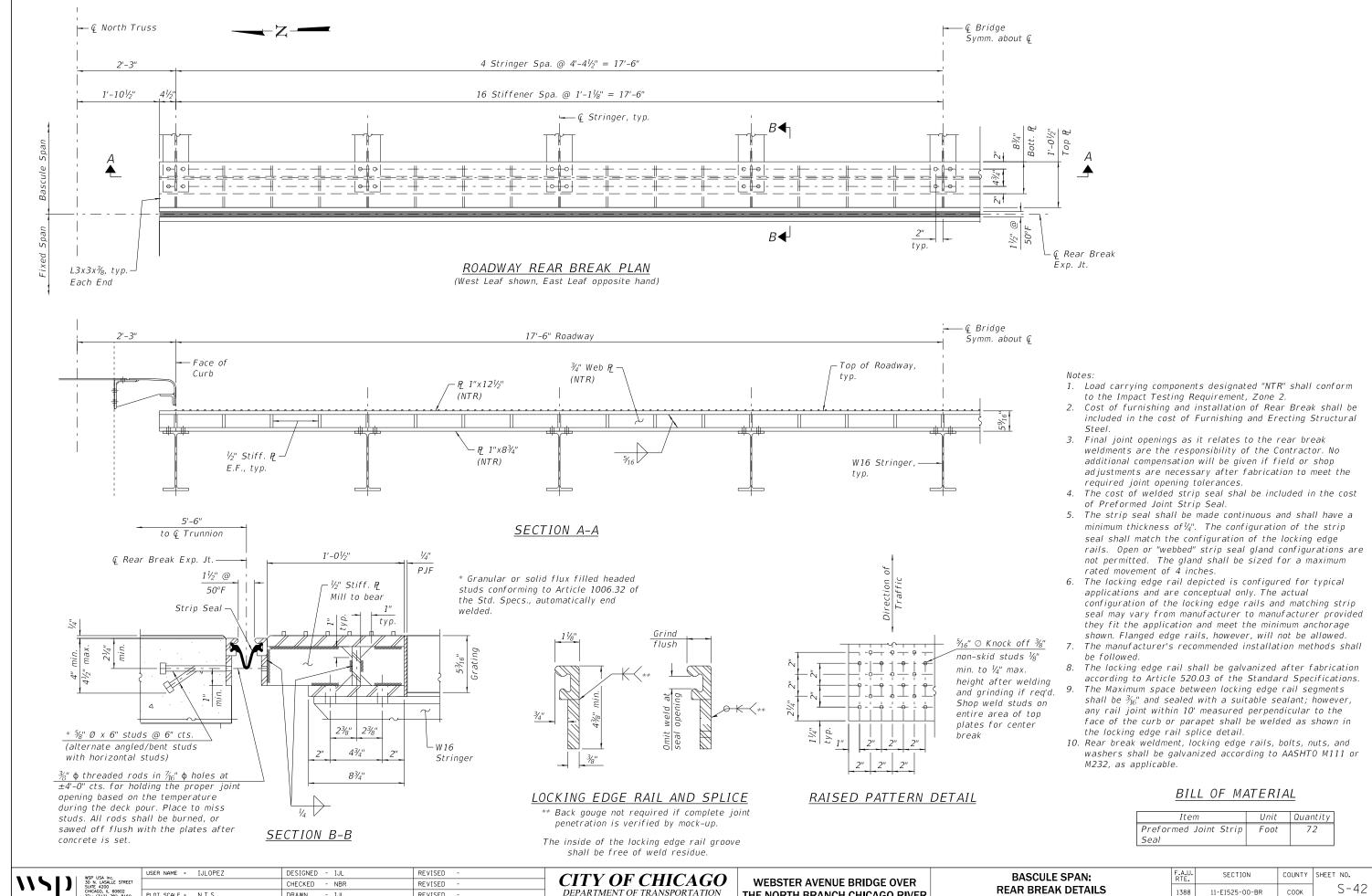
1/4

BASCULE SPAN: **CENTER BREAK DETAILS** (STRUCTURE NO. 016-6057)

— 2-Stiff. - Р<u>.</u> ½"х1½"х7"

1/2" Stiff. P

SECTION COUNTY SHEET NO. S-41 11-E1525-00-BR СООК CDOT PROJECT NO. E-1-525 84 of 210



DIVISION OF ENGINEERING

THE NORTH BRANCH CHICAGO RIVER

(STRUCTURE NO. 016-6057)

CDOT PROJECT NO. E-1-525

85 of 210

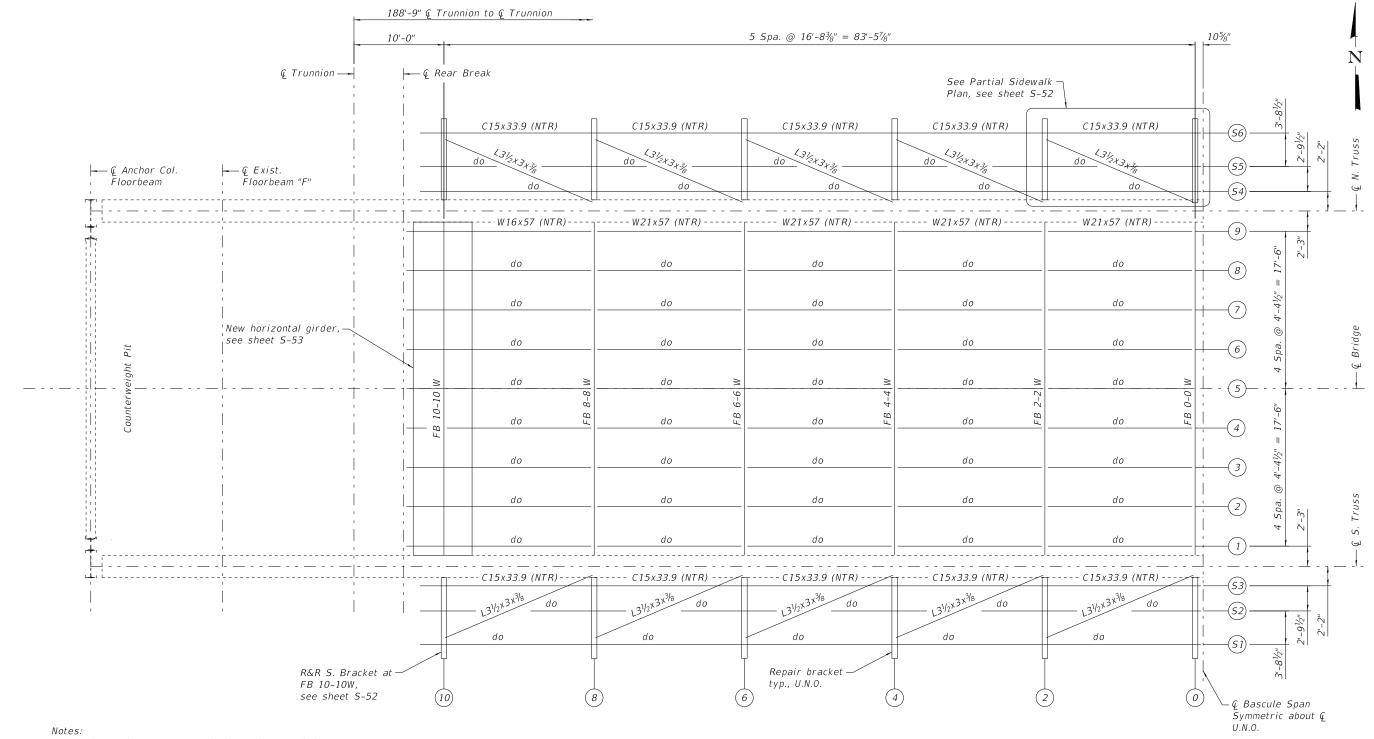
- IJL

CHECKED - JIG

PLOT DATE = \$DATF\$

REVISED

REVISED



- 1. Load carrying components designated "NTR" shall conform to the Impact Testing Requirement, Zone 2.
- 2. The cost of removal of the floor system of stringers, floorbeams, and sidewalk lateral bracing shall be included in the cost of Strucutral Steel Removal. The cost of proposed steel for the floor system shall be included in the cost of Furnishing and Erecting Structural Steel.
- 3. The cost of repairs to the existing sidewalk brackets, including removal and replacement, shall be included in the cost of Structural Steel Repairs. See Special Provision.

FRAMING PLAN

(West Leaf shown, East Leaf opposite hand)

REFERENCE DRAWINGS

<u>Drawing</u> Movable Part- Stringers and Sub-Planking Erection Diagram

<u>Sheet No.</u> 1660570015 1660570245

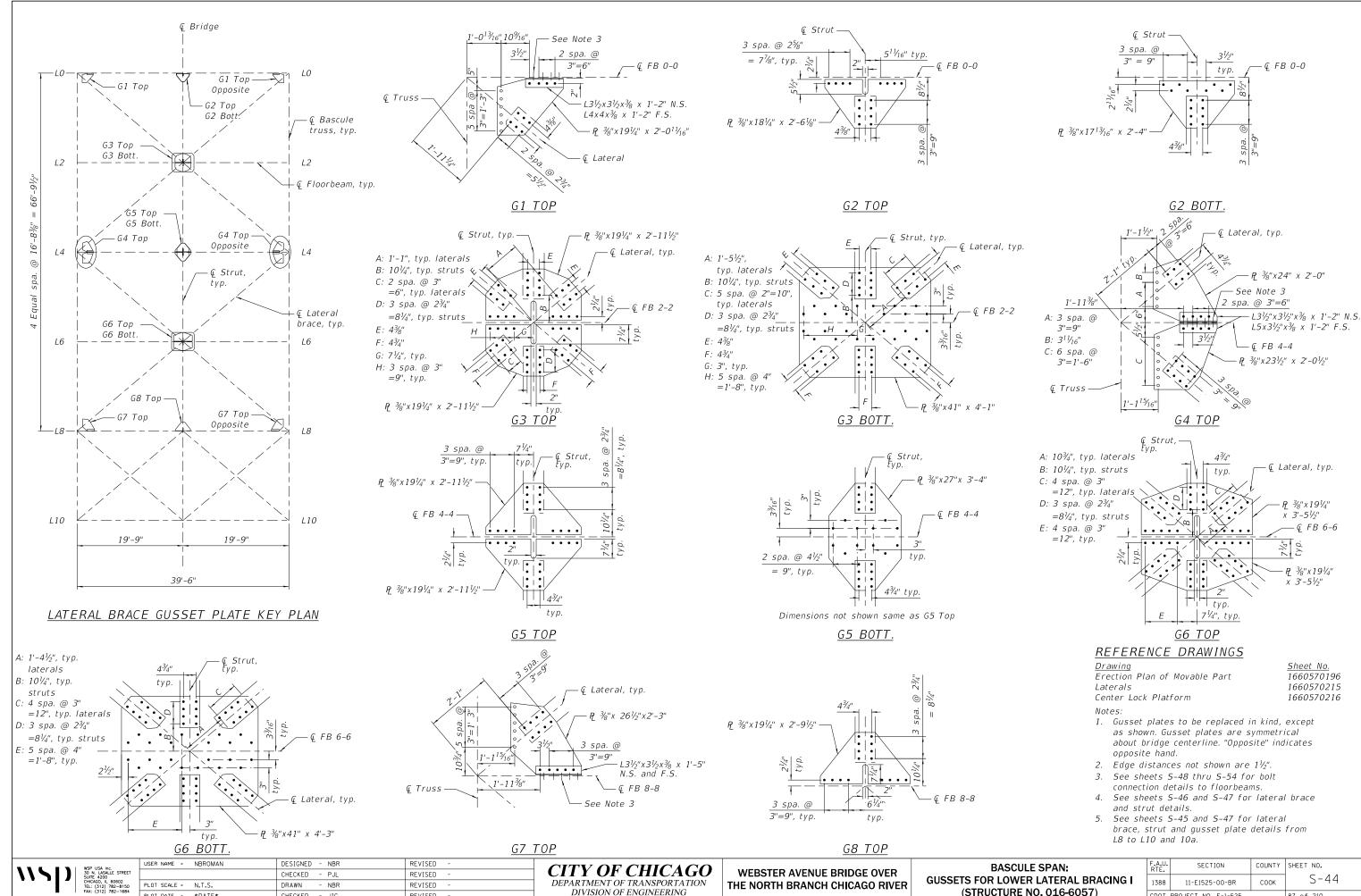


USER NAME = PATELN	DESIGNED - NJP	REVISED -
	CHECKED - PJL	REVISED -
PLOT SCALE = N.T.S.	DRAWN - NJP	REVISED -
PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -

CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

BASCULE SPAN: FRAMING PLAN (STRUCTURE NO. 016-6057) | SECTION | COUNTY | SHEET NO. | | SHEET NO. | | SHEET NO. | | SHEET NO. | SHE



DEPARTMENT OF TRANSPORTATION

DIVISION OF ENGINEERING

THE NORTH BRANCH CHICAGO RIVER

REVISED

REVISED

CHECKED - JIG

GUSSETS FOR LOWER LATERAL BRACING I

(STRUCTURE NO. 016-6057)

1388

11-E1525-00-BR

CDOT PROJECT NO. E-1-525

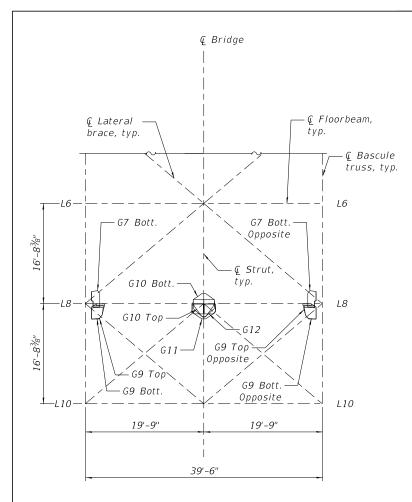
S-44

87 of 210

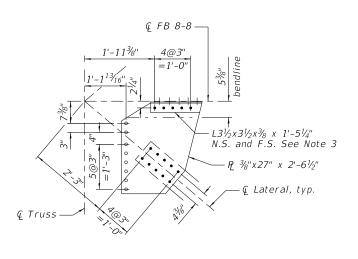
COOK

PLOT SCALE = N.T.S.

PLOT DATE = \$DATF\$

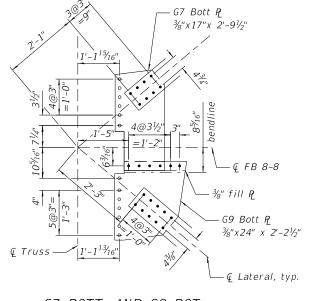


LATERAL BRACE GUSSET PLATE KEY PLAN



G9 TOP

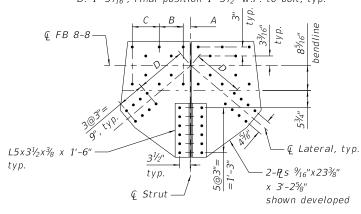
G10 BOTT.



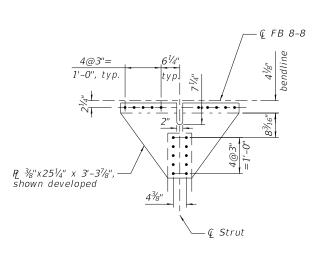
G7 BOTT. AND G9 BOT



- $B: 2 \ spa. @ 4" = 8", typ.$
- C: 2 spa. @ $4\frac{1}{2}$ " = 9", typ.
- D: $1'-5\frac{1}{16}$ ", final position $1'-3\frac{1}{2}$ " W.P. to bolt, typ.

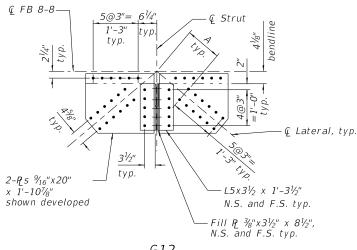


(Strut and laterals not shown for clarity)



G10 TOP

A: $11\frac{7}{8}$ ", final position 1'-0" W.P. to bolt, typ.



(Strut and laterals not shown for clarity)

REFERENCE DRAWINGS

<u>Drawing</u> Bracing for Floor Beam 10-10 Center Lock Platform

<u>Sheet No.</u> 1660570210 1660570216

Notes:

- 1. Gusset plates to be replaced in kind, except as noted. Gusset plates are symmetrical about the bridge centerline. "Opposite" indicates opposite hand.
- 2. Edge distances not shown are $1\frac{1}{2}$ " minimum.
- 3. See sheets S-48 thru S-54 for bolt connection details to the floorbeams.
- See sheets S-46 and S-47 for lateral brace and strut details.
- 5. See sheets S-44 and S-46 for lateral brace, strut and gusset plate details from LO to L8.



USER NAME = NBROMAN	DESIGNED - NBR	REVISED -
	CHECKED - PJL	REVISED -
PLOT SCALE = N.T.S.	DRAWN - NBR	REVISED -
PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -

A: 2½" typ.

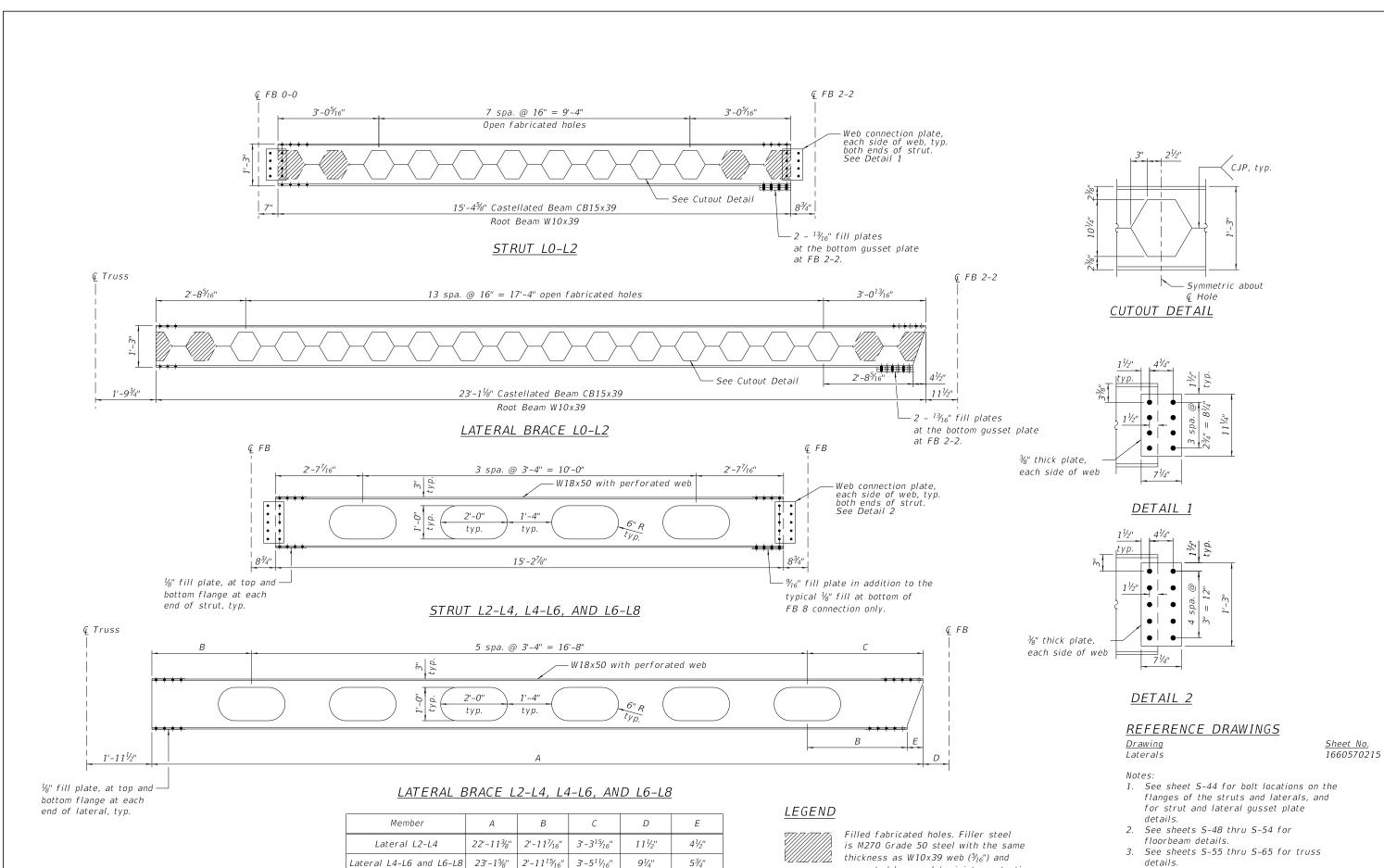
B: 2 spa. @ 4'' = 8'', typ.

C: 2 spa. @ $4\frac{1}{2}$ " = 9", typ.

 $P_{L}^{3}/8" \times 42" \times 3'-10^{7}/8"$ shown developed

- Ç Strut, typ.

	F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
l	1388	11-E1525-00-BR	соок	S-45
	CDOT	PROJECT NO. E-1-525		88 of 210



WSP USA Inc.
39 Pt. AGALE STREET
SUITE AGO
CHICAGO, III. 60602
TEL: (312) 782–1850
FAX: (312) 782–1864

CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

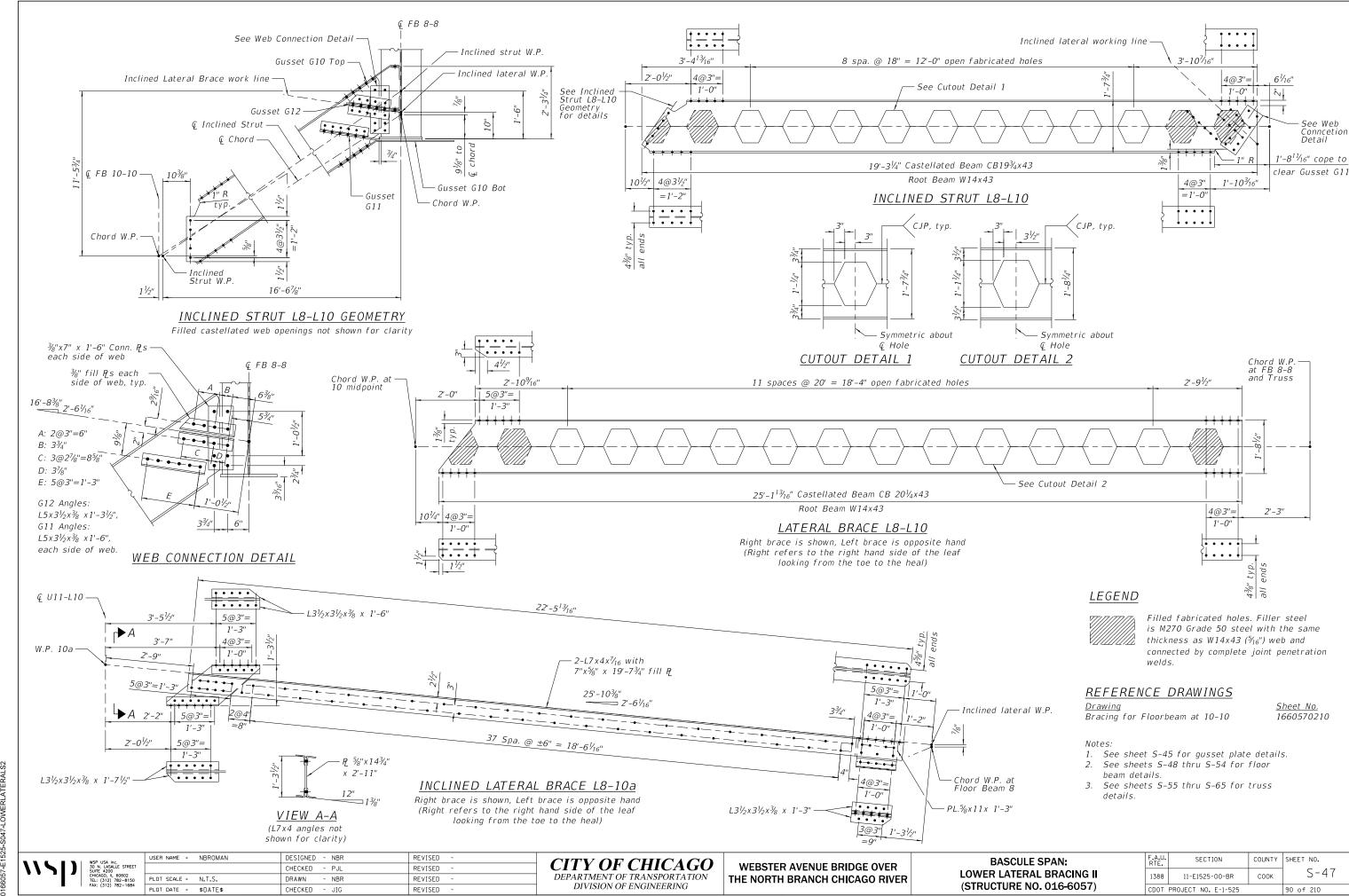
welds.

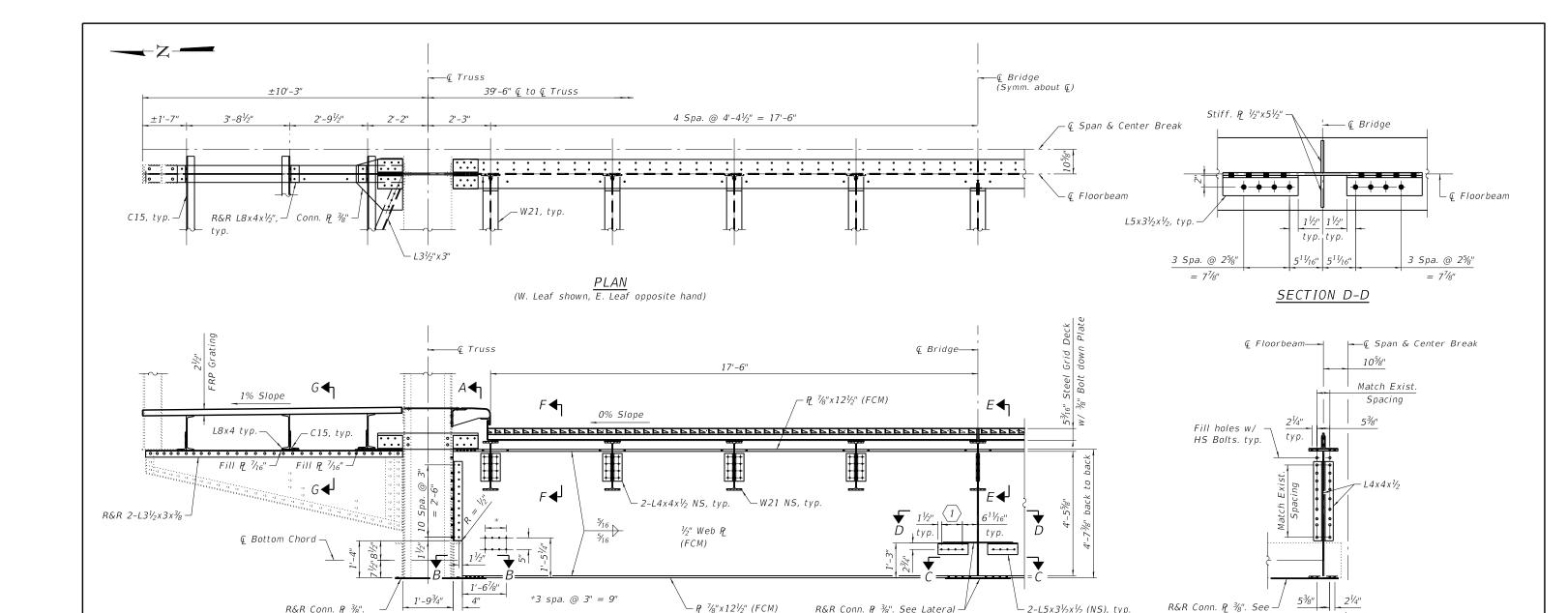
connected by complete joint penetration

BASCULE SPAN: LOWER LATERAL BRACING I (STRUCTURE NO. 016-6057) F.A.U. SECTION COUNTY SHEET NO.

1388 11-E1525-00-BR COOK S-46

CDOT PROJECT NO. E-1-525 89 of 210





ELEVATION (W. Leaf Floorbeam looking East, E. Leaf Floorbeam similar but opposite and looking West)

€ Span &

Center Break

€ Floorbeam

Bott. Flange

€ Span & — ⊊ Bridge Center Break Conn. P2 1/2"x51/2" € Floorbeam - Bott. Flange 3 Spa. @ 3" 3 Spa. @ 3" = 9" SECTION C-C

 $2-L5x3\frac{1}{2}x\frac{1}{2}$ (NS), typ.

3 Spa. @ 3" = 9", typ.

R&R Conn. R 3%". See Lateral

Bracing Details

SECTION A-A

Notes:

Truss Repairs

- 1. Load carrying components designated "NTR" shall conforms to the Impact Testing Requirement, Zone 2.
- 2. Load carrying components designated "FCM" shall be fabricated according to the provisions of Clause 12 of the AASHTO/AWS D1.5 Bridge Welding Code.
- Install HS Bolts at existing holes, cost included with Furnishing and Erecting Structural Steel.
- See sheet S-50 for Tie Plate Detail, Section E-E & F-F, Connection Plate and Stringer Connection.
- See sheet S-51 for Section G-G.
- 6. See sheet S-52 for Lateral Bracing at sidewalk.

REFERENCE DRAWINGS

<u>Drawing</u> Movable Part - Floorbeams Erection Plan - Movable Plan Main Truss Members Main Truss Members Floor Beams Laterals

USER NAME = PATELN DESIGNED - NJP REVISED CHECKED - PJL REVISED DRAWN - NJP REVISED PLOT DATE = 9/16/2020 CHECKED - JIG REVISED

 $A \blacktriangleleft$

SECTION B-B

R&R Conn. If 3/8".

See Truss Repairs

€ Truss ---

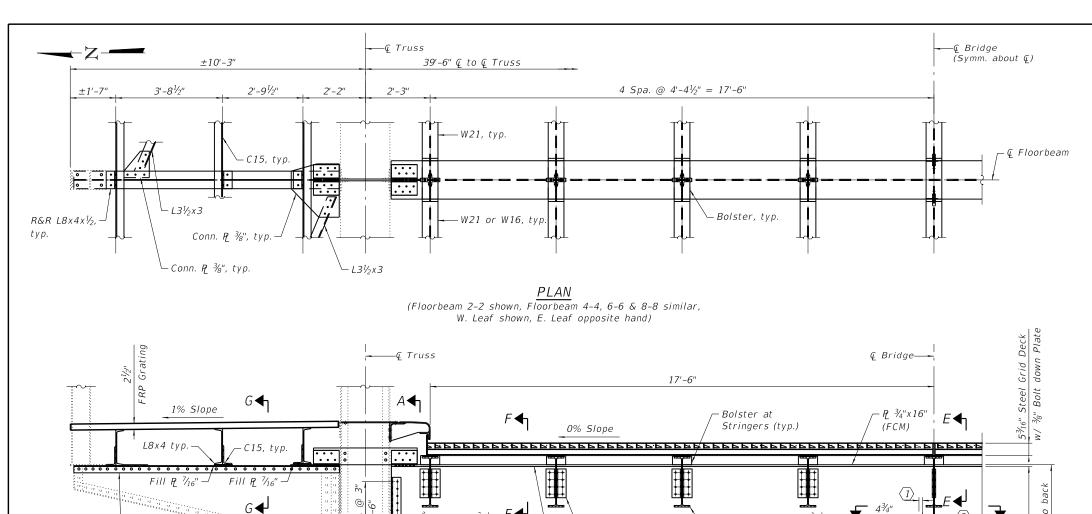
CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

 $P_{L}^{7}/8" \times 12^{1}/2" (FCM)$

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

BASCULE SPAN: FLOORBEAM 0-0 (STRUCTURE NO. 016-6057)

COUNTY SHEET NO. SECTION S-48 11-E1525-00-BR соок CDOT PROJECT NO. E-1-525 91 of 210



See Truss Repairs				
Bracket Panel Pt.	Dimension "A"	Dimension "B"		
2	53/4"	3 spa. @ 3" = 9"		
4	5¾"	3 spa. @ 3" = 9"		
6	51/4"	3 spa. @ 3" = 9"		
8	41/8"	4 spa. @ 3" = 1'-0"		

R&R 2-L3½x3x¾

€ Bottom Chord -

R&R Conn. R 38".

FLOORBEAMS 2-2, 4-4, 6-6 & 8-8 ELEVATION

1'-6¹/2"

1'-9³/₄"

2-L4x4x½ or

 $2-L6x6x^{1/2}$, typ.

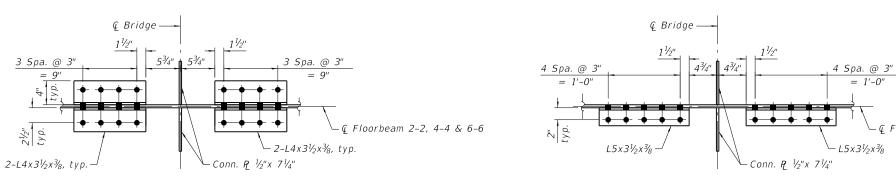
(W. Leaf Floorbeam looking east, E. Leaf Floorbeam similar but opposite hand looking west) $\langle 1 \rangle$ 1½", typ.

R&R Conn. R 3/8". See -

Lat. Bracing Details

 \bigcirc 3 Spa. @ 3" = 9", typ.

③〉 4 Spa. @ 3" = 1'-0", typ.



SECTION I-I

SECTION J-J

—⊈ Floorbeam **-**—⊈ Floorbeam Match Exist. Match Exist. Spacing Spacing Fill Bolt Fill Hole w/ HS-Bolts, typ. typ. typ. Match Exist. Spacing $L6x4x\frac{1}{2}$ $-L4x4x\frac{1}{2}$ FLOORBEAM 6-6 & 8-8 SHOWN FLOORBEAM 2-2 & 4-4 SHOWN

SECTION A-A

Notes:

- 1. Load carrying components designated "NTR" shall conforms to the Impact Testing Requirement, Zone 2.
- Load carrying components designated "FCM" shall be fabricated according to the provisions of Clause 12 of the AASHTO/AWS D1.5 Bridge Welding Code.
- 3 Install HS Bolts at existing holes, cost included with Furnishing and Erecting Structural Steel
- See sheet S-50 for Section E-E & F-F, Tie Plate Detail, Stringer Conn. Detail, Connection Plate Detail and Bolster Detail.
- 5 See sheet S-51 for Section H-H & G-G.

REFERENCE DRAWINGS

THE PREMIET BY WITHOU	
<u>Drawing</u>	Sheet No.
Movable Part-Stringers and Sub-Planking	1660570010
Erection Plan-Movable Plan	1660570196
Main Truss Members	1660570200
Main Truss Members	1660570203
Floorbeam	1660570206
Floorbeam	1660570207
Laterals	1660570215

WSP USA Inc.
30 N. LASALLE STREET
SUITE 4200
CHICAGO, IL 60602
TEL: (312) 782-8150
FAX: (312) 782-1884

CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

-W21 or

½" Web PL (FCM)

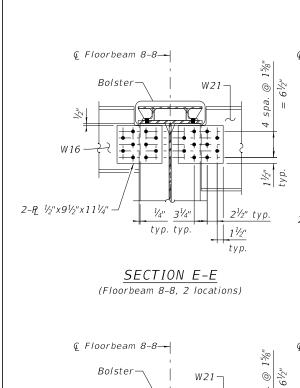
W16, typ.

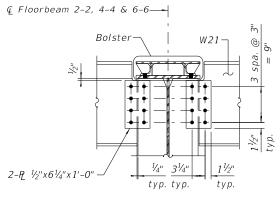
- P_L ¾"х16" (FCM)

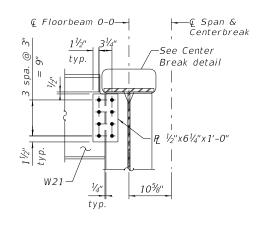
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

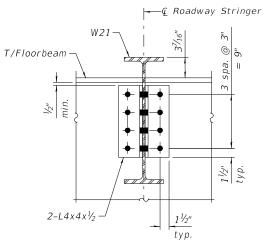
BASCULE SPAN: FLOORBEAM 2-2, 4-4, 6-6 & 8-8 (STRUCTURE NO. 016-6057)

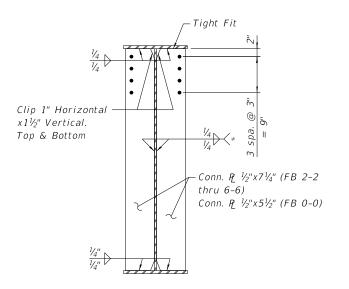
€ Floorbeam 8-8











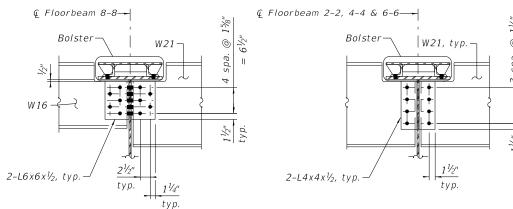
SECTION E-E (Floorbeam 2-2, 4-4 & 6-6, 6 locations)

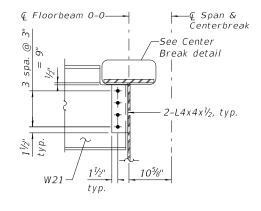
SECTION E-E (Floorbeam 0-0, 2 locations)

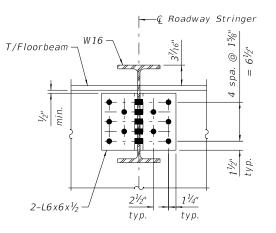
TYPICAL W21 STRINGER CONN.

TYPICAL CONNECTION PLATE

* Terminate $\frac{1}{4}$ " ($\pm \frac{1}{8}$) from the end of plate intersects



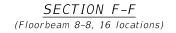




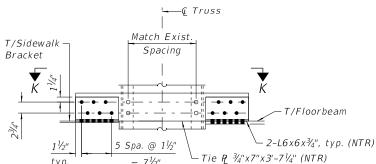
SECTION F-F (Floorbeam 0-0, 16 locations)

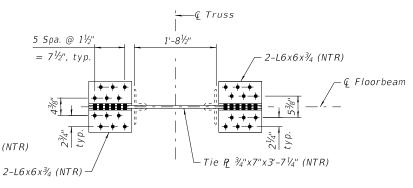
TYPICAL W16 STRINGER CONN.

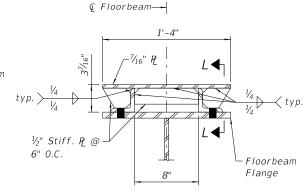
– Tight Fit Clip 1" Horizontal $x1\frac{1}{2}$ " Vertical. Top & Bottom Conn. P₂ ½"x7¼" (FB 8-8)



SECTION F-F (Floorbeam 2-2, 4-4 & 6-6, 48 locations)







CONNECTION PLATE - FB 8-8

* Terminate $\frac{1}{4}$ " ($\pm \frac{1}{8}$) from the end of plate intersects

<u></u> ← <u>Q</u> Stringer Mill to Bear L3x3x1/2

TIE PLATE DETAIL (20 locations)

SECTION K-K

TYPICAL BOLSTER DETAIL

SECTION L-L

Notes:

1. Load carrying components designated "NTR" shall conforms to the Impact Testing Requirement, Zone 2.



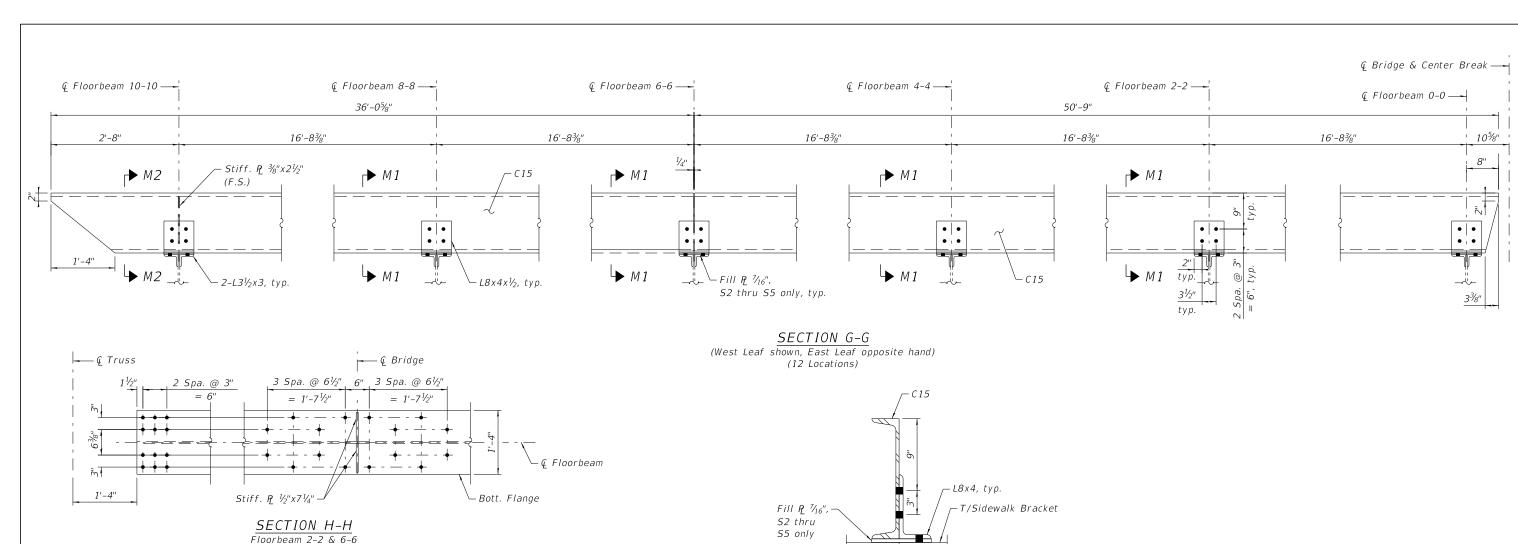
	USER NAME = PATELN	DESIGNED - NJP	REVISED -
		CHECKED - PJL	REVISED -
	PLOT SCALE = N.T.S.	DRAWN - NJP	REVISED -
	PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -
_			

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

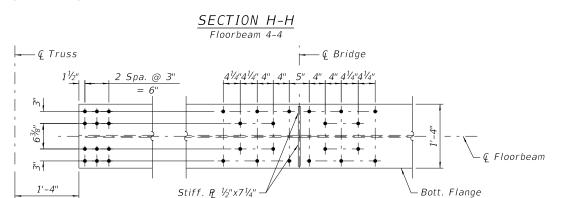
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

BASCULE SPAN: FLOORBEAM DETAILS I (STRUCTURE NO. 016-6057)

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-50
CDOT	PROJECT NO. E-1-525		93 of 210

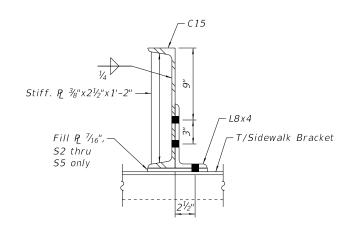


– ⊊ Bridge Truss 2 Spa. @ 3" 2 Spa. @ 4½". 2 Spa. @ 4½" Floorbeam 1'-4" Stiff. PL 1/2"x71/4" -- Bott. Flange



SECTION H-H Floorbeam 8-8

SECTION M1-M1



SECTION M2-M2

1. For locations of Section G-G and Section H-H, see sheets S-48 and S-49.

REFERENCE DRAWINGS

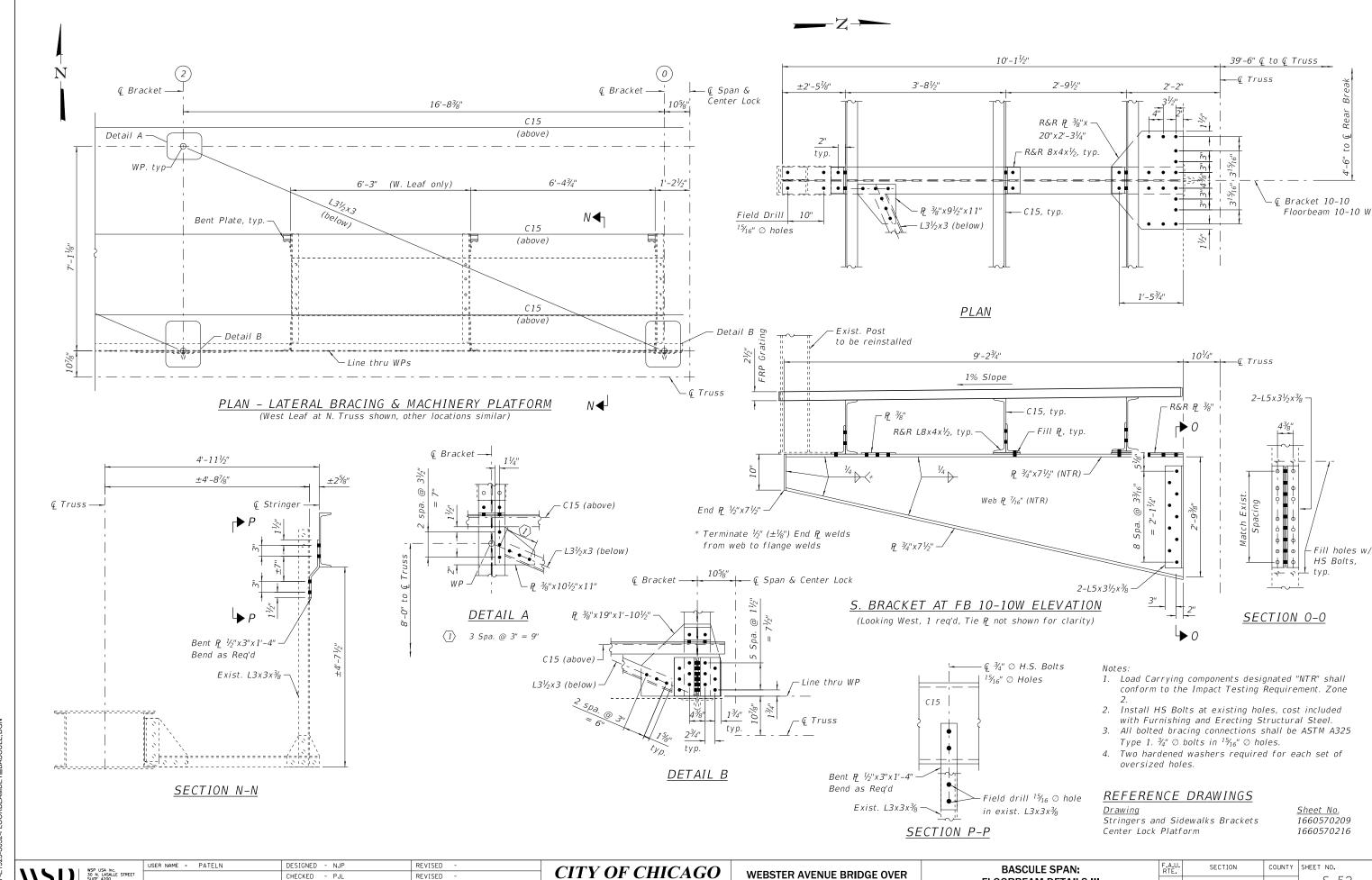
<u>Drawing</u> Floorbeams Floorbeams Walkway Stringers and Cover Plate Details

<u>Sheet No.</u> 1660570206 1660570207 1660570248

115	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684
------------	---

USER NAME = PATELN	DESIGNED - NJP	REVISED -
	CHECKED - PJL	REVISED -
PLOT SCALE = N.T.S.	DRAWN - NJP	REVISED -
PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -

RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-51
CDOT	PROJECT NO. E-1-525		94 of 210



0166057-E1525-S052-FLOORBEAMDETIIIB.

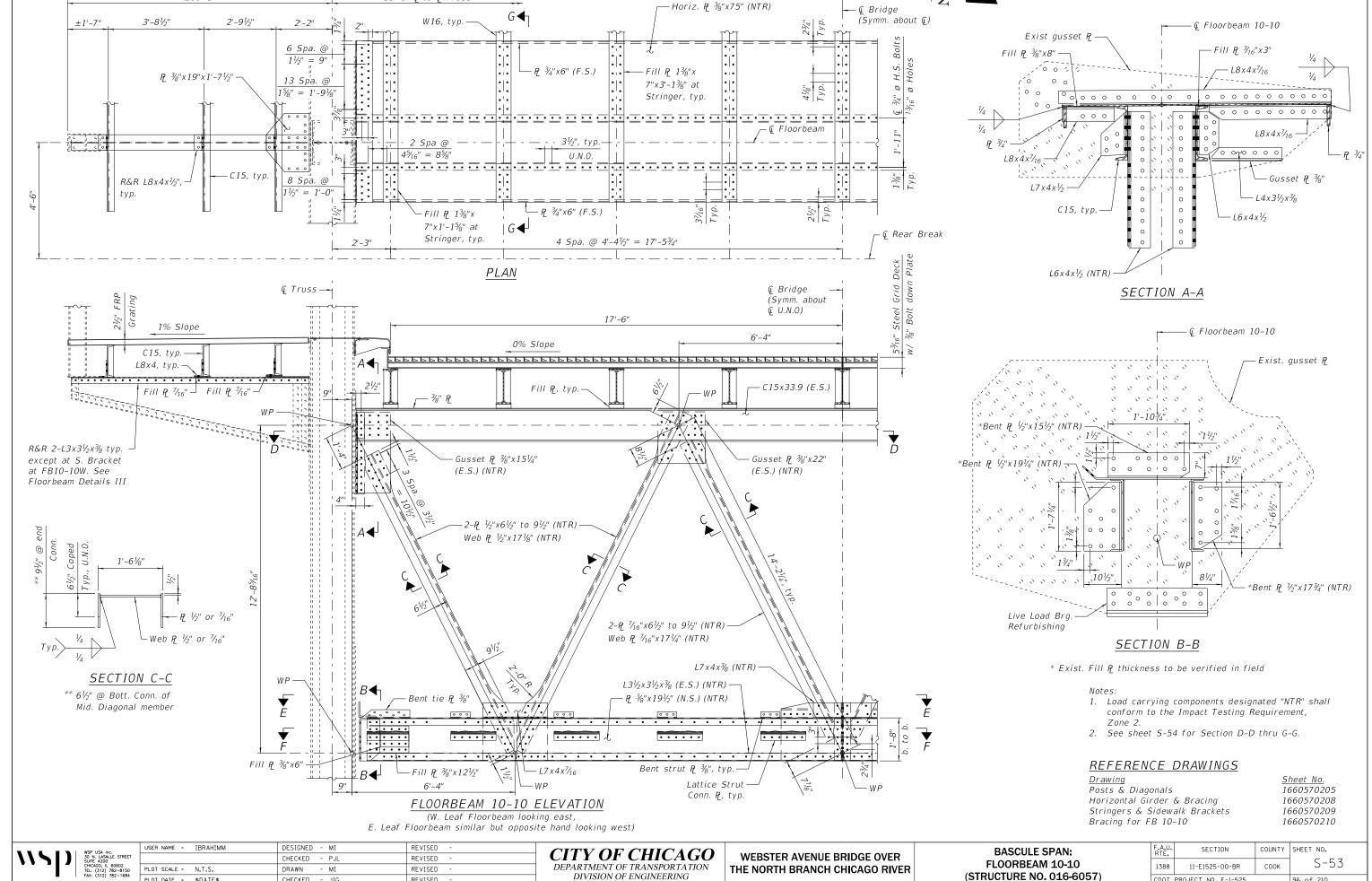
WSP USA inc. 30 N. LASALE S SUITE (420) CHECKOO, 0 606 FEL: (312) 782-FAX: (312) 782-

CITY OF CHICAGO

DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER
THE NORTH BRANCH CHICAGO RIVER

FLOORBEAM DETAILS III (STRUCTURE NO. 016-6057)



CDOT PROJECT NO. E-1-525

96 of 210

← Truss →

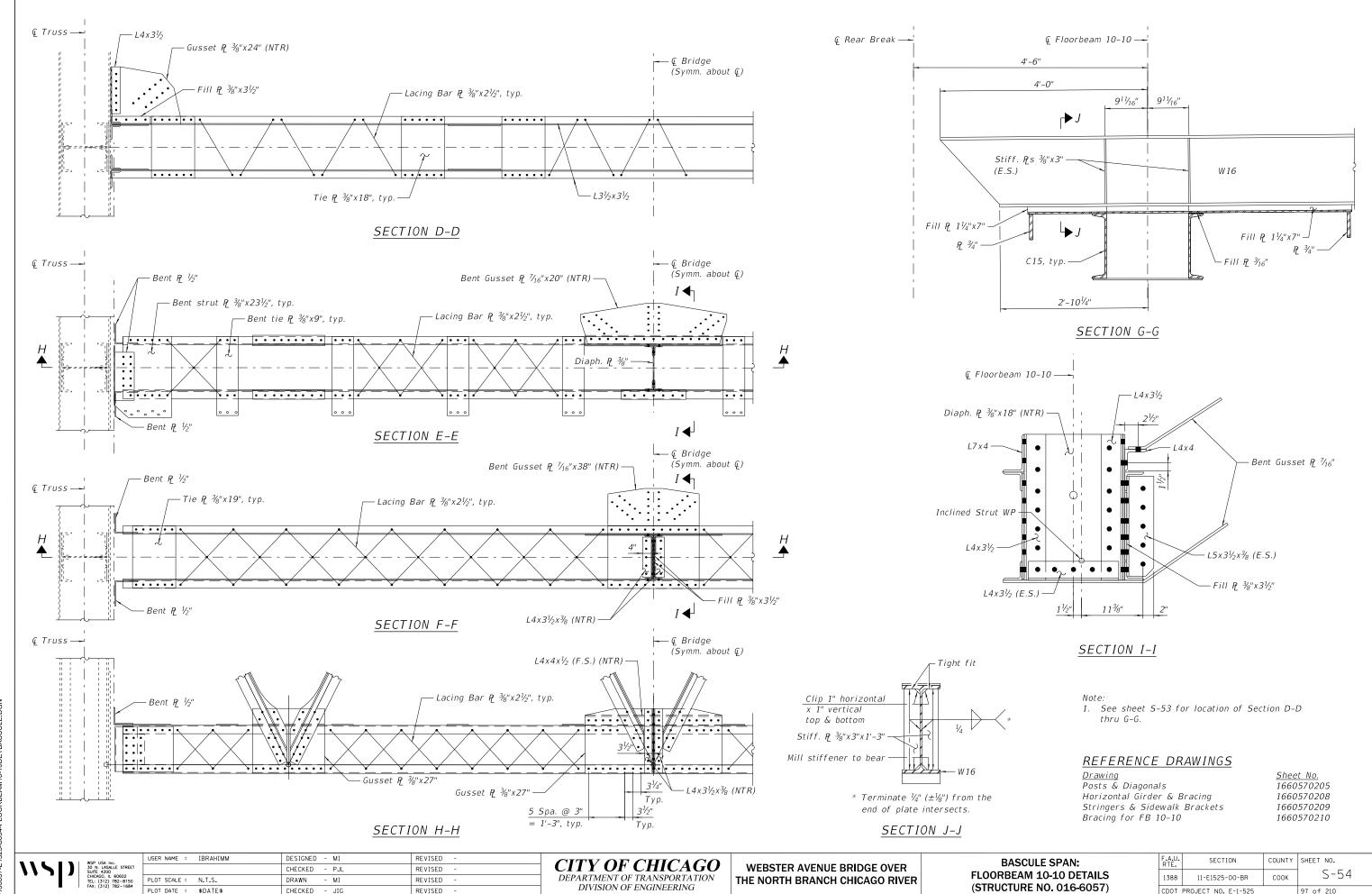
39'-6" & to & Truss

 $\pm 10'-3''$

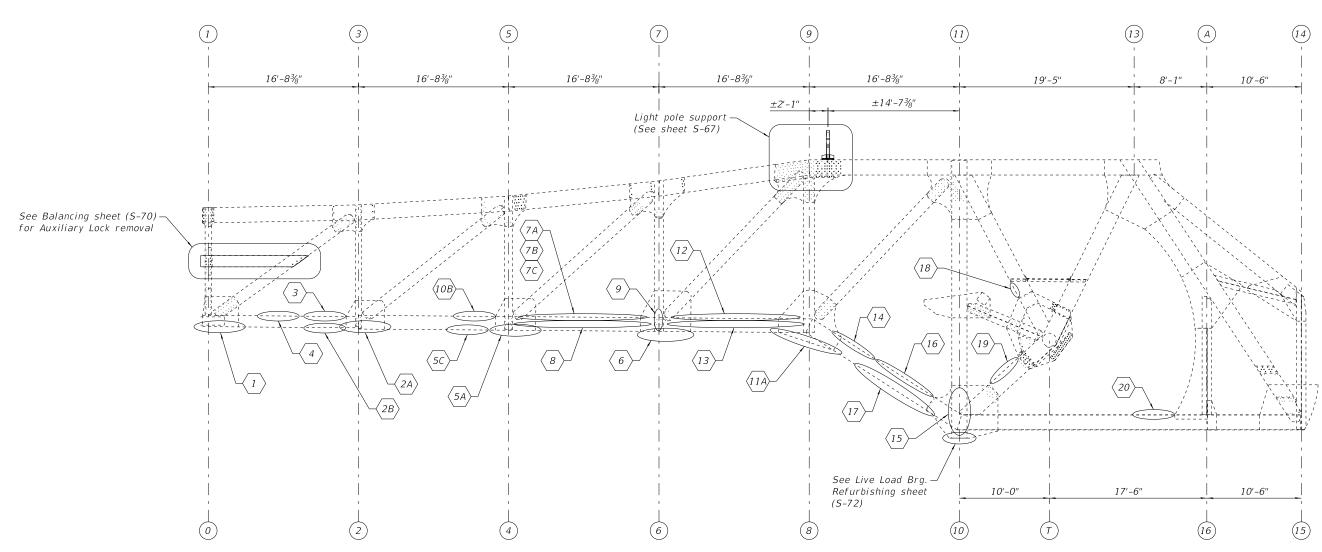
PLOT DATE = \$DATE\$

CHECKED - JIG

REVISED



0166057-E1525-S054-FLOORBEAM10-10DETBA



SOUTHWEST TRUSS ELEVATION (Looking south)

- \langle 1 \rangle R&R Bott. Conn. P2 (See sheet S-59)
- $\langle 2A \rangle$ R&R Bott. Conn. R & Lacing Bars (See sheet S-59)
- $\langle 2B \rangle$ R&R Bott. Lacing Bars (See sheet S-59)
- R&R Top Batten P (See sheet S-59)
- $\langle 4 \rangle$ R&R Top Batten R(See sheet S-59)
- $\langle 5A \rangle$ R&R Bott. Conn. P2 (See sheet S-60)

- $\langle 5C \rangle$ R&R Bott. Lacing Bars (See sheet S-60)
- \langle 6 \rangle R&R Bott. Conn. R & Lacing Bars (See sheet S-60)
- $\langle 7A \rangle$ R&R Top Batten R R&R Bott. Lacing Bars R&R Top Batten R (See sheet S-60)
- 8 R&R Diaphragm Top Angles & P (See sheet S-61)
- Strengthen Diaphragm R (See sheet S-61)
- (10B) R&R Top Batten P (See sheet S-61)

- (11A) R&R Bott. Conn. P., Wedge P.s & Adjacent Lacing Bars (See sheet S-62)
- $\langle 12 \rangle$ R&R Top Batten P2s & Top Lacing Bars (See sheet S-62)
- R&R Diaphragm Top Angles & P (See sheet S-63)
- \langle 14angle R&R Top Batten P (See sheet S-63)

- $\langle 15 \rangle$ Strengthening Post 10-11 at Live Load (See sheet S-64)
- R&R Top Batten R & Top Lacing Bars (See sheet S-65)
- (17) R&R Bott. Batten R (See sheet S-65)
- (18) R&R Bott. Batten R (See sheet S-66)
- $\langle 19 \rangle$ R&R Top Batten R_2 s (See sheet S-66)
- 20 R&R Top Batten P (See sheet S-67)

- 1. Locations to be strengthened shall be verified in the
- 2. Cost of furnishing and erecting truss repair steel elements shall be included in "Structural Steel Repair".
- Cost of removing and replacing rivets in association with furnishing and erecting truss repair steel elements shall be included in "Structural Steel Repair".
- 4. See sheets S-59 thru S-67 for Truss Repair Details.

REFERENCE DRAWINGS

<u>Drawing</u> General Plan & Elevation Erection Plan of Movable Part <u>Sheet No.</u> 1660570123 1660570196

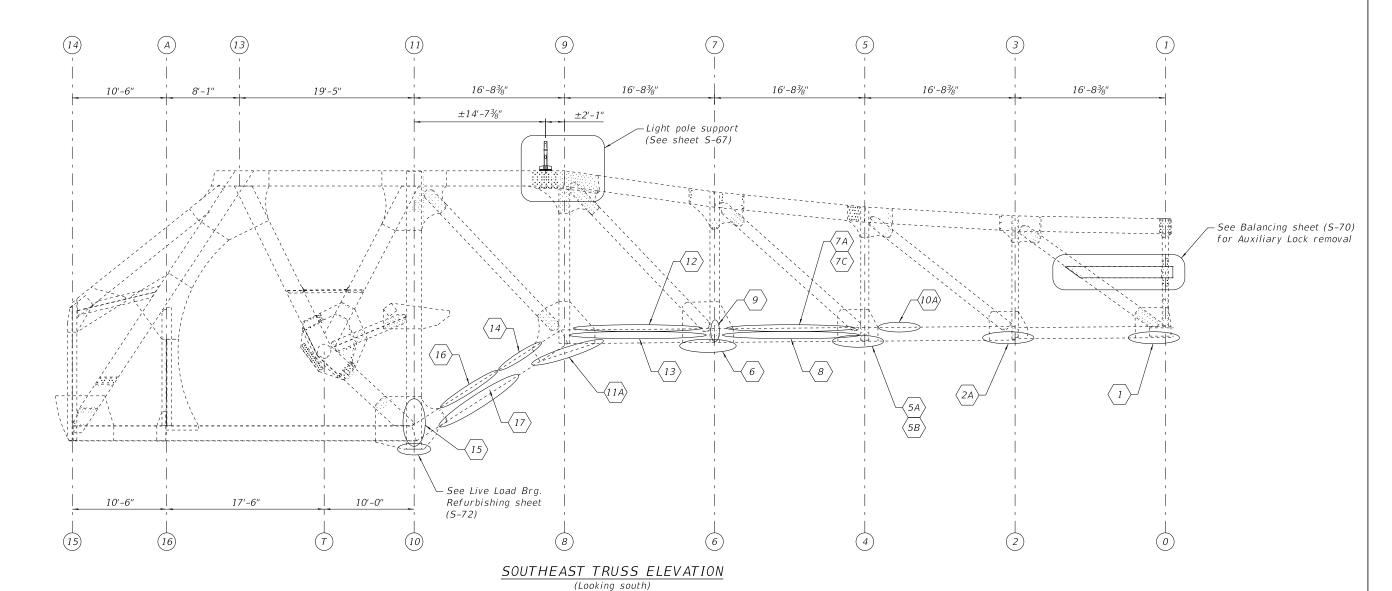
USER NAME = PJLAUX	DESIGNED - LD/PJL	REVISED -
	CHECKED - NBR	REVISED -
PLOT SCALE = N.T.S.	DRAWN - LD/PJL	REVISED -
PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

BASCULE SPAN: SW TRUSS REPAIRS (STRUCTURE NO. 016-6057)

COUNTY SHEET NO. SECTION S-55 11-E1525-00-BR соок CDOT PROJECT NO. E-1-525 98 of 210



- \langle 1 \rangle R&R Bott. Conn. P2 (See sheet S-59)
- ⟨2A⟩ R&R Bott. Conn. № & Lacing Bars (See sheet S-59)
- SA) R&R Bott. Conn. R R&R Bott. Lacing B (See sheet S-60) R&R Bott. Lacing Bars
- \langle 6 \rangle R&R Bott. Conn. IP & Lacing Bars (See sheet S-60)
- ⟨7A⟩ R&R Top Batten ₽ & R&R Top Batten R (See sheet S-60)
- R&R Diaphragm Top Angles & R (See sheet S-61)
- Strengthen Diaphragm P (See sheet S-61)
- (See sheet S-61)

- (11A) R&R Bott. Conn. P., Wedge P.s & Adjacent Lacing Bars (See sheet S-62)
- $\langle 12
 angle$ R&R Top Batten R_2 s & Top Lacing Bars (See sheet S-62)
- R&R Diaphragm Top Angles & P (See sheet S-63)
- \langle 14 \rangle R&R Top Batten R(See sheet S-63)

- $\langle 15 \rangle$ Strengthening Post 10-11 at Live Load (See sheet S-64)
- (See sheet S-65)
- (17) R&R Bott. Batten R (See sheet S-65)

- 1. Locations to be strengthened shall be verified in the
- 2. Cost of furnishing and erecting truss repair steel elements shall be included in "Structural Steel Repair".
- 3. Cost of removing and replacing rivets in association with furnishing and erecting truss repair steel elements shall be included in "Structural Steel Repair".
- 4. See sheets S-59 thru S-67 for Truss Repair Details.

REFERENCE DRAWINGS

<u>Drawing</u>

General Plan & Elevation Erection Plan of Movable Part <u>Sheet No.</u> 1660570123 1660570196

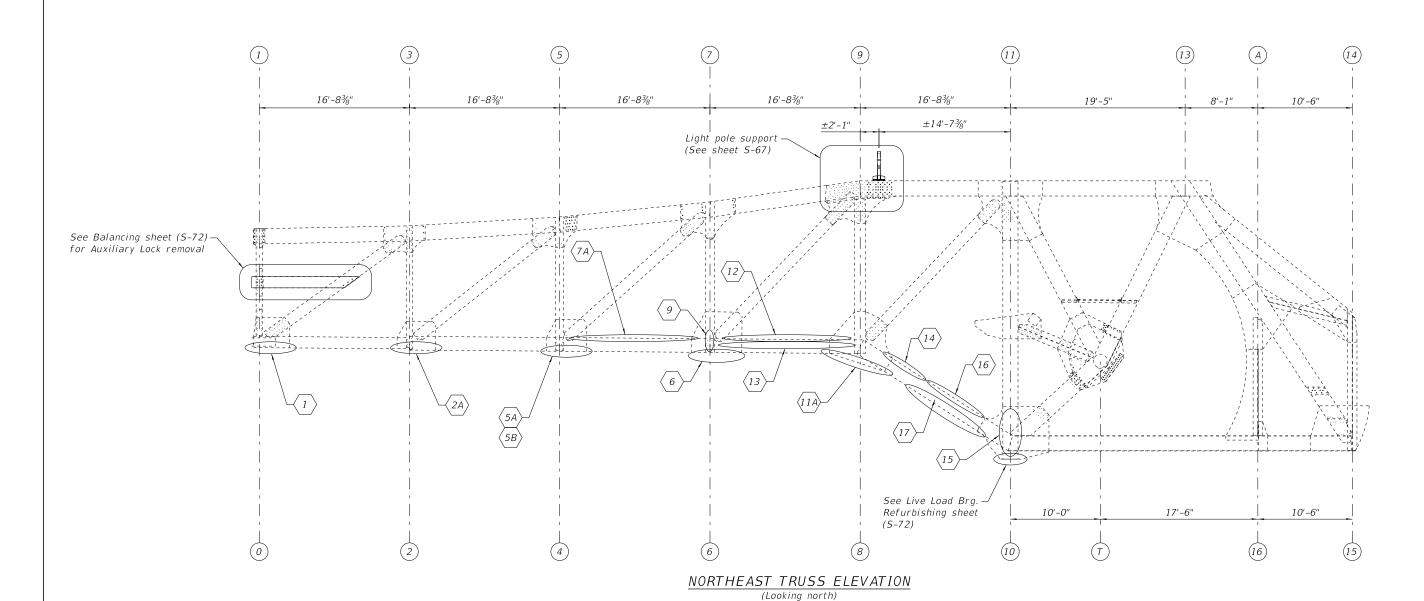
USER NAME = PJLAUX DESIGNED - LD/PJL REVISED CHECKED - NBR REVISED - LD/PJL REVISED PLOT DATE = \$DATE\$ CHECKED - JIG REVISED

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

BASCULE SPAN: SE TRUSS REPAIRS (STRUCTURE NO. 016-6057)

COUNTY SHEET NO. SECTION S-56 11-E1525-00-BR соок CDOT PROJECT NO. E-1-525 99 of 210



1 R&R Bott. Conn. R (See sheet S-59)

2A R&R Bott. Conn. P. & Lacing Bars (See sheet S-59)

5A) R&R Bott. Conn. P. & Lacing Bars R&R Bott. Lacing Bars (See sheet S-60)

6 R&R Bott. Conn. P. & Lacing Bars (See sheet S-60)

 $\langle 7A \rangle$ R&R Top Batten P (See sheet S-60)

 \langle 9 angle Strengthen Diaphragm R(See sheet S-61)

(11A) R&R Bott. Conn. P., Wedge P.s & Adjacent Lacing Bars (See sheet S-62)

 $\langle 12
angle$ R&R Top Batten Rs & Top Lacing Bars (See sheet S-62)

R&R Diaphragm Top Angles & P (See sheet S-63)

 $\langle 14 \rangle$ R&R Top Batten P (See sheet S-63)

 $\langle 15 \rangle$ Strengthening Post 10-11 at Live Load (See sheet S-64)

R&R Top Batten R & Top Lacing Bars (See sheet S-65)

(17) R&R Bott. Batten R (See sheet S-65)

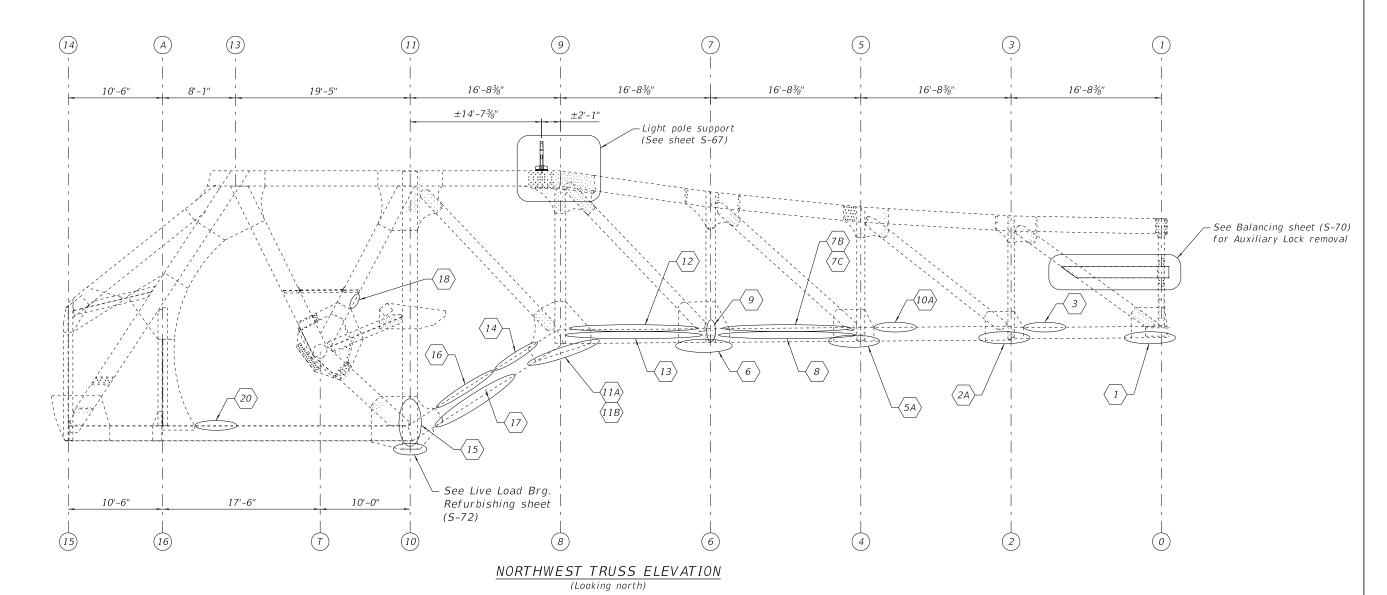
- 1. Locations to be strengthened shall be verified in the
- 2. Cost of furnishing and erecting truss repair steel
- elements shall be included in "Structural Steel Repair".

 3. Cost of removing and replacing rivets in association with furnishing and erecting truss repair steel elements shall be included in "Structural Steel Repair".
- 4. See sheets S-59 thru S-67 for Truss Repair Details.

REFERENCE DRAWINGS

<u>Drawing</u> General Plan & Elevation Erection Plan of Movable Part <u>Sheet No.</u> 1660570123 1660570196

USER NAME = PJLAUX	DESIGNED - LD/PJL	REVISED -
	CHECKED - NBR	REVISED -
PLOT SCALE = N.T.S.	DRAWN - LD/PJL	REVISED -
PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -



- \langle 1 \rangle R&R Bott. Conn. P2 (See sheet S-59)
- $\langle 2A \rangle$ R&R Bott. Conn. R & Lacing Bars (See sheet S-59)
- \langle 3 \rangle R&R Top Batten R (See sheet S-59)
- (5A) R&R Bott. Conn. P. & Lacing Bars (See sheet S-60)
- \langle 6 \rangle R&R Bott. Conn. IP & Lacing Bars (See sheet S-60)
- (7B) R&R Bott. Lacing Bars R&R Top Batten R (See sheet S-60)
- \langle 8 angle R&R Diaphragm Top Angles & P (See sheet S-61)
- \langle 9 angle Strengthen Diaphragm R(See sheet S-61)
- (10A) R&R Top Batten P (See sheet S-61)

- (11A) R&R Bott. Conn. P., Wedge P.s & Adjacent Lacing Bars
- Adjacent La 11B R&R Angle (See sheet S-62)
- $\langle 12 \rangle$ R&R Top Batten P_2 s & Top Lacing Bars (See sheet S-62)
- \langle 13angle R&R Diaphragm Top Angles & P (See sheet S-63)
- \langle 14angle R&R Top Batten R(See sheet S-63)

- $\langle 15 \rangle$ Strengthening Post 10-11 at Live Load (See sheet S-64)
- R&R Top Batten R & Top Lacing Bars (See sheet S-65)
- R&R Bott. Batten PL (See sheet S-65)
- \langle 18angle R&R Top Batten R (See sheet S-66)
- \langle 20angle R&R Bott. Batten R (See sheet S-67)

- 1. Locations to be strengthened shall be verified in the
- 2. Cost of furnishing and erecting truss repair steel elements shall be included in "Structural Steel Repair".
- 3. Cost of removing and replacing rivets in association with furnishing and erecting truss repair steel elements shall be included in "Structural Steel Repair".
- 4. See sheets S-59 thru S-67 for Truss Repair Details.

REFERENCE DRAWINGS

<u>Drawing</u> General Plan & Elevation Erection Plan of Movable Part <u>Sheet No.</u> 1660570123 1660570196

ונוציי	WSP USA Inc. 30 N. LASALLE STREE SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-168
--------	---

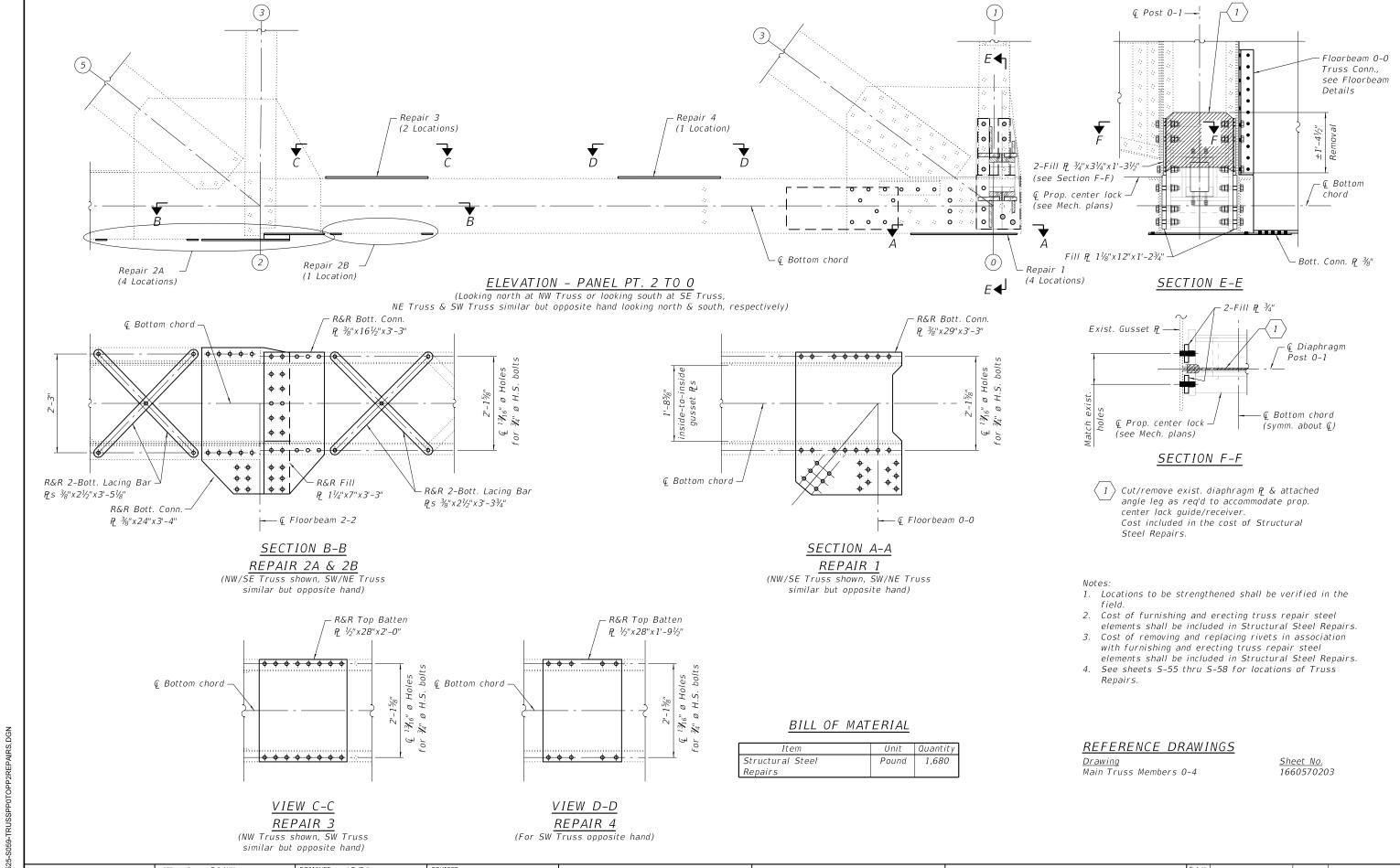
USER NAME = PJLAUX	DESIGNED - LD/PJL	REVISED -
	CHECKED - NBR	REVISED -
PLOT SCALE = N.T.S.	DRAWN - LD/PJL	REVISED -
PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

BASCULE SPAN: NW TRUSS REPAIRS (STRUCTURE NO. 016-6057)

COUNTY SHEET NO. SECTION S-58 11-E1525-00-BR соок CDOT PROJECT NO. E-1-525 101 of 210



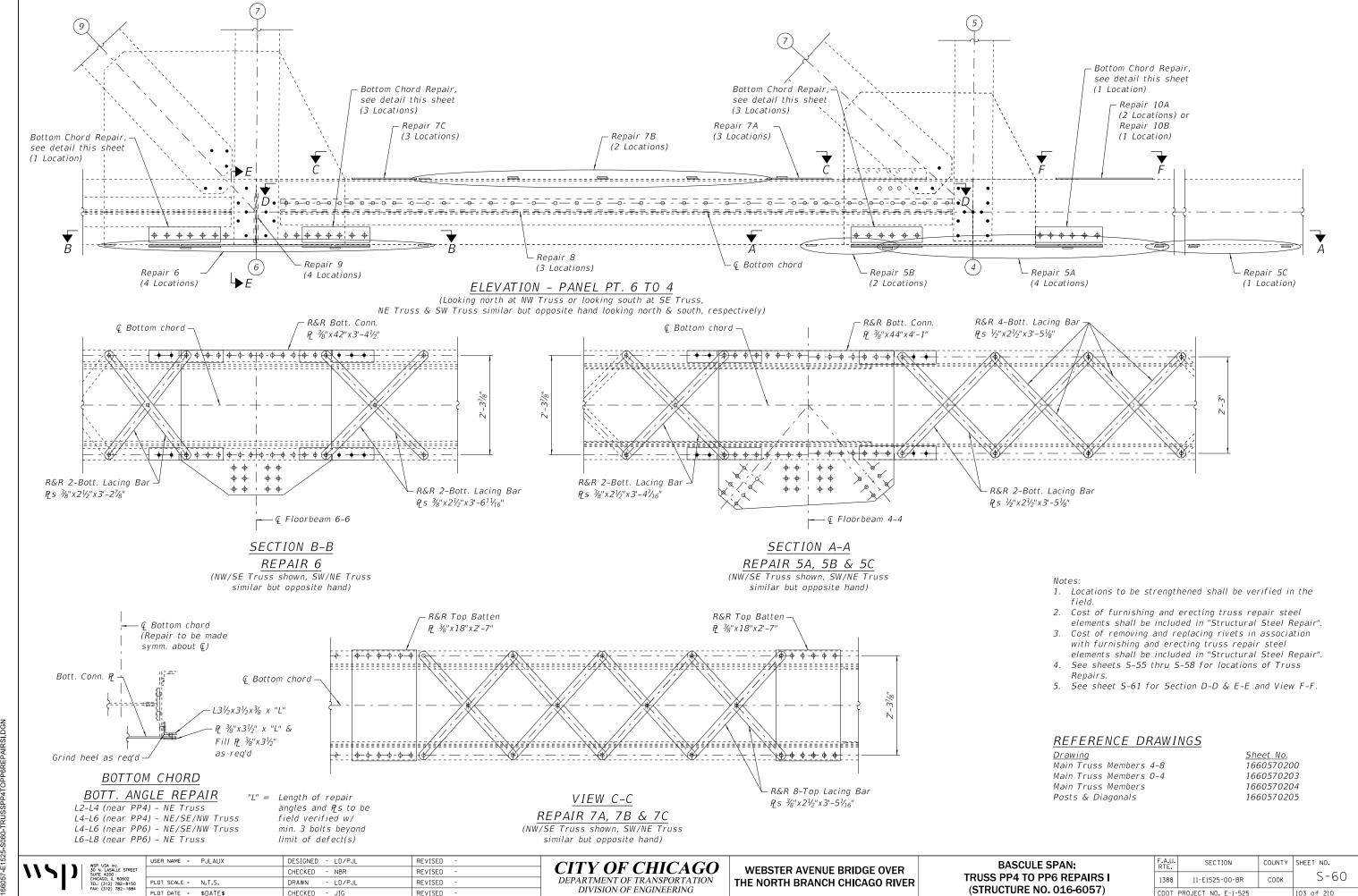
USER NAME = PJLAUX DESIGNED - LD/PJL REVISED CHECKED - NBR REVISED DRAWN - LD/PJL REVISED PLOT DATE = 10/5/2020 CHECKED - JIG REVISED

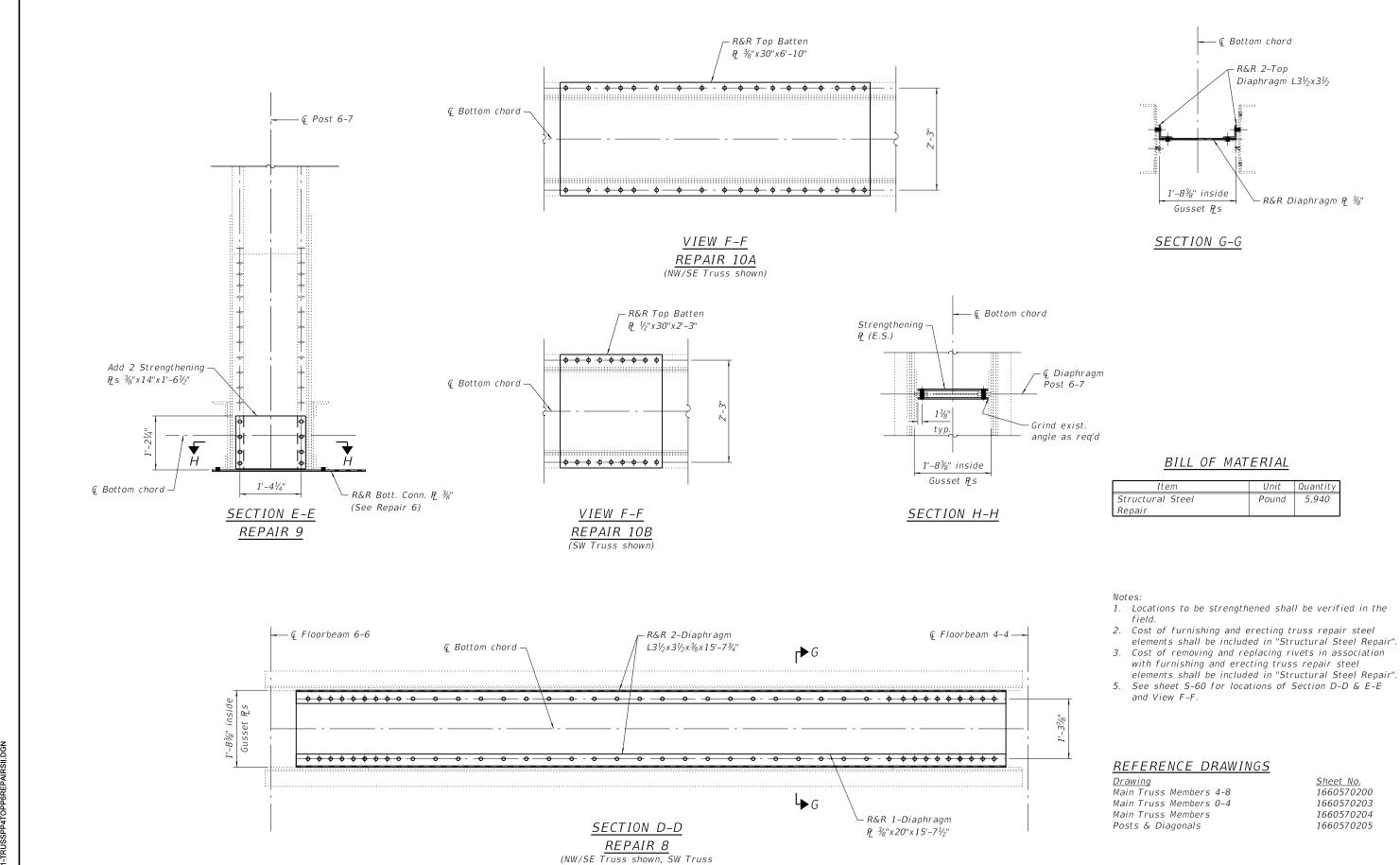
CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

BASCULE SPAN: TRUSS PP0 TO PP2 REPAIRS (STRUCTURE NO. 016-6057)

SECTION COUNTY SHEET NO. S-59 11-E1525-00-BR соок CDOT PROJECT NO. E-1-525 102 of 210





0166057-E1525-S061-TRUSSPP4TOPP6RE

WSP USA Inc. 30 N. ASALLE STREET SUITE 4200 CHICAGO, IL 06602 TEL: (312) 782–8150 FAX: (312) 782–1684
 USER NAME =
 PJLAUX
 DESIGNED - LD/PJL
 REVISED

 CHECKED - NBR
 REVISED

 PLOT SCALE = N.T.S.
 DRAWN - LD/PJL
 REVISED

 PLOT DATE = 10/5/2020
 CHECKED - JIG
 REVISED

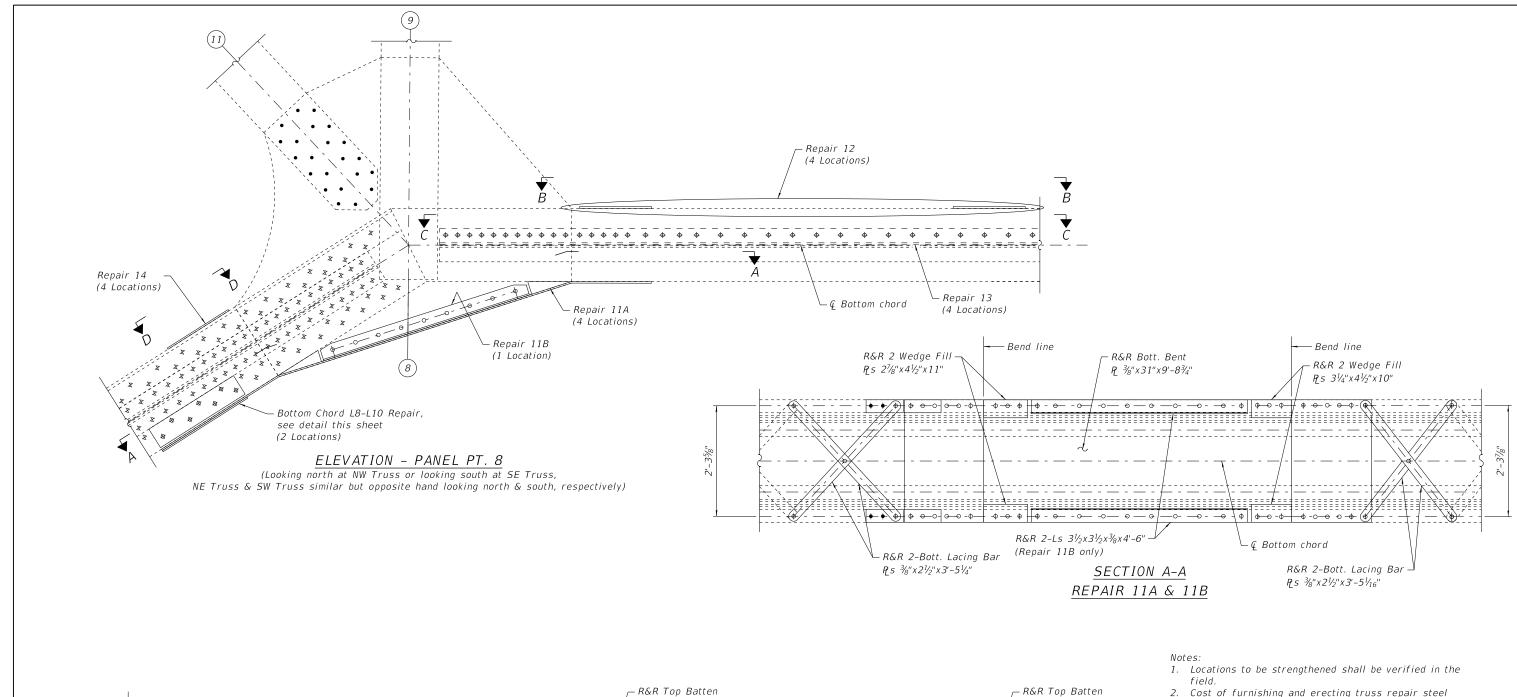
CITY OF CHICAGO

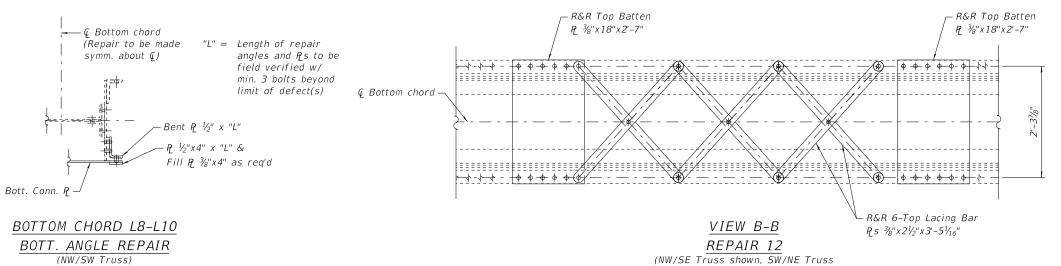
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

similar but opposite hand)

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

BASCULE SPAN: TRUSS PP4 TO PP6 REPAIRS II (STRUCTURE NO. 016-6057)





- 2. Cost of furnishing and erecting truss repair steel elements shall be included in "Structural Steel Repair".
- Cost of removing and replacing rivets in association with furnishing and erecting truss repair steel elements shall be included in "Structural Steel Repair".
- 4. See sheets S-55 thru S-58 for locations of Truss
 Repairs
- 5. See sheet S-63 for Section C-C & View D-D.

REFERENCE DRAWINGS

<u>Drawing</u> Main Truss Members Main Truss Members Posts & Diagonals <u>Sheet No.</u> 1660570200 1660570202 1660570205

WSP USA Inc.
30 N. LASALLE STREET
SUITE 4200
CHICAGO, IL 60602
TEL: (312) 782-8150
FAX: (312) 782-1684

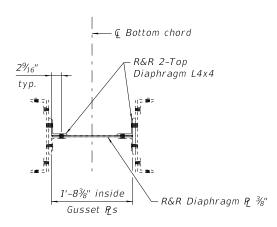
CITY OF CHICAGO

DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

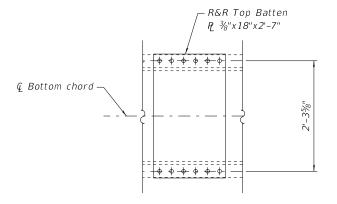
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

similar but opposite hand)

BASCULE SPAN: TRUSS PP8 REPAIRS I (STRUCTURE NO. 016-6057)

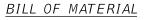


SECTION E-E



<u>VIEW D-D</u> REPAIR 14

(NW Truss shown, SW/NE Truss similar but opposite hand)



Item	Unit	Quantity
Structural Steel	Pound	5,560
Repair		

Notes:

- 1. Locations to be strengthened shall be verified in the field.
- Cost of furnishing and erecting truss repair steel elements shall be included in "Structural Steel Repair".
- Cost of removing and replacing rivets in association with furnishing and erecting truss repair steel elements shall be included in "Structural Steel Repair".
- 5. See sheet S-62 for locations of Section C-C & View D-D.

REFERENCE DRAWINGS

<u>Drawing</u> Main Truss Members Main Truss Members <u>Sheet No.</u> 1660570200 1660570202

	© Bottom chord —	$R\&R$ 2-Diaphragm $L4x4x\frac{3}{8}x15'-5\frac{1}{2}''$	ŗ ≯ E	am 6-6 — -
	⊕ - ⊕ - ⊕ - ⊕ - ⊕ - ⊕	- + - + - + - + + + + + + + + + + + + +	0 0 <th>—</th>	—
	- - -			— - — - — - — -
			\	
 - -		SECTION C-C	R&R 1-Diaphragm R 3%"x20"x15"-51/2"	

WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782–8150 FAX: (312) 782–1684

USER NAME = PJLAUX	DESIGNED - LD/PJL	REVISED -
	CHECKED - NBR	REVISED -
PLOT SCALE = N.T.S.	DRAWN - LD/PJL	REVISED -
PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -

CITY OF CHICAGO

DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

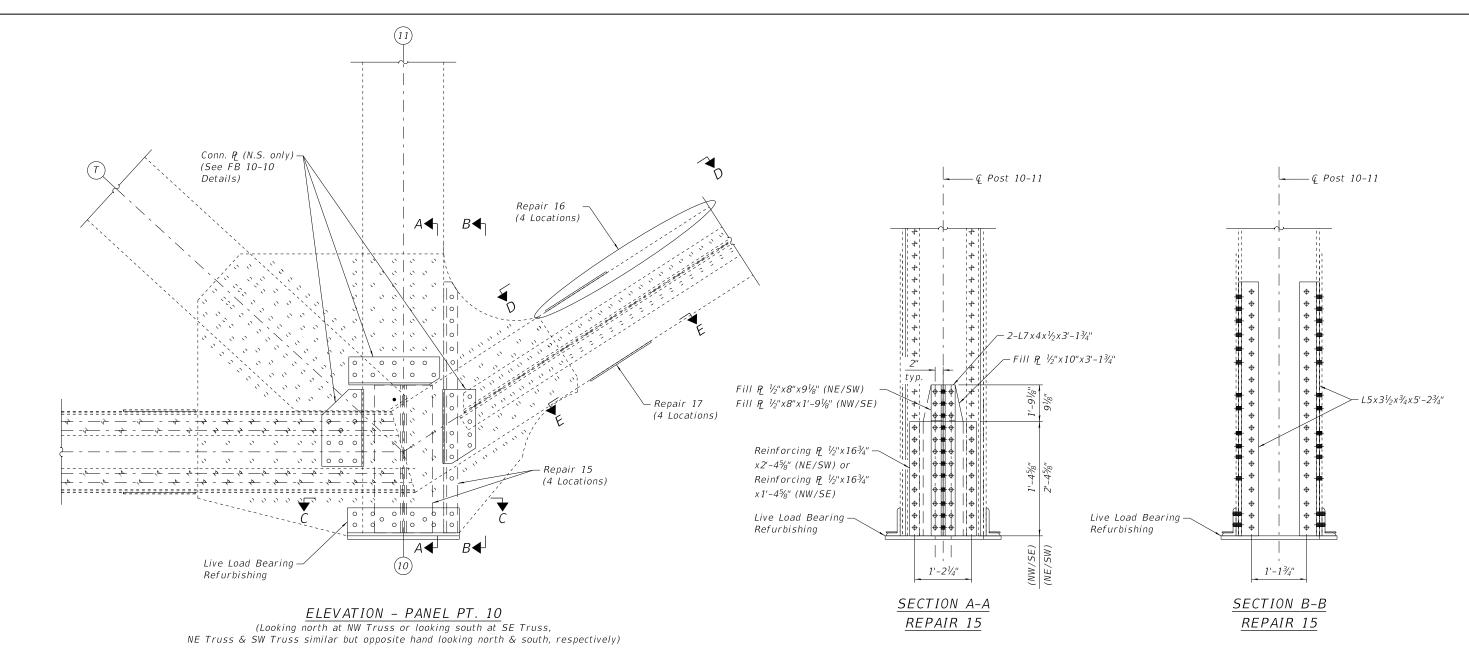
similar but opposite hand)

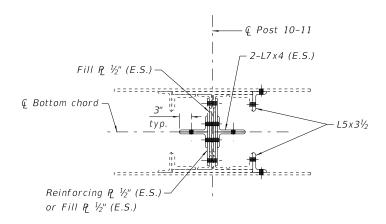
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

BASCULE SPAN: TRUSS PP8 REPAIRS II (STRUCTURE NO. 016-6057) F.A.U. SECTION COUNTY SHEET NO.

1388 11-E1525-00-BR COOK S-63

CDOT PROJECT NO. E-1-525 106 of 210





SECTION C-C

Notes:

- 1. Locations to be strengthened shall be verified in the field.
- Cost of furnishing and erecting truss repair steel elements shall be included in "Structural Steel Repair".
- Cost of removing and replacing rivets in association with furnishing and erecting truss repair steel elements shall be included in "Structural Steel Repair".
- 4. See sheets S-55 thru S-58 for locations of Truss Repairs.
- 5. See sheet S-65 for View D-D and Section E-E.

REFERENCE DRAWINGS

<u>Drawing</u> Main Truss Members Posts & Diagonals <u>Sheet No.</u> 1660570202 1660570205

WSP USA Inc.
30 N. LASALLE STREET
SUITE 4200
CHICAGO, IL 60602

CITY OF CHICAGO

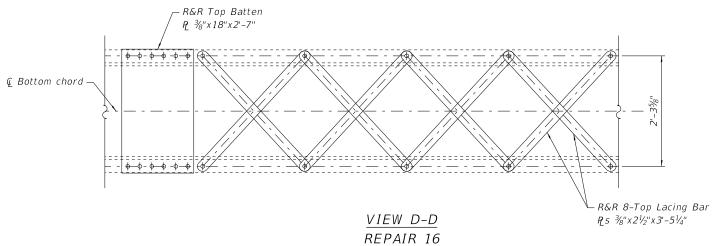
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER
THE NORTH BRANCH CHICAGO RIVER

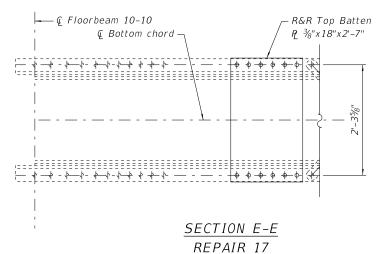
BASCULE SPAN: TRUSS PP10 REPAIRS I (STRUCTURE NO. 016-6057) F.A.U. SECTION COUNTY SHEET NO.

1388 11-E1525-00-BR COOK S-64

CDOT PROJECT NO. E-1-525 107 of 210



(NW/SE Truss shown, SW/NE Truss similar but opposite hand)



(NW/SE Truss shown, SW/NE Truss similar but opposite hand)

BILL OF MATERIAL

Item	Unit	Quantity
Structural Steel	Pound	4,240
Repair		

Notes:

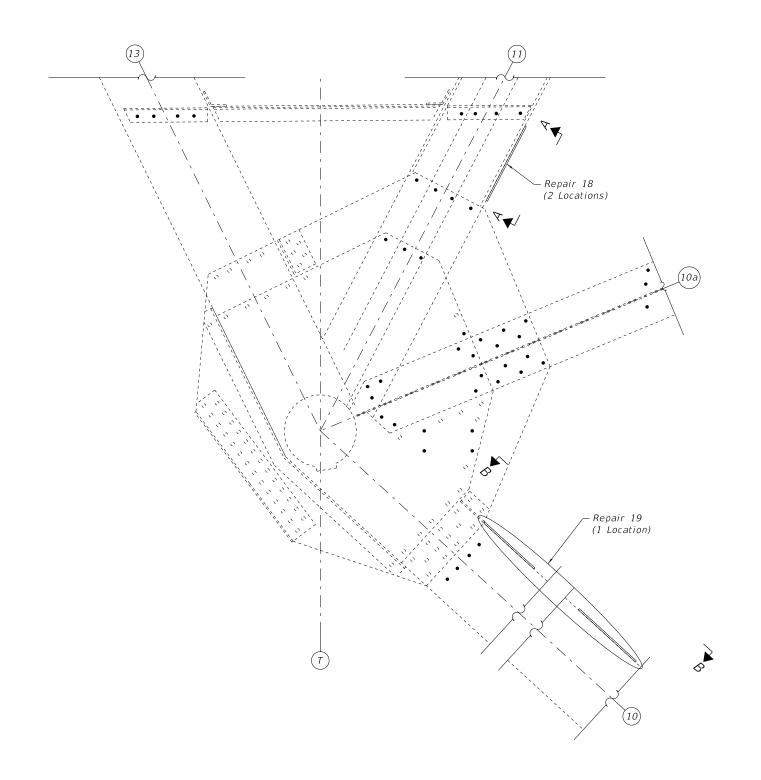
- 1. Locations to be strengthened shall be verified in the field.
- 2. Cost of furnishing and erecting truss repair steel elements shall be included in "Structural Steel Repair".
- Cost of removing and replacing rivets in association with furnishing and erecting truss repair steel elements shall be included in "Structural Steel Repair".
- 5. See sheet S-64 for locations of View D-D & Section E-E.

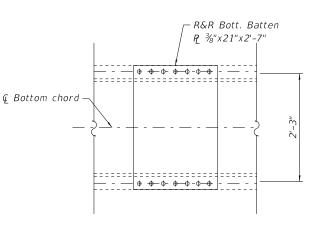
REFERENCE DRAWINGS

<u>Drawing</u> Main Truss Members <u>Sheet No.</u> 1660570202

wsp	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684
-----	---

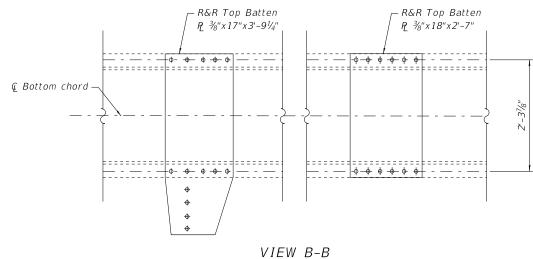
USER NAME = PJLAUX	DESIGNED - LD/PJL	REVISED -
	CHECKED - NBR	REVISED -
PLOT SCALE = N.T.S.	DRAWN - LD/PJL	REVISED -
PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -





<u>VIEW A-A</u> REPAIR 18

(NW Truss shown, SW Truss similar)



Notes:

REPAIR 19
(SW Truss shown)

- 1. Locations to be strengthened shall be verified in the field.
- 2. Cost of furnishing and erecting truss repair steel elements shall be included in "Structural Steel Repair".
- Cost of removing and replacing rivets in association with furnishing and erecting truss repair steel elements shall be included in "Structural Steel Repair".
- 5. See sheets S-55 thru S-58 for locations of Truss Repairs.

ELEVATION - PANEL PT. T

(Looking north at NW Truss or looking south at SE Truss, NE Truss & SW Truss similar but opposite hand looking north & south, respectively)

BILL OF MATERIAL

Item	Unit	Quantity
Structural Steel	Pound	360
Renair		

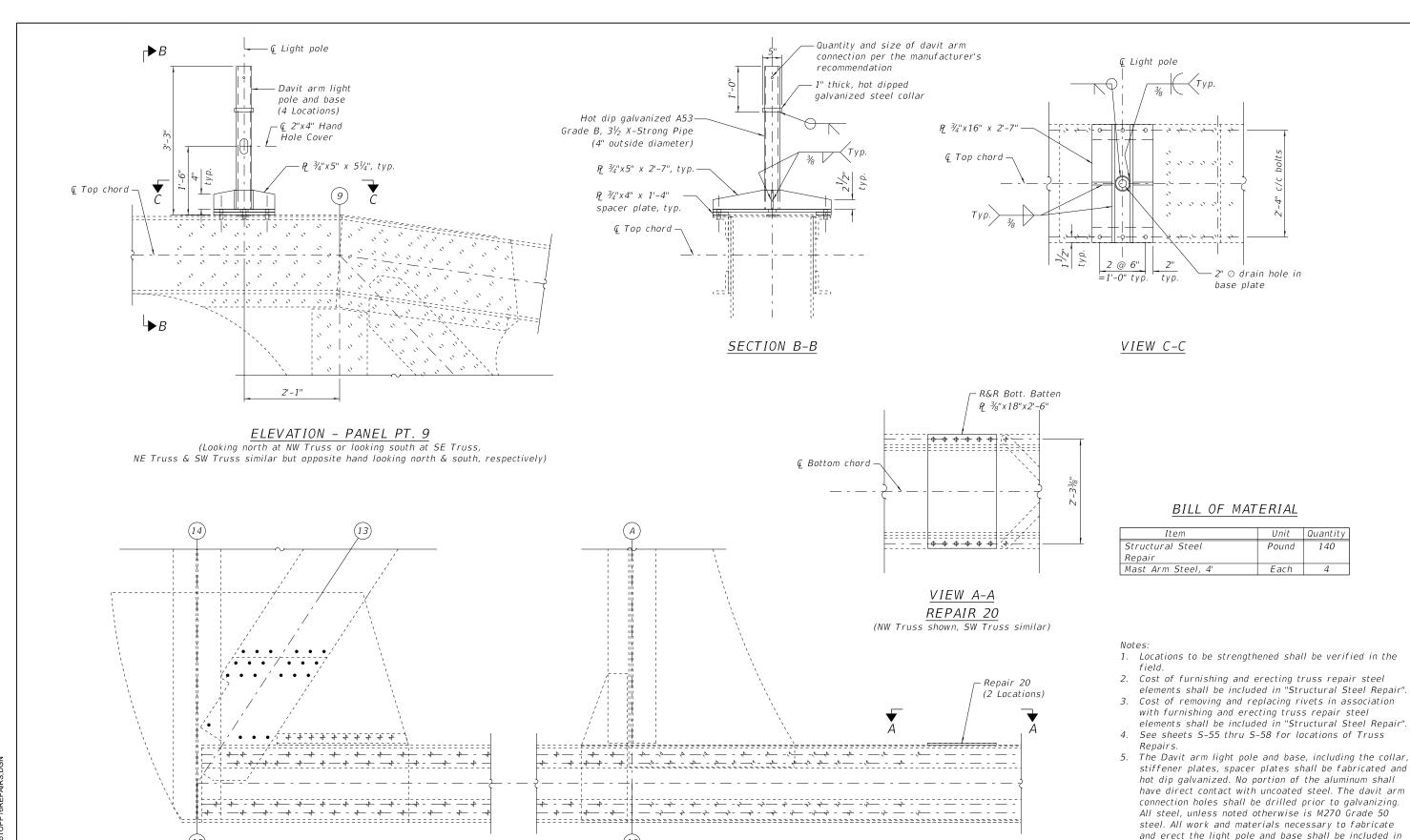
REFERENCE DRAWINGS

<u>Drawing</u> Main Truss Members <u>Sheet No.</u> 1660570199

WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 IEL: (312) 782-8150 FAX: (312) 782-1684

CHECKED - NBR REVISED - PLOT SCALE = N.T.S. DRAWN - LD/PJL REVISED - PLOT DATE = \$DATE\$ CHECKED - JIG REVISED -	USER NAME = PJLAUX	DESIGNED - LD/PJL	REVISED -
		CHECKED - NBR	REVISED -
PLOT DATE = \$DATE\$ CHECKED - JIG REVISED -	PLOT SCALE = N.T.S.	DRAWN - LD/PJL	REVISED -
	PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-66
CDOT	PROJECT NO. E-1-525		109 of 210



ELEVATION - PANEL PT. 15 TO 16

(Looking north at NW Truss or looking south at SE Truss, NE Truss & SW Truss similar but opposite hand looking north & south, respectively)

item Mast Arm Steel, 4'. REFERENCE DRAWINGS

<u>Drawing</u>

Main Truss Members 14-15 Main Truss Members 9-13

Sheet No. 1660570198 1660570201

140

4

USER NAME = PJLAUX DESIGNED - LD/PJL REVISED CHECKED - NBR REVISED PLOT SCALE = N.T.S. - LD/PJL REVISED PLOT DATE = \$DATF\$ CHECKED - JIG REVISED

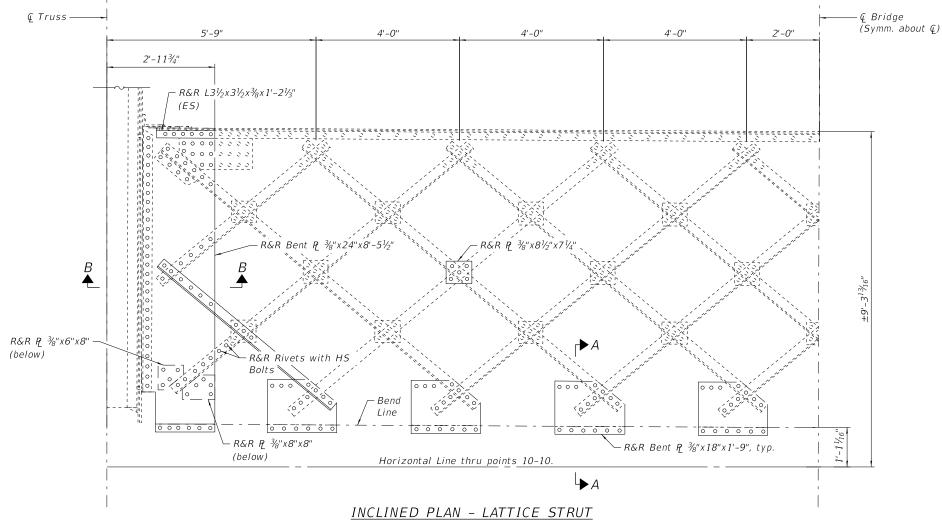
DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

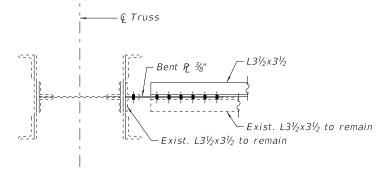
BASCULE SPAN: TRUSS PP9 & PP15 TO PP16 REPAIRS (STRUCTURE NO. 016-6057)

SECTION COUNTY SHEET NO. S-67 11-E1525-00-BR COOK CDOT PROJECT NO. E-1-525 110 of 210

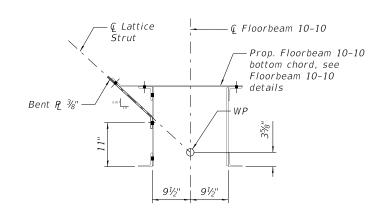








SECTION B-B



SECTION A-A

BILL OF MATERIAL

Item	Unit	Quantity
Structural Steel Repairs	Pound	2,820
Removal of Deteriorated Connectors and	Each	8
Replacement with High Strength Bolts		

Quantity shown includes West and East Leaf.

REFERENCE DRAWINGS

Drawing Latticed Strut and Horizontal Girder Horizontal Girder and Bracing Bracing for Floorbeam at 10-10

Sheet No. 1660570013 1660570208 1660570210

- 1. Locations to be repaired shall be verified in the field.
- 2. Cost of furnishing and erecting lattice strut steel repairs and removing and replacing rivets associated with these elements shall be included in the cost of Structural Steel Repairs.
- 3. Cost of removing and replacing rivets that are not associated with Structural Steel Repairs shall be included in Removal of Deteriorated Connectors and Replacement with High Strength Bolts.

USER NAME = RALMASRI DESIGNED - RAM REVISED CHECKED - IJL REVISED PLOT SCALE = N.T.S. RAM REVISED PLOT DATE = \$DATE\$ CHECKED - JIG REVISED

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

BASCULE SPAN: LATTICE STRUT REPAIR DETAILS (STRUCTURE NO. 016-6057)

COUNTY SHEET NO. SECTION S-68 11-E1525-00-BR соок CDOT PROJECT NO. E-1-525 111 of 210

INTERIOR ST	TRINGER	MOMENT T	ABLE
		0.5 FB	-to-FB
		Spa	cing
		W16x57	W21x57
Is	(in⁴)	758	1170
Ic(n)	(in⁴)	-	-
Ic(3n)	(in⁴)	-	-
Ic(cr)	(in⁴)	=	-
Ss	(in³)	92.2	111
Sc(n)	(in³)	=	=
Sc(3n)	(in³)	-	-
Sc(cr)	(in³)	-	-
DC 1	(k/')	0.30	0.30
MDC1	('k)	8.9	10.3
DC2	(k/')	-	-
MDC2	('k)	-	-
DW	(k/')	-	-
MDW	('k)	-	-
LLDF		0.44	0.44
MŁ + IM	('k)	132.9	78.4
Mu (Strength I)	('k)	243.7	150.1
Øf Mn	('k)	-	-
fs DC1	(ksi)	1.2	1.1
fs DC2	(ksi)	-	-
fs DW	(ksi)	_	_
fs (Ł+IM)	(ksi)	17.3	8.5
fs (Service II)	(ksi)	23.6	12.1
0.80Rh Fyf	(ksi)	40.0	40.0
fs (Total)(Strength		31.7	16.2
Øf Fn	(ksi)	47.4	50.0
Vf	(k)	98.6	80.2

		0.5 C-to-C Trusses		
		Floorbeam 0-0 Floorbeam 2		
			(typ.)	
Is	(in⁴)	22,670	26,109	
Ic(n)	(in⁴)	ı	1	
Ic(3n)	(in⁴)	ı	1	
Ic(cr)	(in⁴)	ı	ı	
Ss	(in³)	819	916	
Sc(n)	(in³)	ı	ı	
Sc(3n)	(in³)	ı	ı	
Sc(cr)	(in³)	ı	ı	
DC1	(k/')	1.06	0.88	
MDC1	('k)	208	200	
DC2	(k/')	-	-	
MDC2	('k)	-	-	
DW	(k/')	-	-	
MDW	('k)	-	-	
LLDF		-	-	
M4 + IM	('k)	1224	1024	
Mu (Strength I)	('k)	2522	2041	
Øf Mn	('k)	-	-	
fs DC1	(ksi)	3.1	2.6	
fs DC2	(ksi)	-	-	
fs DW	(ksi)	-	-	
fs (½+IM)	(ksi)	17.9	13.4	
fs (Service II)	(ksi)	27.7	20.1	
0.80Rh Fyf	(ksi)	40.0	40.0	
fs (Total)(Strength	I) (ksi)	37.0	26.7	
Øf Fn	(ksi)	50.0	46.7	
Vf	(k)	43.8	36.7	

FLOORBEAM MOMENT TABLE

STRINGER REACTION TABLE							
		W 16	W16x57 W21x57				
		FB 10-10	FB 6-6				
			(typ.)				
R₽	(k)	4.0	2.3	2.5			
R4	(k)	<i>37.3</i>	29.2				
Rı	(k)	22.8 8.7		8.7			
R _{Total}	(k)	64.1	40.4				

FLC	FLOORBEAM REACTION TABLE							
		FB 0-0	FB 2-2					
			(typ.)					
R₽	(k)	21.0	18.8					
R4	(k)	70.4	73.2					
Rı	(k)	41.2	19.0					
R _{Total}	(k)	132.6	111.5					

- Is. Ss: Non-composite moment of inertia and section modulus of the steel section used for computing fs(Total-Strength I, and Service II) due to non-composite dead loads (in.4 and in.3).
- Ic(n), Sc(n): Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing fs(Total-Strength I, and Service II) in uncracked sections due to short-term composite live loads (in.4 and in.3).
- Ic(3n), Sc(3n): Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing fs(Total-Strength I, and Service II) in uncracked sections, due to long-term composite (superimposed) dead loads (in.⁴ and in.³).
- Ic(cr), Sc(cr): Composite moment of inertia and section modulus of the steel and longitudinal deck reinforcement, used for computing fs (Total-Strength I and Service II) in cracked sections, due to both short-term composite live loads and long-term composite (superimposed) dead loads (in.4 and in.3).
 - DC1: Un-factored non-composite dead load (kips/ft.).
 - MDC1: Un-factored moment due to non-composite dead load (kip-ft.). DC2: Un-factored long-term composite (superimposed excluding future wearing surface) dead load (kips/ft.).
 - MDC2: Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip-ft.).
 - DW: Un-factored long-term composite (superimposed future wearing surface only) dead load (kips/ft.).
 - MDW: Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip-ft.).
 - M_L + IM: Un-factored live load moment plus dynamic load allowance (impact) (kip-ft.).
- Mu (Strength I): Factored design moment (kip-ft.). $1.25 (MDC1 + MDC2) + 1.5 MDW + 1.75 M_{L+IM}$
 - Of Mn: Compact composite positive moment capacity computed according to Article 6.10.7.1 or non-slender negative moment capacity according to Article A6.1.1 or A6.1.2 (kip-ft).
 - fs DC1: Un-factored stress at edge of flange for controlling steel flange due to vertical non-composite dead loads as calculated below (ksi). MDC1/ Snc
 - fs DC2: Un-factored stress at edge of flange for controlling steel flange due to vertical composite dead loads as calculated
 - MDC2/ Sc(3n) or MDC2/ Sc(cr) as applicable.
 - fs DW: Un-factored stress at edge of flange for controlling steel flange due to vertical composite future wearing surface loads as calculated below (ksi). MDW/ Sc(3n) or MDW/ Sc(cr) as applicable.
 - fs (4+IM): Un-factored stress at edge of flange for controlling steel flange due to vertical composite live load plus impact loads as calculated below (ksi).
- $M_{4} + IM / Sc(n)$ or $M_{4} + IM / Sc(cr)$ as applicable. fs (Service II): Sum of stresses as computed below (ksi).
- $fsDC1 + fsDC2 + fsDW + 1.3 fs(_{L+IM})$ 0.80RhFyf: Non-Composite stress capacity for Service II loading according to Article 6.10.4.2 (ksi).
- fs (Total)(Strength I): Sum of stresses as computed below on non-compact section (ksi).

 - Øf Fn: Non-Compact composite positive or negative stress capacity for Strength I loading according to Article 6.10.7 or 6.10.8 (ksi).
 - Vf: Maximum factored shear range in span computed according to Article 6.10.10.

USER NAME = PATELN	DESIGNED - NJP	REVISED -
	CHECKED - PJL	REVISED -
PLOT SCALE = N.T.S.	DRAWN - NJP	REVISED -
PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -

LEAF REMOVAL ITEMS

	Weight	X	Υ	Mx	My
Item	(Lbs)	(ft)	(ft)	(Lb-ft)	(Lb-ft)
Jack Beams	-10,930	50.25	5.92	-549,230	-64,710
* 5" Rdwy Open Grid	-59,290	50.48	6.31	-2,992,960	-374,120
Center Break	-4,500	93.71	6.53	-421,700	-29,390
Concrete Fill in Rear Break	-5,080	7.00	5.82	-35,560	-29,570
Curb Plate w/checkered PL	-12,940	50.04	7.04	-647,520	-91,120
* 2½" Sidewalk Grid including Conc.	-55,780	50.70	7.09	-2,828,050	-395,480
Floorbeam 0-0	-13,030	93.49	3.98	-1,218,170	-51,860
Floorbeam 2-2	-16,000	76.79	3.85	-1,228,640	-61,600
Floorbeam 4-4	-15,670	60.09	3.72	-941,610	-58,290
Floorbeam 6-6	-16,130	43.40	3.59	-700,040	-57,910
Floorbeam 8-8	-16,610	26.70	3.46	-443,490	-57,470
Floorbeam 10-10 & Horiz. Girder	-22,510	9.88	-3.57	-222,510	80,440
Roadway Stringers	-65,390	49.91	5.17	-3,263,610	-338,070
S. Sdwk Brackets at FB 10-10 (W. Leaf only)	-710	10.00	4.06	-7,100	-2,880
Sidewalk Laterals	-60	51.75	5.71	-3,110	-340
Auxiliary Locks	-1,000	87.63	8.74	-87,630	-8,740
Center Lock (W. Leaf)	-4,190	88.35	2.63	-370,200	-11,030
Center Lock (E. Leaf)	-1,220	93.49	2.09	-114,060	-2,550
Center Lock Platform (W. Leaf)	-470	87.00	2.00	-40,890	-940
Center Lock Platform (E. Leaf)	-240	84.12	2.00	-20,190	-480
Total W. Leaf	-320,290			-16,002,020	-1,544,140
Total E. Leaf	-316,380			-15,718,080	-1,550,200

LEAF ADDITION ITEMS

	Weight	Χ	Y	Mx	Му
Item	(Lbs)	(ft)	(ft)	(Lb-ft)	(Lb-ft)
FRP Grating	16,030	50.56	7.10	810,480	113,810
Sdwk Center Break	494	93.50	6.65	46,220	3,290
Sdwk Rear Break	374	6.33	5.62	2,370	2,100
Rdwy. Open Grid Deck	4,230	48.08	6.29	203,390	26,610
Rdwy. Conc. Filled Deck	160,390	50.12	6.39	8,038,710	1,024,890
Rdwy. Center Break	3,080	93.50	6.65	287,980	20,480
Rdwy. Rear Break	3,160	6.33	5.62	20,000	17,760
Curb Plate w/Checkered PL	16,860	49.56	6.95	835,580	117,180
Floorbeam 0-0	6,800	93.50	3.89	635,800	26,450
Floorbeam 2-2	7,550	76.80	3.66	579,840	27,630
Floorbeam 4-4	7,560	60.10	3.50	454,360	26,460
Floorbeam 6-6	7,590	43.40	3.33	329,410	25,270
Floorbeam 8-8	7,590	26.70	3.17	202,650	24,060
Floorbeam 10-10 & Horiz. Girder	20,120	10.00	3.13	201,200	62,980
Roadway Stringers	46,710	50.05	5.23	2,337,840	244,290
S. Sdwk Brackets at FB 10-10 (W. Leaf only)	710	10.00	4.06	7,100	2,880
Sidewalk Laterals	1,604	51.75	5.71	83,010	9,160
Roadway Laterals	18,728	60.00	1.97	1,123,660	36,860
Center Lock (W. Leaf)	3,390	91.63	2.76	310,610	9,340
Center Lock (E. Leaf)	770	93.49	2.76	71,990	2,120
Center Lock Platform (W. Leaf)	590	87.00	2.00	51,330	1,180
Center Lock Platform (E. Leaf)	300	84.12	2.00	25,240	600
Light Pole	1,000	7.98	23.21	7,980	23,210
Total W. Leaf	334,560			16,569,520	1,845,890
Total E. Leaf	330,940			16,297,710	1,835,210

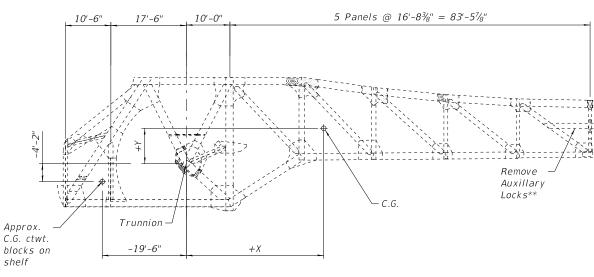
SUMMARY

	Weight	Χ	Y	Мx	Мy
Item	(Lbs)	(ft)	(ft)	(Lb-ft)	(Lb-ft)
Table 1: Total W. Leaf Removal	-320,290	49.96	4.82	-16,002,020	-1,544,140
Table 2: Total W. Leaf Addition	334,560	49.53	5.52	16,569,520	1,845,890
Net Change	14,270			567,500	301,750
Table 1: Total E. Leaf Removal	-316,380	49.68	4.90	-15,718,080	-1,550,200
Table 2: Total E. Leaf Addition	330,940	49.25	5.55	16,297,710	1,835,210
Net Change	14,560			579,630	285,010
East Ctwt. Adjustment	13,350	-19.50	-4.17	-260,330	-55,630
(30 Blocks Btwn. A1 & B1)					
West Ctwt. Adjustment	13,350	-19.50	-4.17	-260,330	-55,630
(30 Blocks Btwn. A2 & B2)					
NET TOTAL W. LEAF	27,620			307,170	246,120
NET TOTAL E. LEAF	27,910			319,300	229,380

* The following unit weights are used in the balancing calculation: Existing Sidewalk Grid Deck = 33.4 psf Existing Roadway Open Grid Deck = 19.0 psf Proposed Sidewalk Grid Deck = 10.0 psfProposed Roadway Open Grid Deck = 23.2 psf (includes allowance for coatings, boltdown plate and accessories)

Proposed Roadway Conc. Filled Deck = 53.1 psf

** Cost of removal of Auxilliary Locks included in Balancing of Bridge and Alteration of Counterweights. Auxiliary Locks shall become property of the City.



TRUSS ELEVATION

- 1. It is assumed the bridge is balanced.
- 2. Weight denotes the weight of member element replacement and not necessarily the total weight of member and reflect final balancing at the completion of the proposed rehabilitation. Items to be replaced in-kind are not included in the balance conditions.
- 3. The distances X and Y are measured from the center of trunnion. see
- 4. The contractor shall submit to the Commissioner for approval bridge balancing calculations as detailed in the Special Provisions. The Cost shall be included in Balancing of Bridge and Alteration of Counterweights.
- 5. Balancing of the movable leaves will be paid for as Balancing of Bridge and Alteration of Counterweights. Fabrication and installation of new counterweight blocks (if reg'd) and shimming and adjusting the anchor columns and live load bearings as required to properly balance the bridge is included in the Cost of Balancing of Bridge and Alteration of Counterweights. The Contractor shall make all adjustments and relocations necessary to attain the partially or fully balanced condition to the satisfaction of the Commissioner.
- All dimensions and elevations shown are based on existing drawings. Space available in counterweight pockets and steel racks based on existing drawings. The Contractor shall verify.

REFERENCE DRAWINGS

Counterweight Calculations Counterweight Balance

Sheet No. 1660570109 1660570128

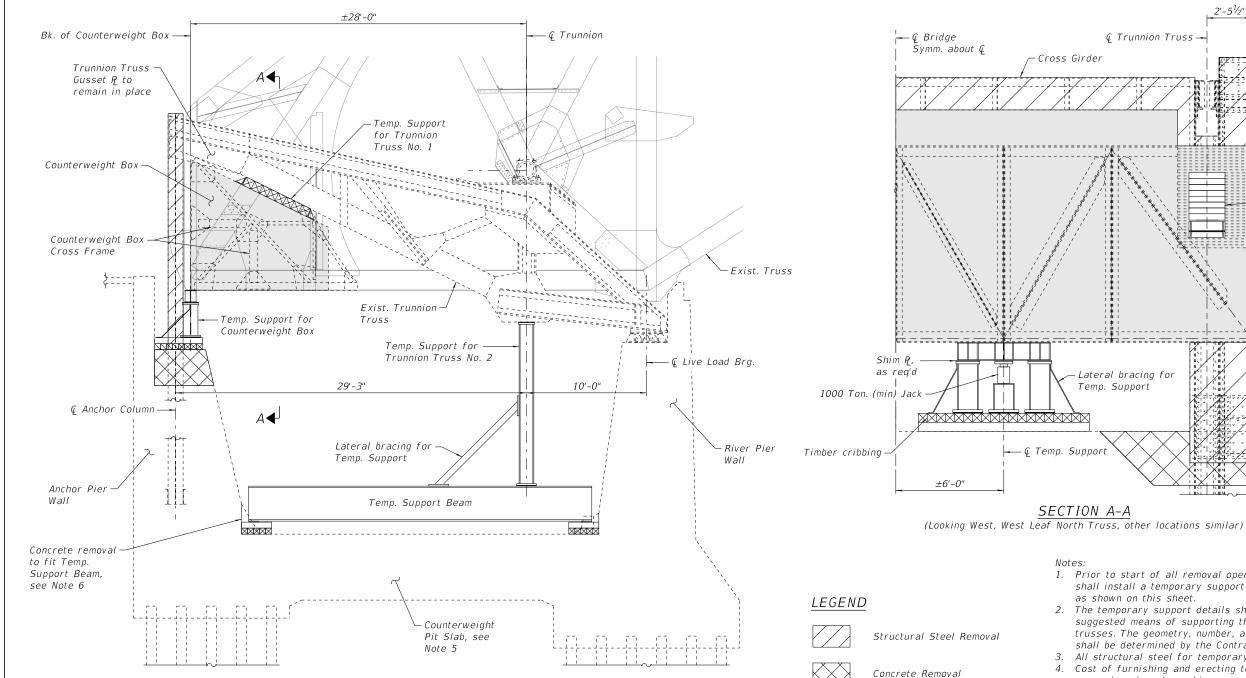
USER NAME = RALMASRI DESIGNED - RAM REVISED CHECKED - PJL REVISED REVISED PLOT DATE = \$DATE\$ CHECKED - JIG REVISED

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

BASCULE SPAN BRIDGE BALANCING (STRUCTURE NO. 016-6057)

SECTION COUNTY SHEET NO. S-70 11-E1525-00-BR соок CDOT PROJECT NO. E-1-525 113 of 210



SECTION THRU COUNTERWEIGHT PIT AT TRUNNION TRUSS

(Looking North, West Leaf North Truss, other locations similar. Longitudinal Girder not shown for clarity)

SUGGESTED WORK PLAN

- A. Remove fixed span concrete deck and superstructure steel framing. Anchor column and cross girder between columns are to remain in place at this stage. Temporary support of sidewalk may be required to maintain pedestrian access. See sheet S-6, for fixed span removal details.
- B. Install temporary support for Counterweight Box. The temporary support shall provide means for adjusting height as work progresses.
- Remove Bascule Span deck and steel framing. Maintain pedestrian access at all times. See sheet S-7 for bascule span removal details. Remove center lock. Maintain auxiliary center lock in place until new center lock is installed. New center lock shall be delivered to 31st and Sacramento and installed by IHC.
- D. Refurbish Live Load Bearing as detailed on sheet S-72. Adjustments may be required on counterweight box temporary support to obtain a workable gap in live load bearing to perform work as detailed on sheet S-72.
- E. Remove trunnion bearing caps prior to any jacking to avoid damaging studs.
- Jack counterweight box to relieve load at Trunnion and full contact is achieved at the Live Load Bearing. Install strain gauge at Trunnion Shafts prior to jacking counterweight box and monitor throughout the operation such that strain limits are not exceeded. Install shims at Temporary Support to transfer load to temporary support frame. Verify full contact at Live Load Bearing remains. See Special Provision for Temporary Support and Mechanical Specifications.
- Install and secure in place temporary supports for trunnion trusses. Trunnion truss may need to be stiffened locally at location of temporary supports.
- H. Remove and replace Anchor Columns and Cross Girders. Extreme care shall be taken to not damage the Trunnion Truss gusset plate connection to anchor column. Gusset plate is to remain in place. See sheets S-73 thru S-75 for Anchor Column replacement details.
- Repair Trunnion Truss and Trunnion Truss Bracing. See sheets S-76 thru S-77 for repair details.
- Repair Longitudinal Girder. See sheet S-78 and S-79 for repair details.
- K. Remove Trunnion Truss Temp. Supports.
- L. Reattach trunning bearing caps.

Lateral bracing for

Temp. Support

1. Prior to start of all removal operations in the bascule span, the Contractor shall install a temporary support for the counterweight boxes on each leaf as shown on this sheet.

 $2'-5^{1/2}$ " $2'-5^{1/2}$ "

- @ Anchor Column

Temp. Support for Trunnion Truss

C Longitudinal Girder

– Long. Girder

to remain in

place

← Trunnion Truss →

- 2. The temporary support details shown on this sheet are to be taken as a suggested means of supporting the counterweight boxes and trunnion trusses. The geometry, number, and location of temporary support points shall be determined by the Contractor.
- 3. All structural steel for temporary supports shall be AASHTO M270 Grade 50.
- 4. Cost of furnishing and erecting temporary supports, equipment, concrete removal, and workmanship necessary to temporary support the counterweights and the trunnion trusses shall be included in the cost of Temporary Support. See Special Provisions.
- 5. Placing temporary supports directly on the counterweight pit slab is not recommended, except as shown herein. If Contractor elects to place supports directly on the slab, calculations and details prepared, signed, and sealed by an Illinois Licensed Structural Engineer must be submitted and approved supporting the feasibility of the proposed temporary support.
- Extreme care shall be taken during concrete removal operations as not to damage the existing reinforcement. Any reinforcement bars that are damaged during concrete removal operations shall be repaired or replaced using an approved bar splicer or anchorage system. Cost included with Temporary Support.

REFERENCE DRAWINGS

NETENCE DIAWINGS	
<u>Drawing</u>	Sheet No.
Sub and Superstructure, Fixed part,	
Anchor Columns etc.	1660570020
Substructure Main Piers	1660570045
Main Truss Members	1660570197
Main Truss Members	1660570198
Repairs to Anchor Columns	1660570120
CWT Box Plates & Cross Frames	1660570211

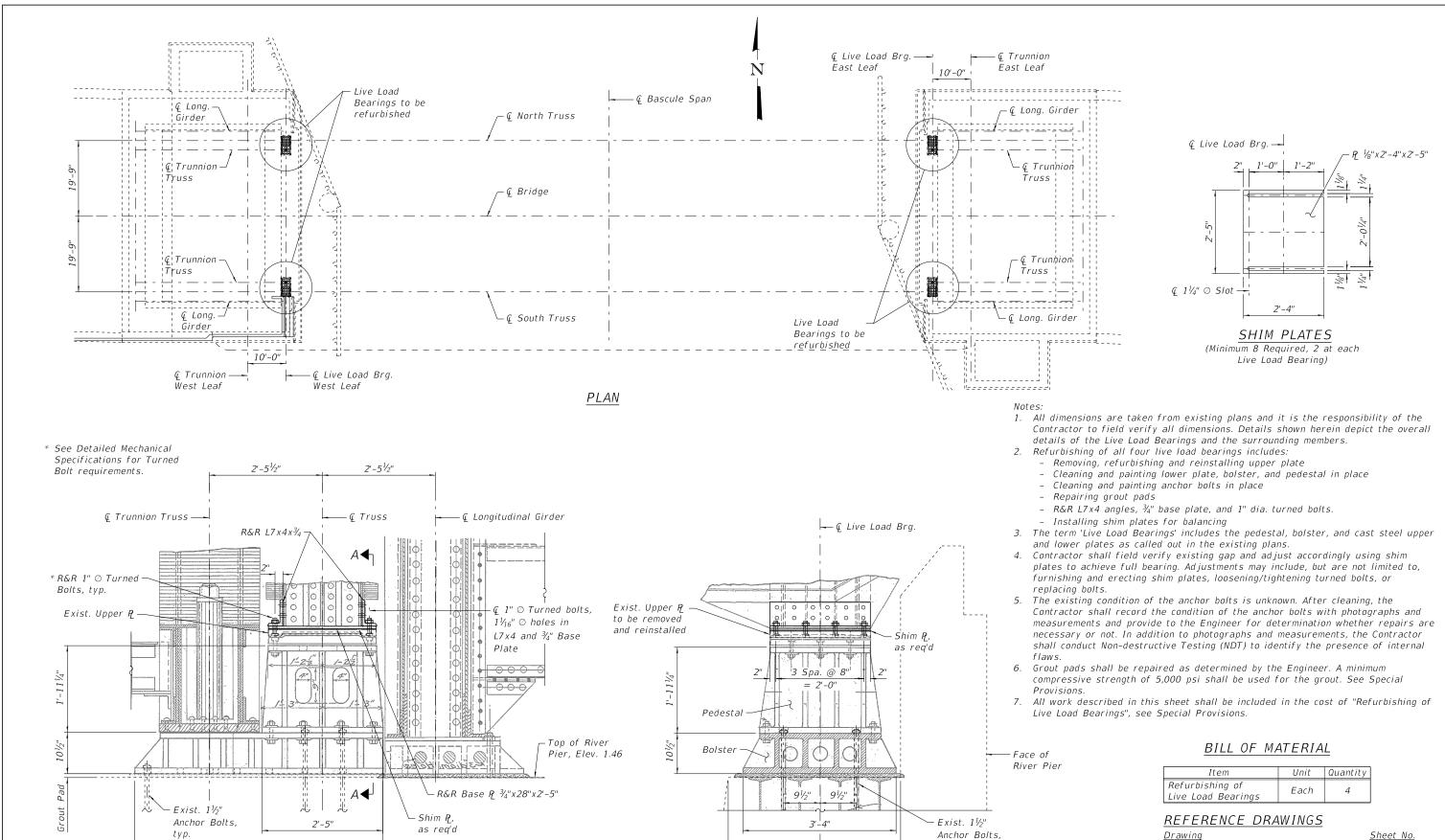
USER NAME = IJLOPEZ DESIGNED - IJL REVISED CHECKED - NBR REVISED - IJL REVISED PLOT DATE = \$DATF\$ CHECKED - JIG REVISED

DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER SUGGESTED WORK PLAN AND TEMPORARY SUPPORT DETAIL AT COUNTERWEIGHT PIT (STRUCTURE NO. 016-6057)

SECTION COUNTY SHEET NO. S-71 11-E1525-00-BR СООК CDOT PROJECT NO. E-1-525 114 of 210

CITY OF CHICAGO



ELEVATION	- LIVE	LOAD	BEARING

(Southwest Live Load Bearing shown, looking east, others similar)

5'-41/5"

SECTION A-A

2'-0"

USER NAME = PJLAUX DESIGNED - RA/IJL REVISED CHECKED - PJI REVISED RA/IJL REVISED PLOT DATE = \$DATE\$ CHECKED - JIG REVISED

DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

1'-9"

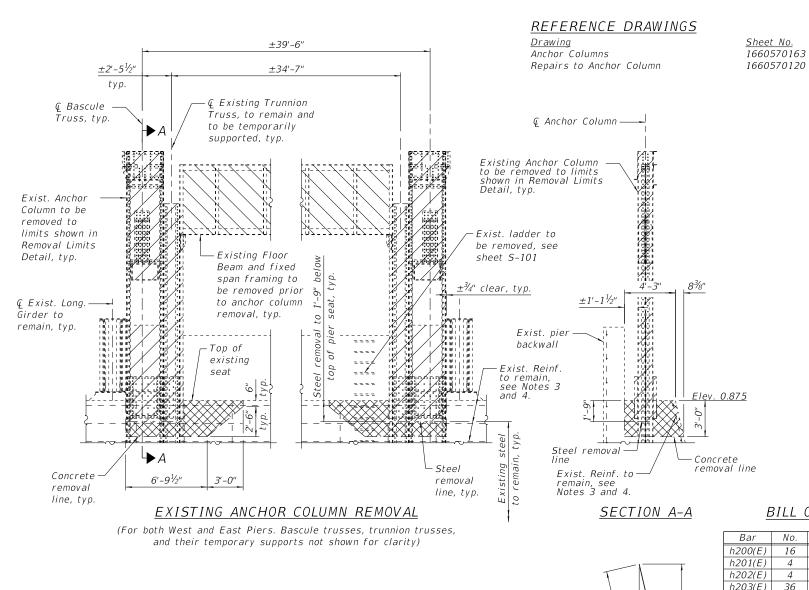
typ.

LIVE LOAD BEARING REFURBISHING

SECTION COUNTY SHEET NO. S-72 соок 11-E1525-00-BR CDOT PROJECT NO. E-1-525 115 of 210

Steel Castings
Iron Castings
Bolts & Washers
Trunnion Trusses
Main Piers
Posts & Diagonals

CITY OF CHICAGO



← Trunnion Truss

±1½"

Grind smooth to the final steel

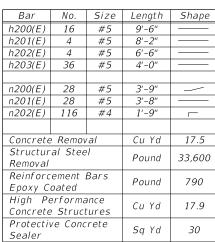
removal line for new column to

Existing Anchor

Column Retrofit

Elev. -0.875

BILL OF MATERIAL



Quantities shown include all four locations.

BAR n202(E)

LEGEND





BAR n200(E)

Note A: Cut anchor column above final removal line, and remove column. After column removal, perform final cut and grind steel to remain to the final

cut line

Elev. 0.875, Top-

Bolts, ±4'-0" long

of existing

concrete

REMOVAL LIMITS DETAIL

 $\pm 2' - 5^{1/2''}$

Final cut 3" below exist.

repair plates, typ.

 ← Longitudinal

Concrete removal line.

Do not undermine

bearing area or

anchorage

Longitudinal Girder

Girder

Exist. Casting for

Longitudinal Girder

(Southwest and Northeast Anchor Columns shown, Northwest and Southeast Columns opposite hand) (Longitudinal Girder not shown for clarity)

 $\pm 2' - 5^{1/2''}$

USER NAME = NBROMAN DESIGNED - NBR REVISED CHECKED - IJI REVISED PLOT SCALE = N.T.S DRAWN REVISED PLOT DATE = \$DATF\$ CHECKED REVISED

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

Superstructures

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER **ANCHOR COLUMN REPLACEMENT DETAILS I** (STRUCTURE NO. 016-6057)

SECTION COUNTY SHEET NO. S-73 11-E1525-00-BR СООК CDOT PROJECT NO. E-1-525 116 of 210

5. Grind any nicks, gouges, and shallow

surface cracks after cutting anchor

steel removal line for cracks using

magnetic particle testing. Any cracks

column. Cost included with Structural

The Contractor shall exercise extreme

damage to existing longitudinal girder,

care during removal operations to prevent

trunnion truss, portions of anchor column

to remain in place, and concrete pier. Any

remain in place shall be repaired at the

damage to any of the existing members to

found shall be brought up to the Engineer

prior to installation of proposed anchor

column. Inspect ground surfaces along

* Drill and grout bars according to Article

*4-#4 n202

E) bars, align

with h200(E)

Exist reinf to

bars E.F.

*2-#4 n202(E,

remain, typ.

584 of the Standard Specifications, with a minimum embedment of 9". Cost included with Reinforcement Bars. Epoxy Coated.

9'-91/2"

4-#4 h200(E) bars

spaced as shown in

Section B-B

PIER SEAT RECONSTRUCTION AT ANCHOR COLUMN

*10-#4 n202(E) bars @ 12" cts., B.F.

9-#5 h203(E) bars @ 12" cts

*** 5'-0"

slope 2%

bar

har

1-#5 h201(E)

- 1-#5 h202(E)

*7-#4 n202(E) bars @ 12" cts., B.F.

*7-#5 n200(E) bars @ 12" cts., F.F.

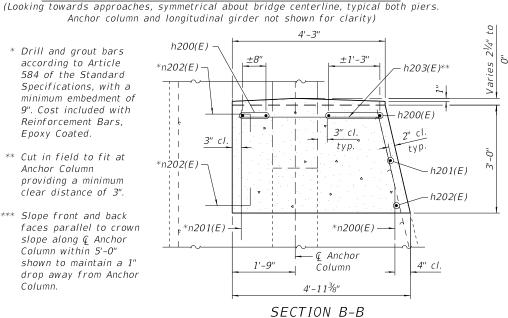
*7-#5 n201(E) bars @ 12" cts., B.F.

6'-91/2"

 $\rightarrow B$

** Cut in field to fit at Anchor Column providing a minimum clear distance of 3".

*** Slope front and back faces parallel to crown slope along & Anchor Column within 5'-0" shown to maintain a 1" drop away from Anchor Column.



Steel Repair.

Contractor's expense

Top of Exist.

h200(E)

*4-#4 n202(E)

bars, align with

Pier Seat

*2-#5 n200(E) bars @ 12" cts., F.F.

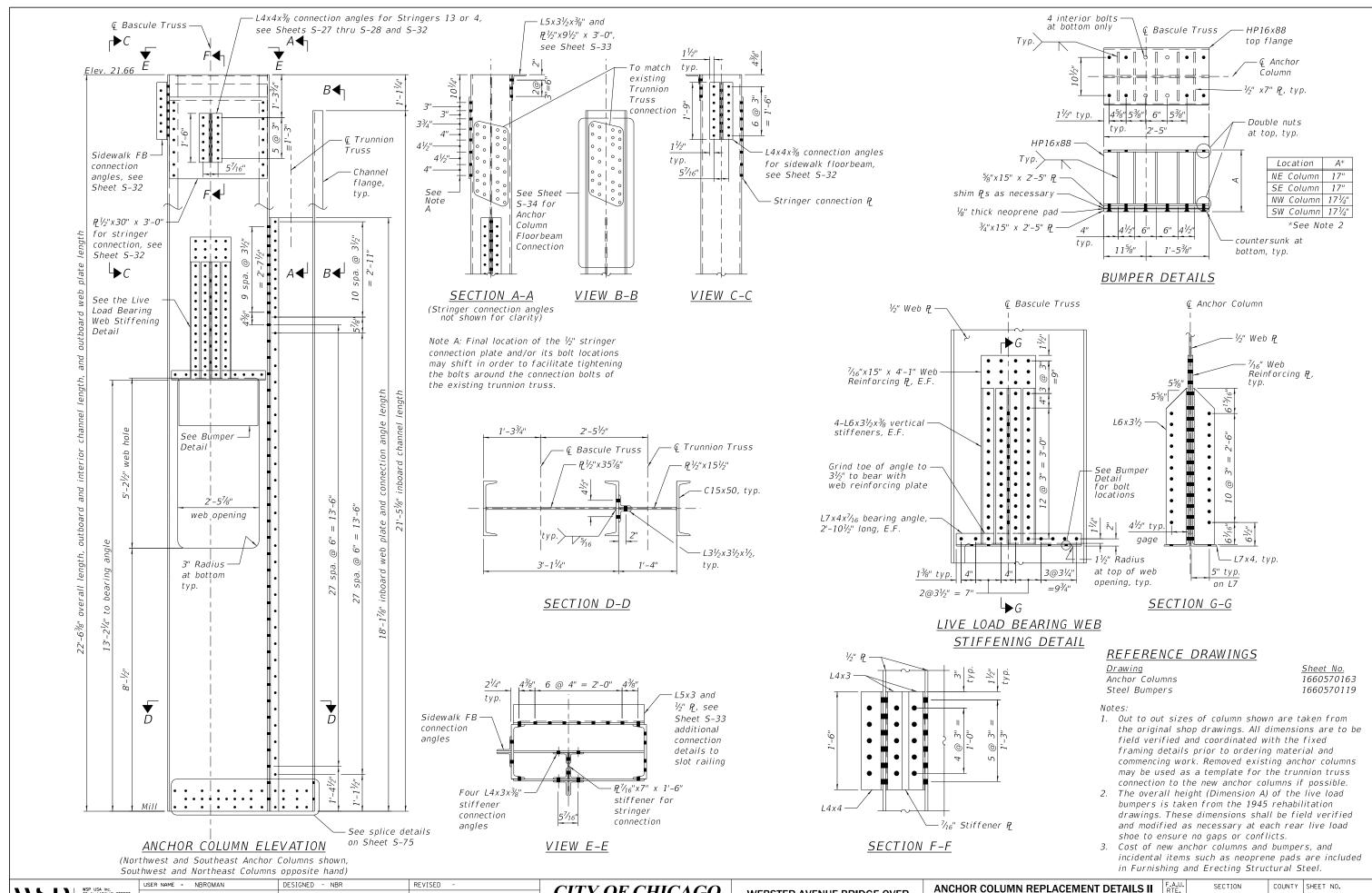
*2-#5 n201(E) bars @ 12" cts., B.F.

(Cut bars in field to fit)

Elev. 0.875

- from the original shop drawings and from the 1945 repair drawings to the anchor columns. All dimensions are to be field verified prior to ordering material and commencing work.
- 4. Existing reinforcement shall be cleaned and incorporated into the new construction. Cost included with Concrete Removal.

- 1. All sizes and dimensions shown are taken
- See sheets S-74 and S-75 for new anchor column and splice details
- Any existing reinforcement bars that are damaged during concrete removal operations shall be repaired or replaced using an approved bar splicer or anchorage system. Cost considered incidental to Concrete Removal.



0166057-E1525-S071-ANCHORCC

WSP USA Inc.
30 N. LASALLE STREET
SUITE 4200.
CHICACO, II. 60602
TEL: (312) 782–8150
FAX: (312) 782–1684

CITY OF CHICAGO

DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

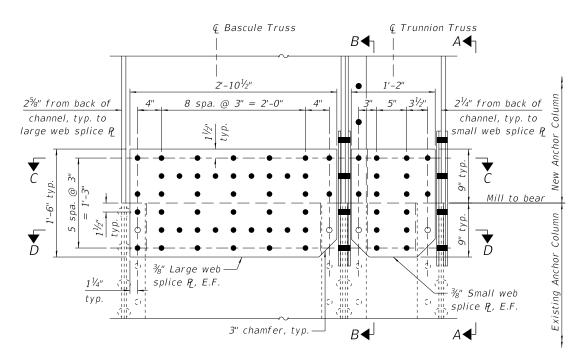
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

(STRUCTURE NO. 016-6057)

F.A.U. SECTION COUNTY SHEET NO.

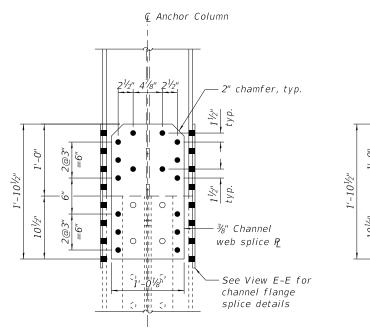
1388 11-E1525-00-BR COOK S-74

CDOT PROJECT NO. E-1-525 117 of 210



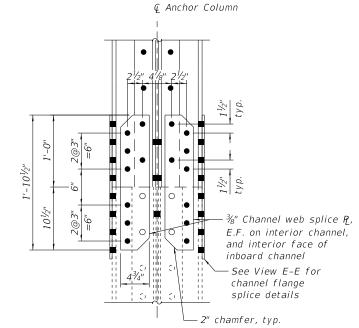
ANCHOR COLUMN SPLICE ELEVATION

(Northwest and Southeast Anchor Columns shown, Southwest and Northeast Columns opposite hand) (Channel flanges not shown for clarity. For fill plates, see Sections C-C and D-D)

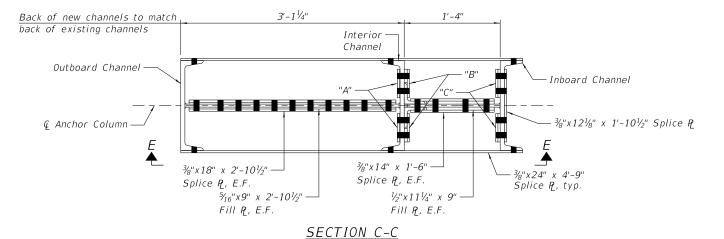


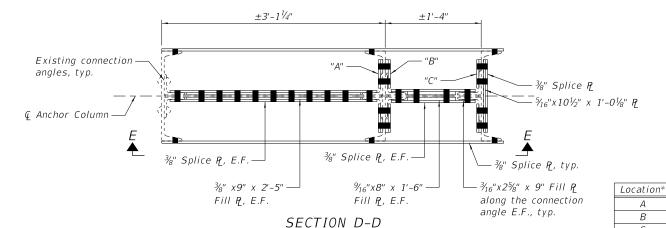
SECTION A-A

 $2^{1}/_{4}$ " from back of channel



SECTION B-B





to bolt line, typ. typ.

- ¾" Splice ₽ E.F.

VIEW E-E

Existing Section Fill P

3%"x21/4"x 101/2

 $\frac{1}{2}$ "x2 $\frac{1}{4}$ " x 10 $\frac{1}{2}$ and $\frac{1}{8}$ "x2"x 10 $\frac{1}{2}$ "

- 1. All dimensions are to be field verified and coordinated with the fixed framing details prior to ordering material and commencing work.
- 2. Bolting clearances are tight, and a careful tightening sequence should be followed in addition to careful bolt orientation to avoid conflicts when placing bolts adjacent to bolts already placed.
- 3. Cost of new splice plates, fill plates, and incidental items such as locating and drilling holes in existing steel are included in Furnishing and Erecting Structural Steel.

С		¾"x4¾" x	1'-0"		3/8" x 13/4" x 1	101/2
* The plates		d C locations	are sym	nmetrical	about	

New Section Fill P

 $\frac{1}{16}$ " $\times 4\frac{3}{4}$ " $\times 1'-0$ "

½"x2½" x 1'-0"

Splice P

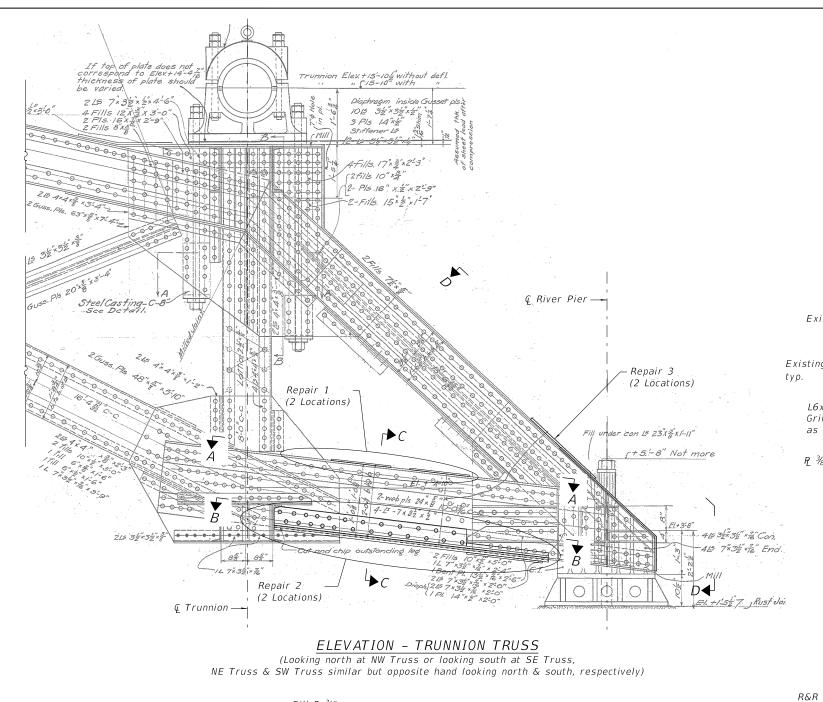
 $\frac{3}{8}$ " x $4\frac{3}{4}$ " x $1'-10\frac{1}{2}$ "

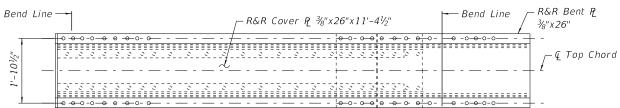
wsp	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684
-----	---

USER NAME = NBROMAN	DESIGNED - NBR	REVISED -
	CHECKED - IJL	REVISED -
PLOT SCALE = N.T.S.	DRAWN - NBR	REVISED -
PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -

В

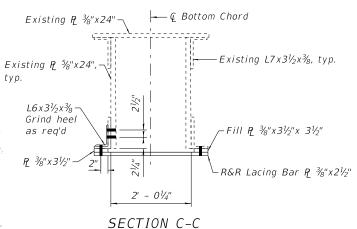
I REPLACEMENT DETAILS III	F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
IDE NO. 040 0057)	1388	11-E1525-00-BR	соок	S-75
JRE NO. 016-6057)	CDOT	PROJECT NO. E-1-525		118 of 210





VIEW D-D REPAIR 3

(Southwest & Northwest Trunnion Truss)



BILL OF MATERIAL

Item	Unit	Quantity
Structural Steel	Pound	2,540
Repairs		

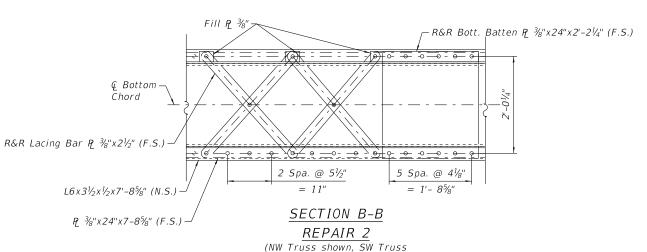
Notes:

- 1. Locations to be strengthened shall be verifed in the field.
- 2. Cost of furnishing and erecting trunnion truss repair steel elements shall be included in Structural Steel Repairs.
- 2. Cost of removing and replacing rivets in association with furnishing and erecting trunnion truss repair steel elements shall be included in Structural Steel Repairs.

REFERENCE DRAWINGS

<u>Drawing</u> Fixed Part- Trunnion Truss Trunnion Truss

Sheet No. 1660570017 1660570190



similar but opposite hand)

R&R Top Batten P 3/8" x 15" x 2'-21/4" ______ @ Bottom -Chord

SECTION A-A REPAIR 1

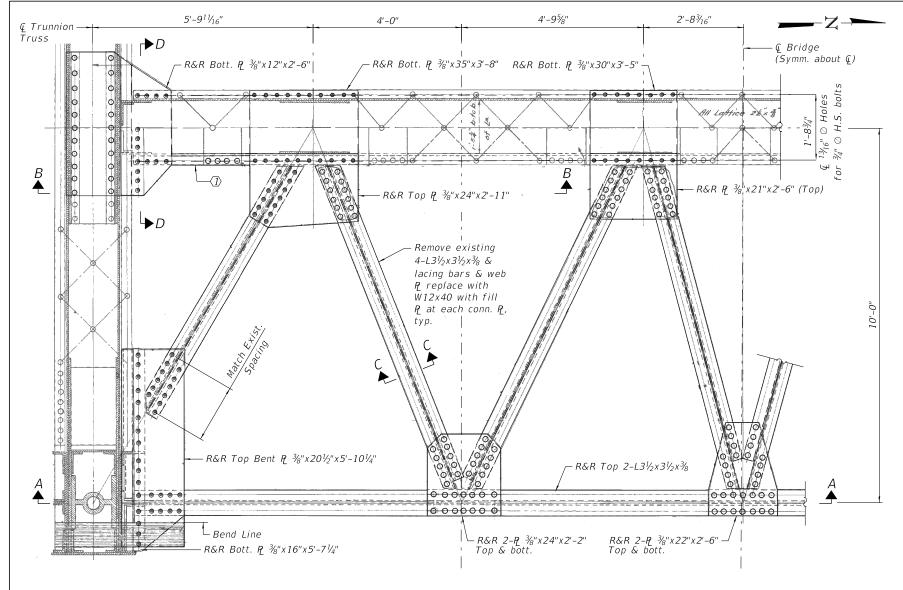
(NW Truss shown, SW Truss similar but opposite hand)

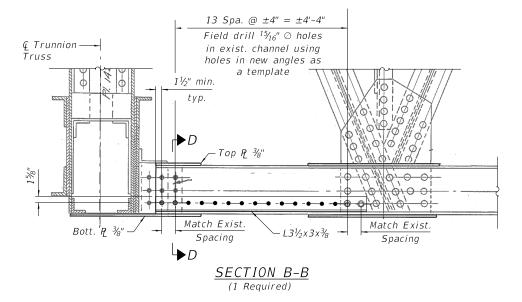
USER NAME = RALMASRI DESIGNED - RAM REVISED CHECKED - PJL REVISED PLOT SCALE = N.T.S. RAM REVISED PLOT DATE = \$DATF\$ CHECKED - JIG REVISED

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER TRUNNION TRUSS REPAIR DETAILS (STRUCTURE NO. 016-6057)

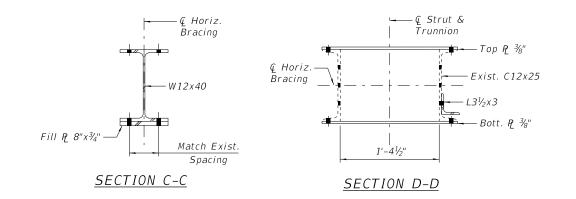
F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-76
CDOT	PROJECT NO. E-1-525		119 of 210

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING



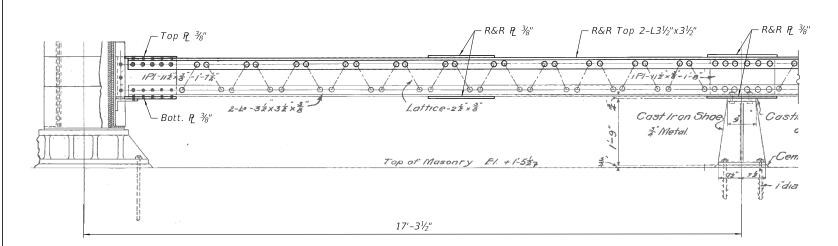


 $\langle \overline{I} \rangle$ Add L3½x3x¾ to exisitng C12 flange, west leaf only at SW trunnion Truss.



PLAN OF HORIZONTAL BRACING BETWEEN TRUNNION TRUSSES

(West Leaf shown, East Leaf similar)



SECTION A-A

REFERENCE DRAWINGS

Drawing
Structural Work in Rehabilitation
of Horizontal Truss West Side
Structural Work in Rehabilitation
of Horizontal Truss West Side
Bracing for Trunnion Truss
Trunnion Truss1660570190
Bracing for Trunnion Truss
Fixed Part-Trussed Bracing
Between Trunnion Trusses

1660570132 1660570193

1660570133

Sheet No.

or Trunnion Truss 1660570194 t-Trussed Bracing Trunnion Trusses 1660570194

Notes:

- 1. Cost of furnishing and erecting Trunnion Truss Lateral Bracing Steel Elements and replacing rivets associated with these elements shall be included in the cost of Structural Steel Repairs.
- 2. Cost of removing and replacing rivets that are not associated with furnishing and erecting steel elements shall be included in Removal of Deteriorated Connectors and Replacement with High Strength Bolts.

BILL OF MATERIAL

Item	Unit	Quantity
Structural Steel Repairs	Pound	36,830

WSP USA In 30 N. LASAI SUITE 4200 CHICAGO, IL TEL: (312) FAX: (312)

 USER NAME
 RALMASRI
 DESIGNED
 - RAM
 REVISED

 CHECKED
 - IJL
 REVISED

 PLOT SCALE = N.T.S.
 DRAWN
 - RAM
 REVISED

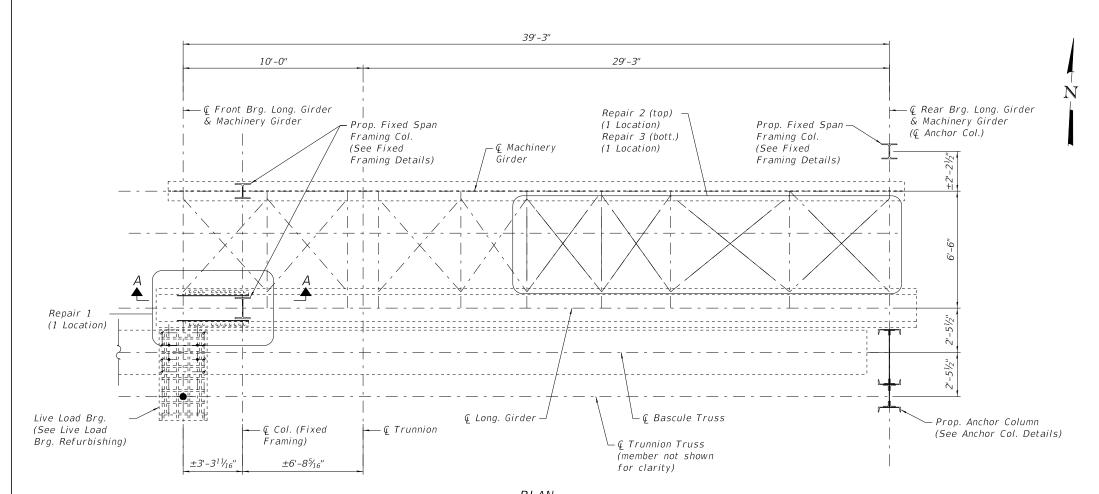
 PLOT DATE = \$DATE\$
 CHECKED
 - JIG
 REVISED

CITY OF CHICAGO

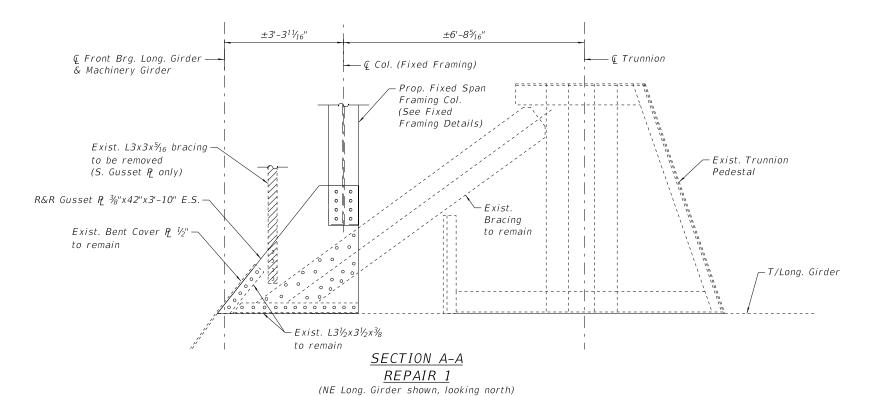
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER
THE NORTH BRANCH CHICAGO RIVER

TRUNNION TRUSS BRACING REPAIR DETAILS
(STRUCTURE NO. 016-6057)



PLAN
(NE Long. Girder & NE Machinery Girder shown, other locations similar)



Notes

- 1. Locations to be strengthened shall be verified in the field.
- 2. Cost of furnishing and erecting long, girder and machinery girder repair steel elements shall be included in Structural Steel Repairs.
- 3. Cost of removing and replacing rivets in association with furnishing and erecting long, girder and machinery girder repair steel elements shall be included in Structural Steel Repairs.
- 4. See sheet S-79 for Repair 2 & 3 Details.

REFERENCE DRAWINGS

Drawing
Fixed Part - Anchor Columns, Etc.

Machinery Girders
Machinery Girders
Bracing for Machinery Girders
Trunnion Pedestal and Strut

Sheet No.
1660570020
1660570188
1660570188
1660570191
1660570192

WSP USA Inc.
30 N. LASALLE STREET
SUITE 4200C
CHICAGO, IL 60602
TEL: (312) 782-8150
FAX: (312) 782-1884

 USER NAME = RALMASRI
 DESIGNED - PUL
 REVISED

 CHECKED - RAM
 REVISED

 PLOT SCALE = N.T.S.
 DRAWN - PUL
 REVISED

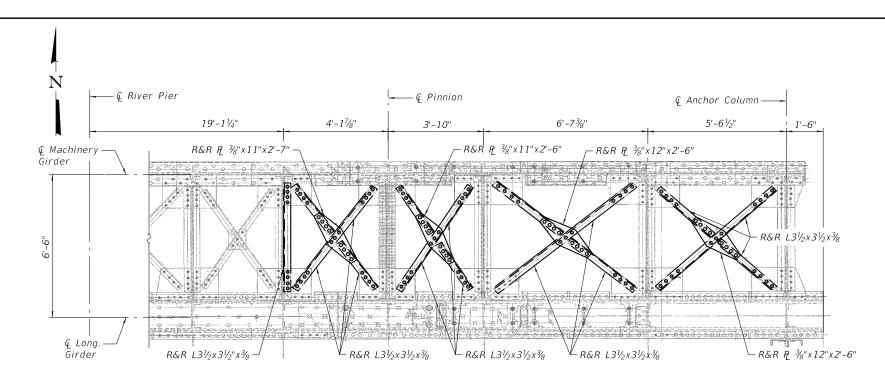
 PLOT DATE = \$DATE\$
 CHECKED - JIG
 REVISED

CITY OF CHICAGO

DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

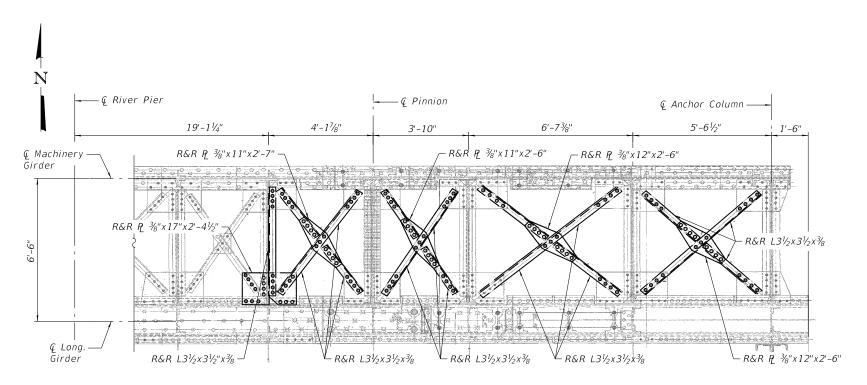
WEBSTER AVENUE BRIDGE OVER
THE NORTH BRANCH CHICAGO RIVER

LONGITUDINAL GIRDER & MACHINERY GIRDER REPAIRS I (STRUCTURE NO. 016-6057)



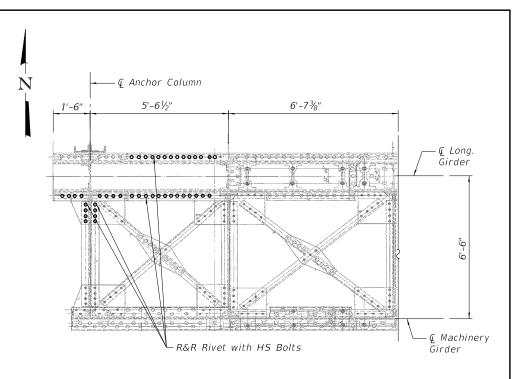
<u>PARTIAL PLAN</u> REPAIR 2

(NE Long. Girder & Machinary Girder, Top Bracing shown)



<u>PARTIAL PLAN</u> <u>REPAIR 3</u>

(NE Long. Girder & Machinary Girder, Bott. Bracing shown)



PARTIAL PLAN REPAIR 4

(SE Long. Girder & Machinary Girder, Top Bracing shown)

BILL OF MATERIAL

Item	Unit	Quantity
Structural Steel	D	1.510
Repairs	Pound	1,510
Removal of		
Deteriorated		
Connectors and	Each	43
Replacement with High		
Strength Bolts		
~		

REFERENCE DRAWINGS

<u>Drawing</u> Fixed Part-Longitudinal and Machinery Girder Trunnion Pedestal and Strut <u>Sheet No.</u> 1660570019

1660570192

Notes:

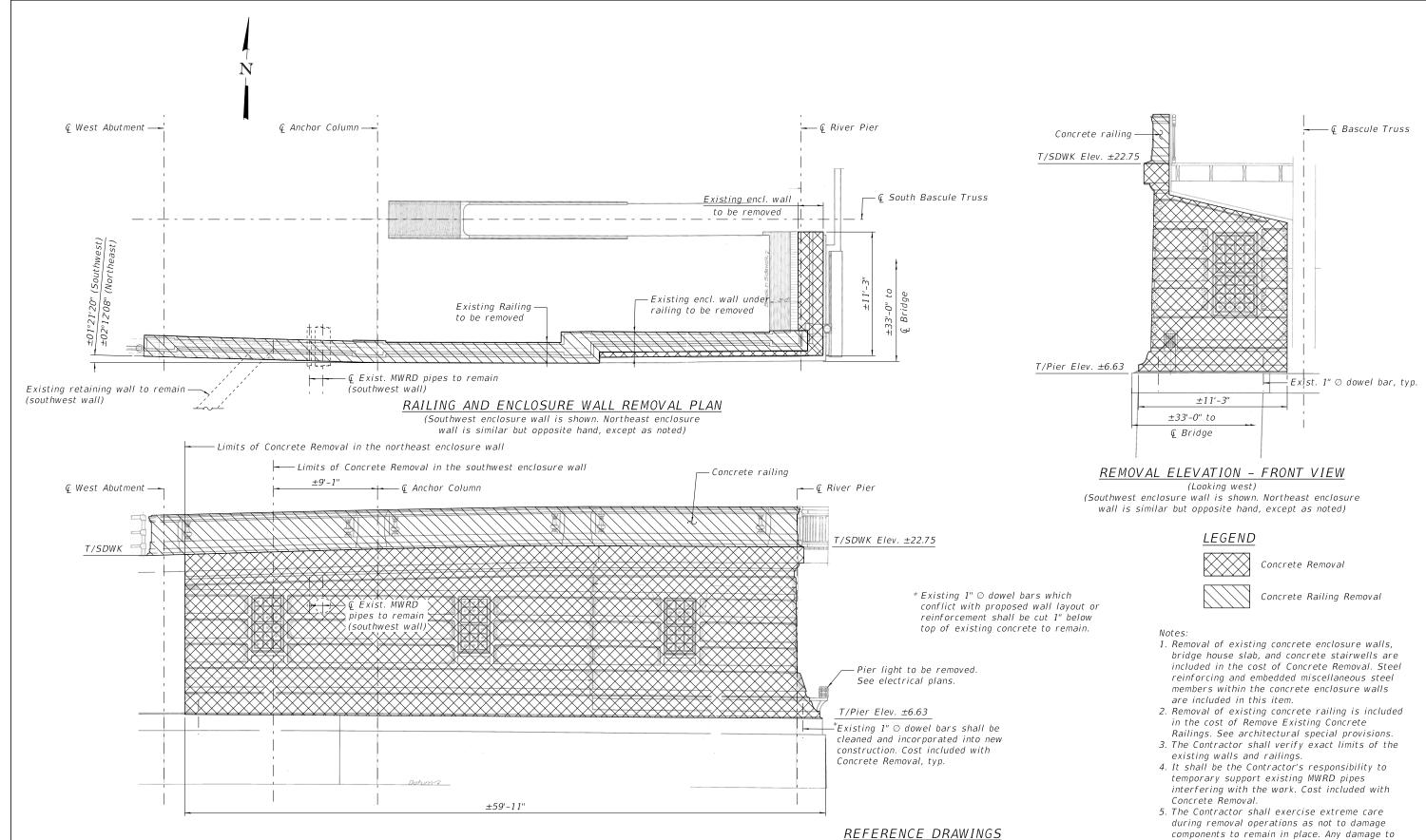
- 1. Locations to be strengthened shall be verified in the field.
- Cost of furnishing and erecting long. girder and machinery girder repair steel elements shall be included in Structural Steel Repairs.
- 3. Cost of removing and replacing rivets in association with furnishing and erecting long, girder and machinery girder repair steel elements shall be included in Structral Steel Repairs.

wsp	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684
-----	---

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

LONGITUDINAL GIRDER & MACHINERY GIRDER REPAIRS II (STRUCTURE NO. 016-6057)

F.A.U. SECTION		COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-79
CDOT	PROJECT NO. E-1-525		122 of 210



0166057-E1525-S080-ENCL.WALL_RE

WSP USA Inc. 30 N. ASALLE STREET SUIF, 200 GHICAGO, It 60602 TEL: (312) 782-8150 FAX: (312) 782-1684

CITY OF CHICAGO

DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

<u>REMOVAL ELEVATION - SIDE VIEW</u>

(Looking north)

(Southwest enclosure wall is shown. Northeast enclosure

wall is similar but opposite hand, except as noted)

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

<u>Drawing</u>

Operator's House & Enclosure Walls

Substructure Main Piers

ENCLOSURE WALLS: REMOVAL DETAILS I (STRUCTURE NO. 016-6057)

<u>Sheet No.</u>

1660570036

1660570037

1660570038

1660570045

F.A.U. SECTION COUNTY SHEET NO.

1388 11-E1525-00-BR COOK S-80

CDOT PROJECT NO. E-1-525 123 of 210

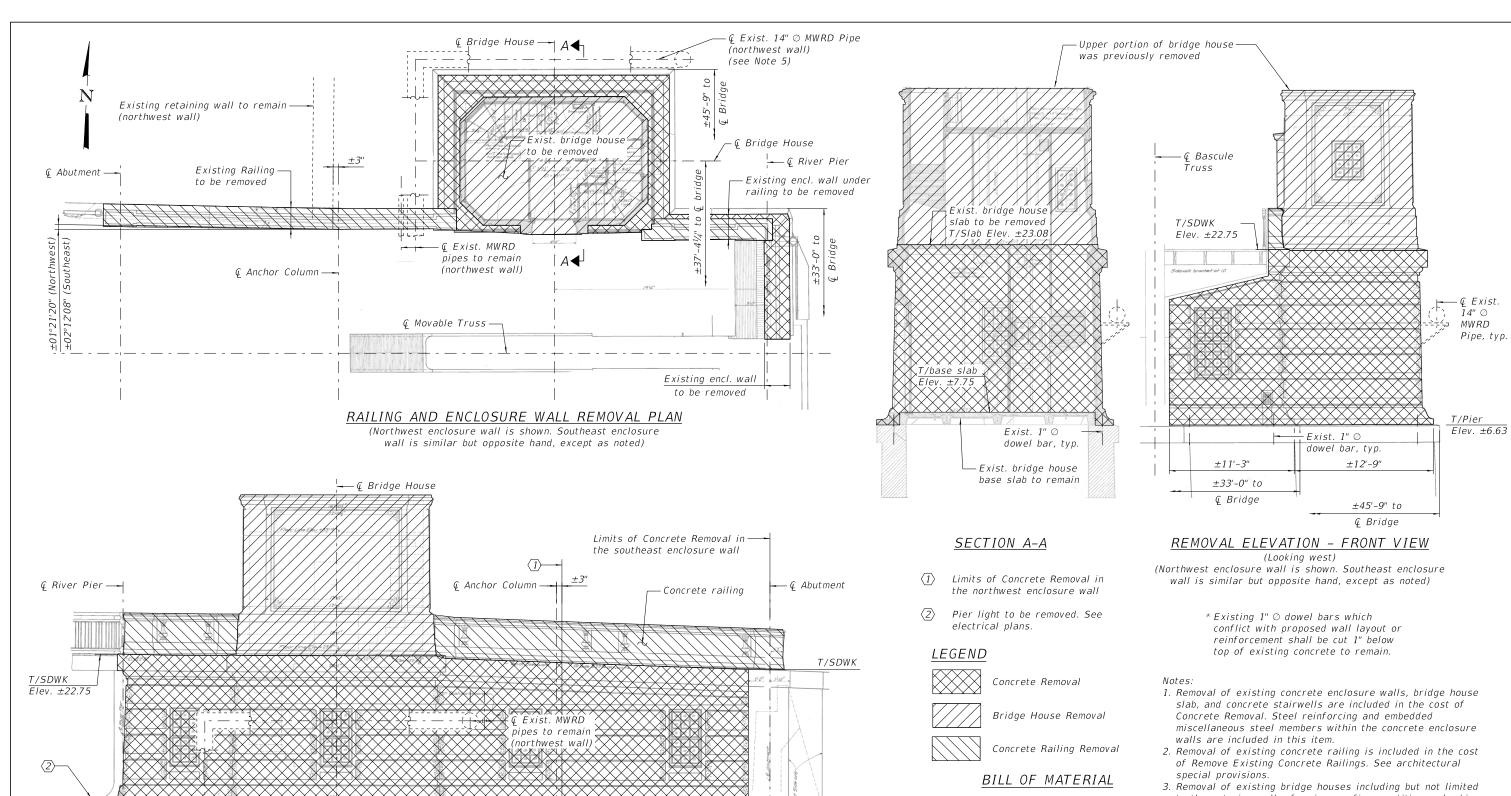
portions that are to remain in place shall be

6. See sheets S-6 and S-7 for fixed and bascule

satisfaction of the Commissioner.

span removal details.

repaired at the Contractor's expense and to the



Item	Unit	Quantity
Concrete Removal	Cu. Yd.	184.1

REFERENCE DRAWINGS <u>Drawing</u> Operator's House & Enclosure Walls

Substructure Main Piers

Sheet No. 1660570036 1660570037 1660570038 1660570045

- to the exterior walls, framing, roofing, partitions, plumbing, and electrical within the limits shown is included in Remove Existing Bridge Houses. See architectural special provisions.
- 4. The Contractor shall verify exact limits of the existing walls and railings.
- 5. It shall be the Contractor's responsibility to temporary support existing MWRD pipes interfering with the work. Cost included with Concrete Removal.
- 6. The Contractor shall exercise extreme care during removal operations as not to damage components to remain in place. Any damage to portions that are to remain in place shall be repaired at the Contractor's expense and to the satisfaction of the Commissioner
- 7. See sheets S-6 and S-7 for fixed and bascule span removal details.

USER NAME = IBRAHIMM DESIGNED - MI REVISED CHECKED - PJL REVISED PLOT SCALE = N.T.S. REVISED PLOT DATE = \$DATF\$ CHECKED - JIG REVISED

· Ç Exist. 14" ∅

(northwest wall)

MWRD pipe

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

THE NORTH BRANCH CHICAGO RIVER

ENCLOSURE WALLS: REMOVAL DETAILS II (STRUCTURE NO. 016-6057)

SECTION COUNTY SHEET NO. S-81 11-E1525-00-BR соок CDOT PROJECT NO. E-1-525 124 of 210

±59'-11"

REMOVAL ELEVATION - SIDE VIEW

(Looking south)

(Northwest enclosure wall is shown. Southeast enclosure

wall is similar but opposite hand, except as noted)

WEBSTER AVENUE BRIDGE OVER

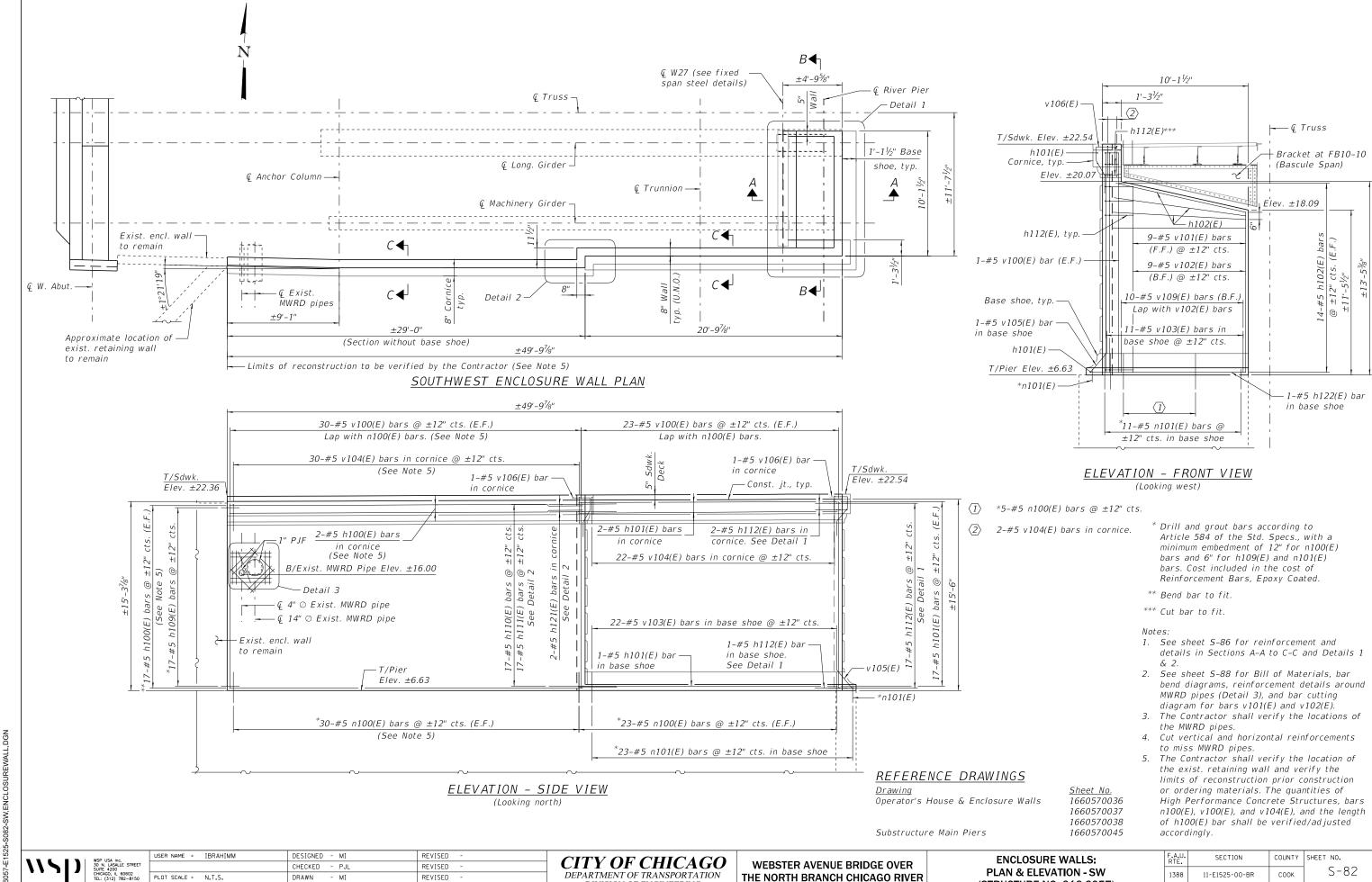
T/Pier Elev. ±6.63

Concrete Removal, typ.

 * Existing 1" arnothing dowel bars shall be

cleaned and incorporated into new

construction. Cost included with



DIVISION OF ENGINEERING

(STRUCTURE NO. 016-6057)

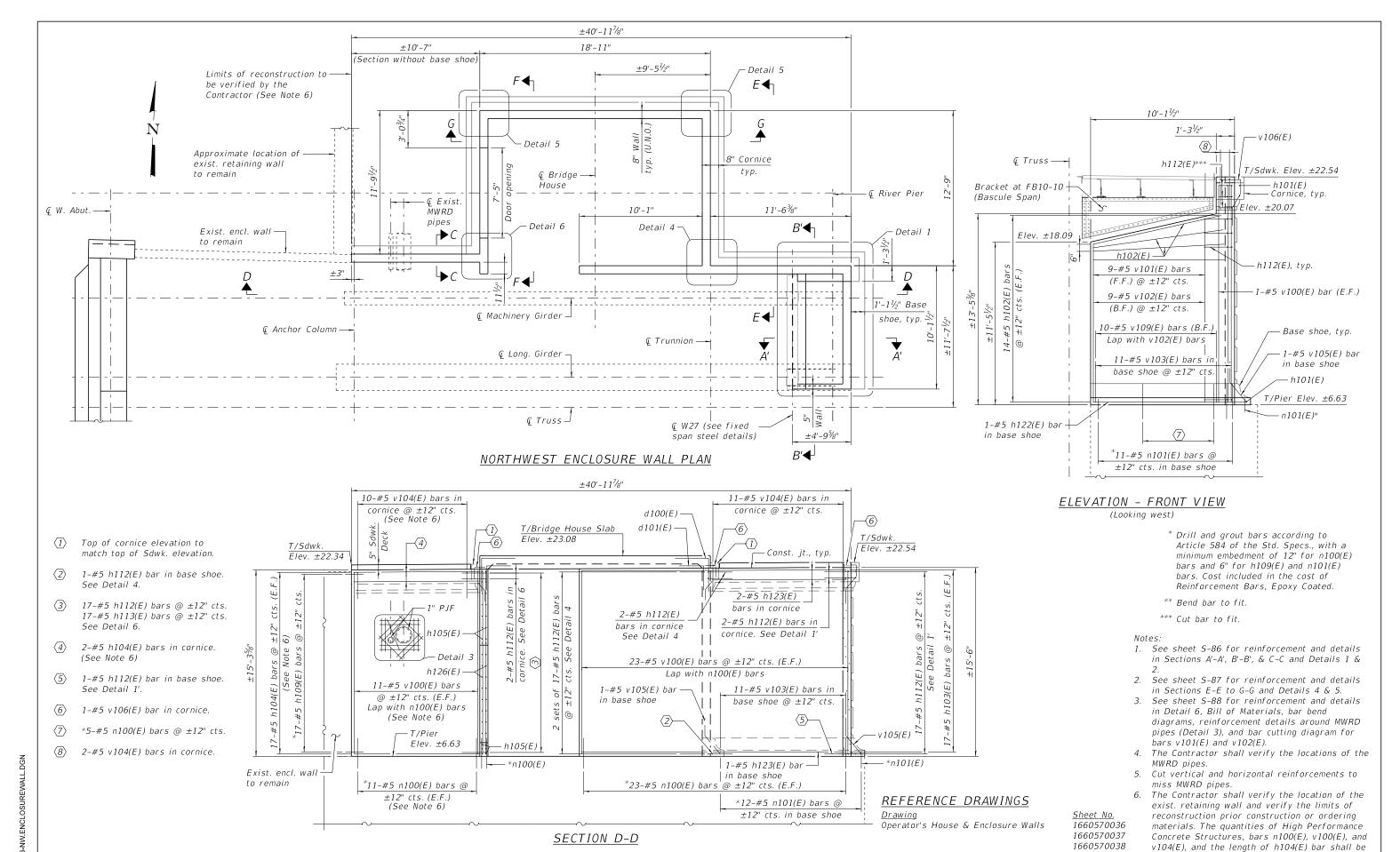
CDOT PROJECT NO. E-1-525

125 of 210

PLOT DATE = \$DATF\$

CHECKED - JIG

REVISED



DESIGNED - MI REVISED USER NAME = IBRAHIMM CHECKED - PJL REVISED REVISED PLOT DATE = \$DATF\$ CHECKED - JIG REVISED

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

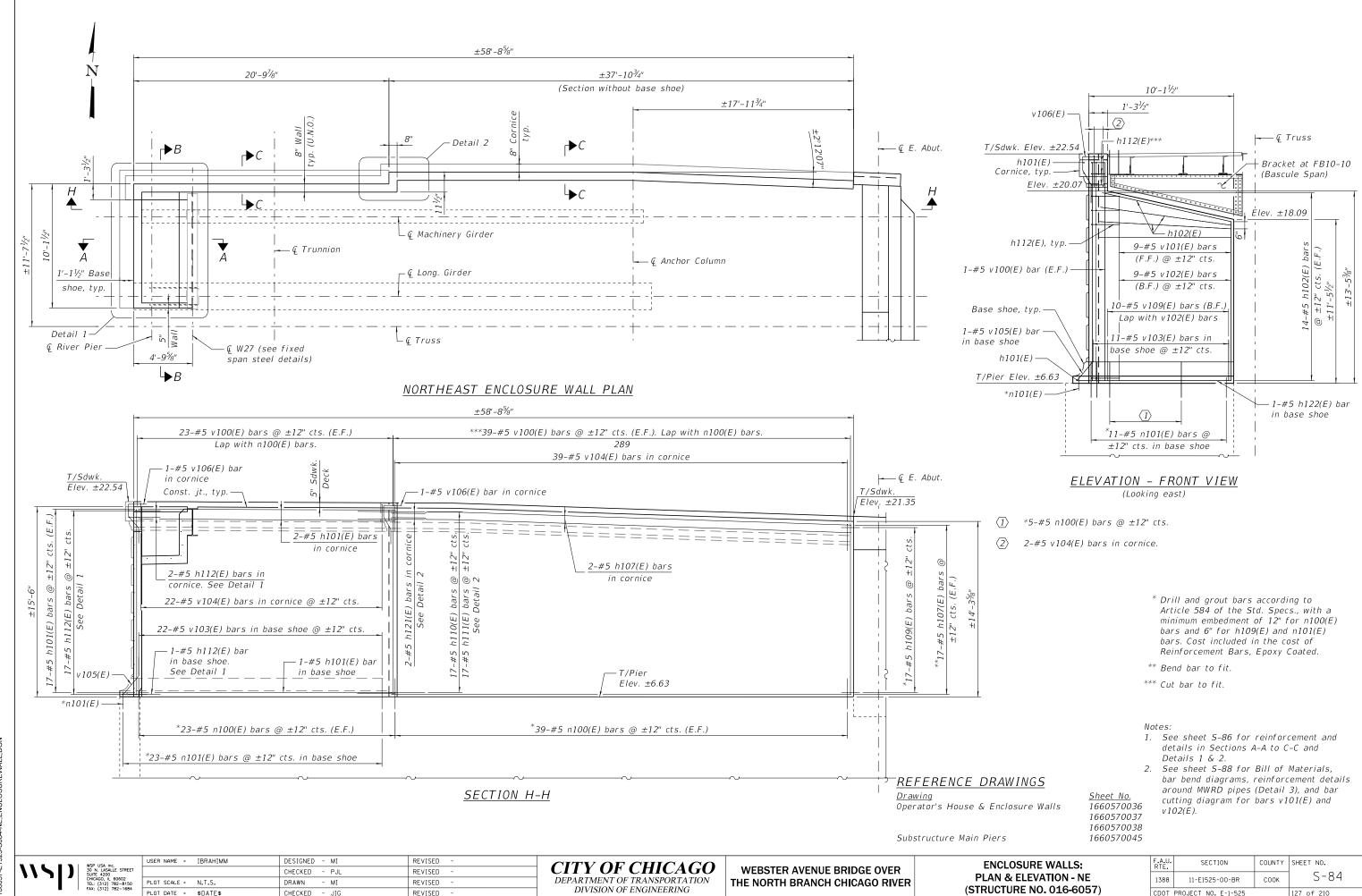
Substructure Main Piers

ENCLOSURE WALLS: PLAN & ELEVATION - NW (STRUCTURE NO. 016-6057)

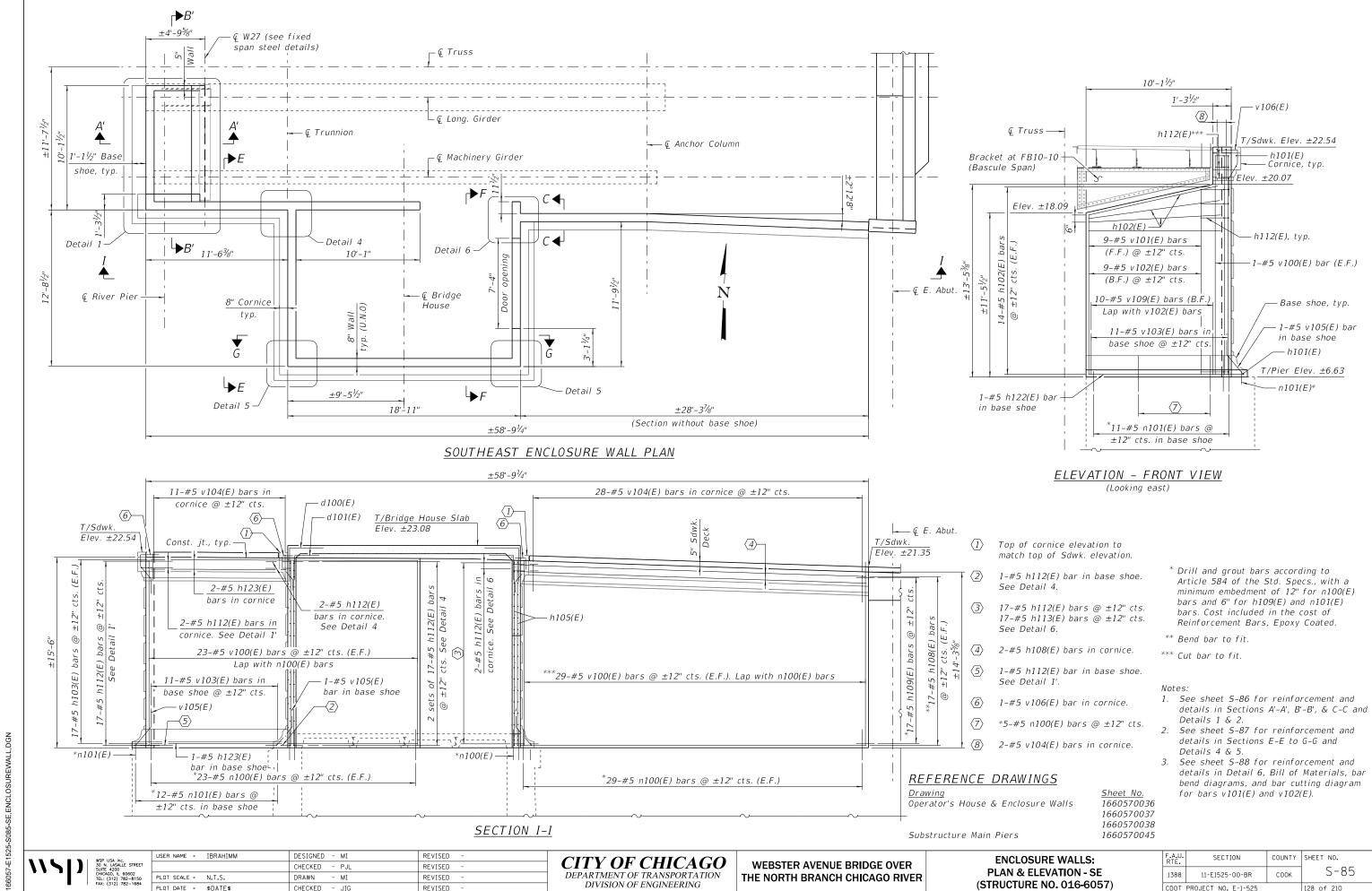
1660570045

SECTION COUNTY SHEET NO. S-83 11-E1525-00-BR CDOT PROJECT NO. E-1-525 126 of 210

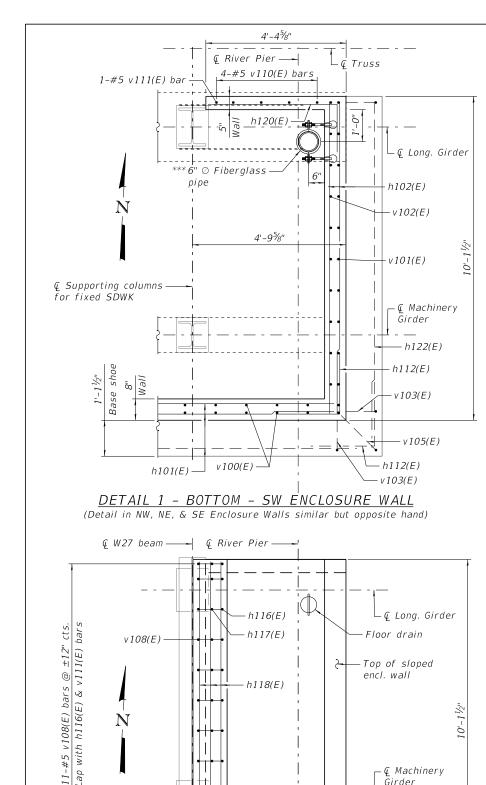
verified/adjusted accordingly.

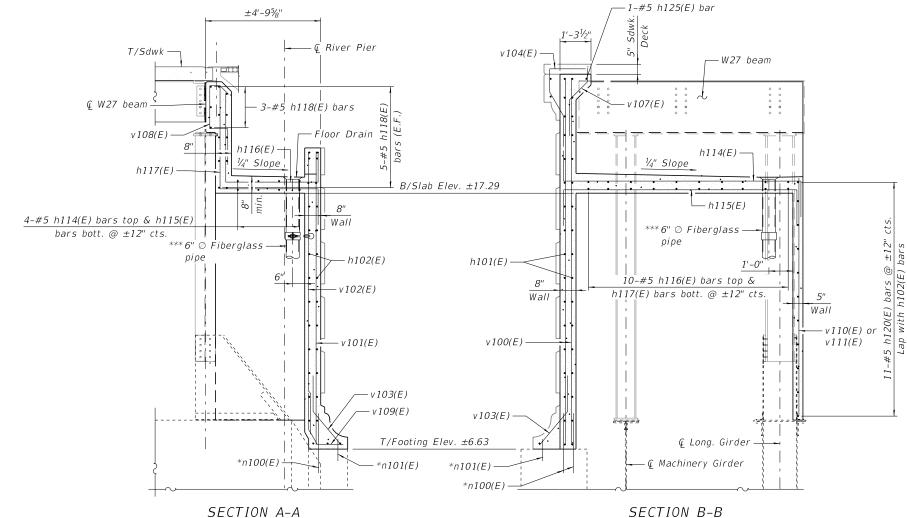


0166057-F1525-S084-NF ENCLOSURE



REVISED





(Section A'-A' similar but opposite hand)

(Section B'-B' similar but opposite hand)

* Drill and grout bars according to Article 584 of the Std. Specs., with a minimum embedment of 12" for n100(E) bars and 6" for h109(E) and n101(E) bars. Cost included in the cost of Reinforcement Bars, Epoxy Coated.

** Cut bar to fit.

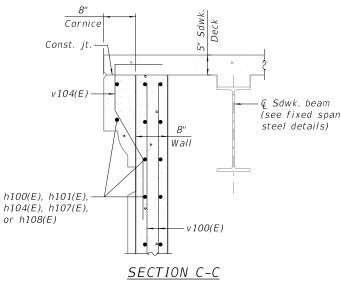
*** Fiberglass pipe shall conform to ASTM D2996, with short-time rupture strength hoop tensile stress of 30,000 psi min.

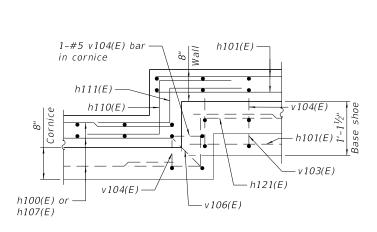
The exterior surfaces of the floor drains shall be coated or pigmented by the manufacturer with a color that matches the concrete.

The clamping device and inserts shall be galvanized according to AASHTO M-232. Cost of fiberglass pipe, clamping device and galvanizing included with Drainage System.

Notes:

- See sheets S-82 & S-84 for locations of Sections A-A & B-B and Detail 2.
- See sheets S-83 & S-85 for locations of Sections A'-A' & B'-B'.
- See sheets S-82 thru S-85 for locations of Section C-C and Detail 1. See Architechural plans for cornice and base
- shoe dimensions.
- See sheet S-88 for Bill of Materials, bar bend diagrams, and fiberglass pipe details and support.





DETAIL 2 - SW ENCLOSURE WALL (Detail in NE Enclosure Wall similar but opposite hand)

DETAIL 1 - TOP - SW ENCLOSURE WALL

v104(E)

h125(E)

(Detail in NW, NE, & SE Enclosure Walls similar but opposite hand)

h101(E)

N

USER NAME = IBRAHIMM DESIGNED - MI REVISED CHECKED - PJL REVISED PLOT SCALE = N.T.S. DRAWN REVISED PLOT DATE = \$DATE\$ CHECKED - JIG REVISED

ℚ Machinery

*h112(E)

· v106(E)

Girder

4-#5 v107(E) bars

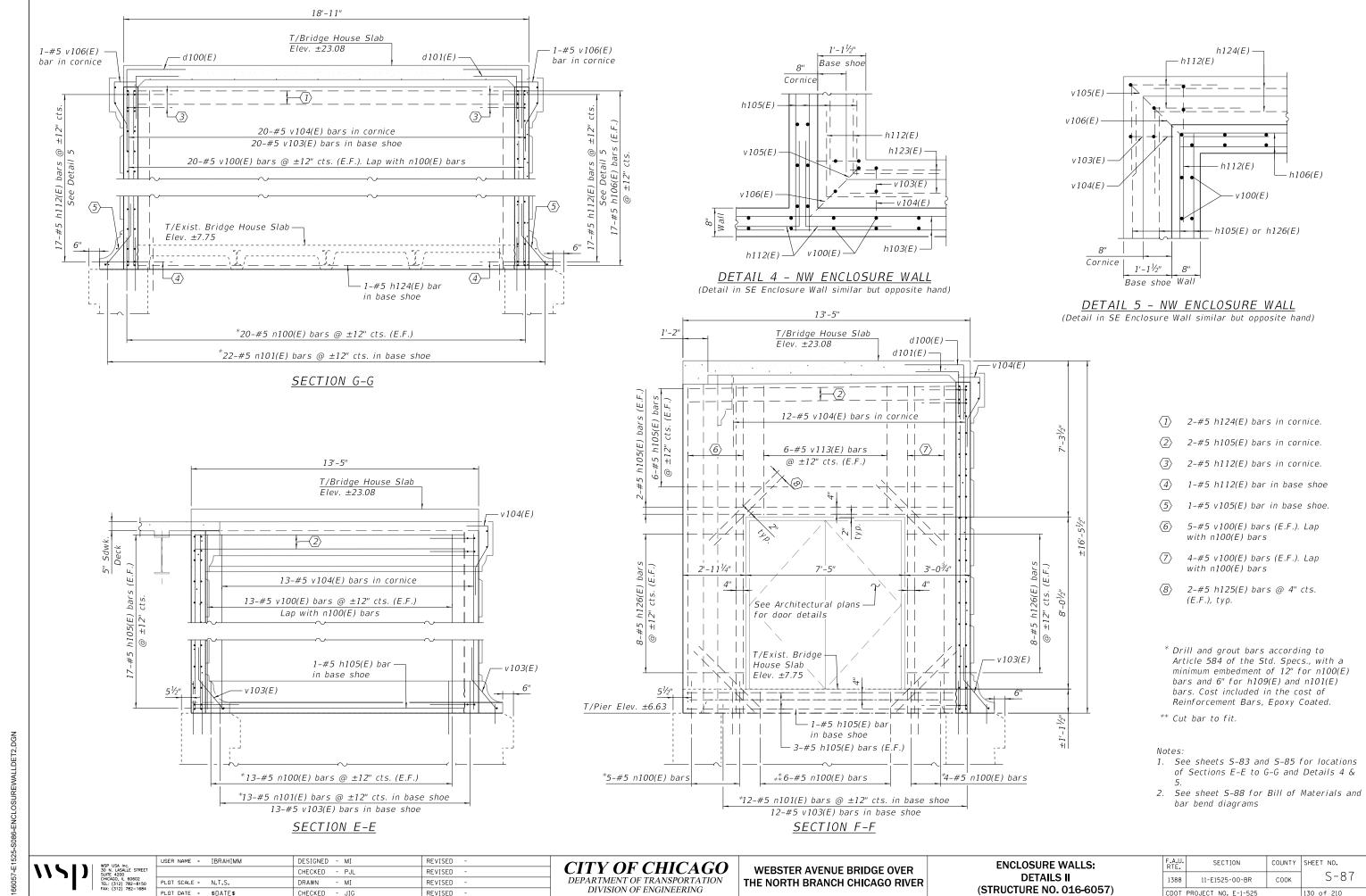
--- v 104(E)

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

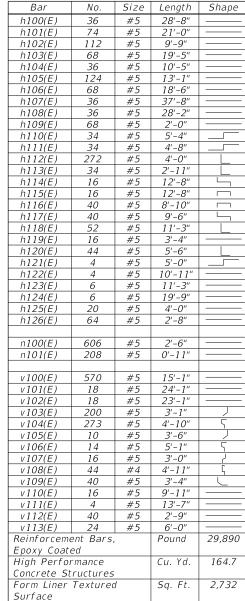
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

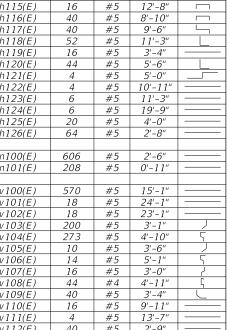
ENCLOSURE WALLS: DETAILS I (STRUCTURE NO. 016-6057)

SECTION COUNTY SHEET NO. S-86 11-E1525-00-BR COOK CDOT PROJECT NO. E-1-525 129 of 210



BILL OF MATERIAL







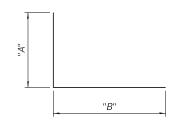
*Fiberglass pipe shall conform to ASTM D2996, with short-time rupture strength hoop tensile stress of 30,000 psi min.

The exterior surfaces of the floor drains shall be coated or pigmented by the manufacturer with a color that matches the concrete.

The clamping device and inserts shall be galvanized according to AASHTO M-232. Cost of fiberglass pipe, clamping device and galvanizing included with Drainage System.

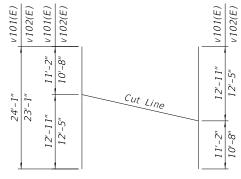
Notes:

- 1. See sheets S-82 and S-83 for location of Detail 3.
- 2. See sheets S-83 and S-85 for location of Detail 6.



BARS h112(E), h113(E), h118(E), & h120(E)

Bar	"A"	"B"
h112(E)	2'-0"	2'-0"
h113(E)	11"	2'-0"
h118(E)	1'-6"	9'-9"
h120(E)	1'-6"	4'-0"



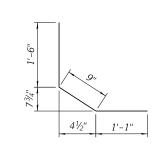
FIELD CUTTING DIAGRAM

(Order v101(E) & v102(E) full length, cut as shown and use the remainder of bars in opposite face)

-		9'-8"	h114(E)
1,-6"	-2"	3'-10"	h117(E)
,1	4		
E)	E)		9
h114(E)	h117(E)		1,-
hΊ	h1	l	
<u>B</u>	ARS	h114(E) & h1	17(E)

h115(E)	h116(E)	9'-8" 3'-8"	h115(E) h116(E)
1,-6"	3'-8''		1'-6"

BARS h115(E) & h116(E)



BAR v109(E)

— € 14" ⊘ MWRD pipe DETAIL 3

2-#5 v112(E) bars

@ 4" cts. (E.F.), typ.

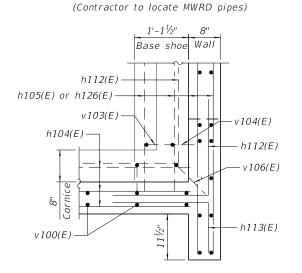
1-#5 v112(E) bar (E.F.)-

2-#5 v112(E) bars

1-#5 v112(E) bar

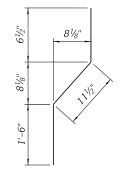
(E.F.), typ.

@ 4" cts. (E.F.), typ.



DETAIL 6 - NW ENCLOSURE WALL

(Detail in SE Enclosure Wall similar but opposite hand)



BARS h110(E), h111(E), & h121(E)

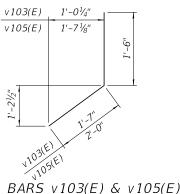
2'-0"

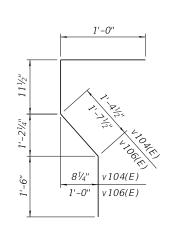
h111(E) h110(E)

h121(E)

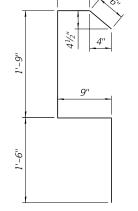
2'-0"

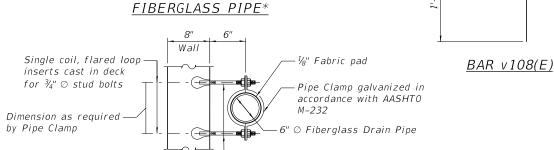
BAR v107(E)





BARS v104(E) & v106(E)



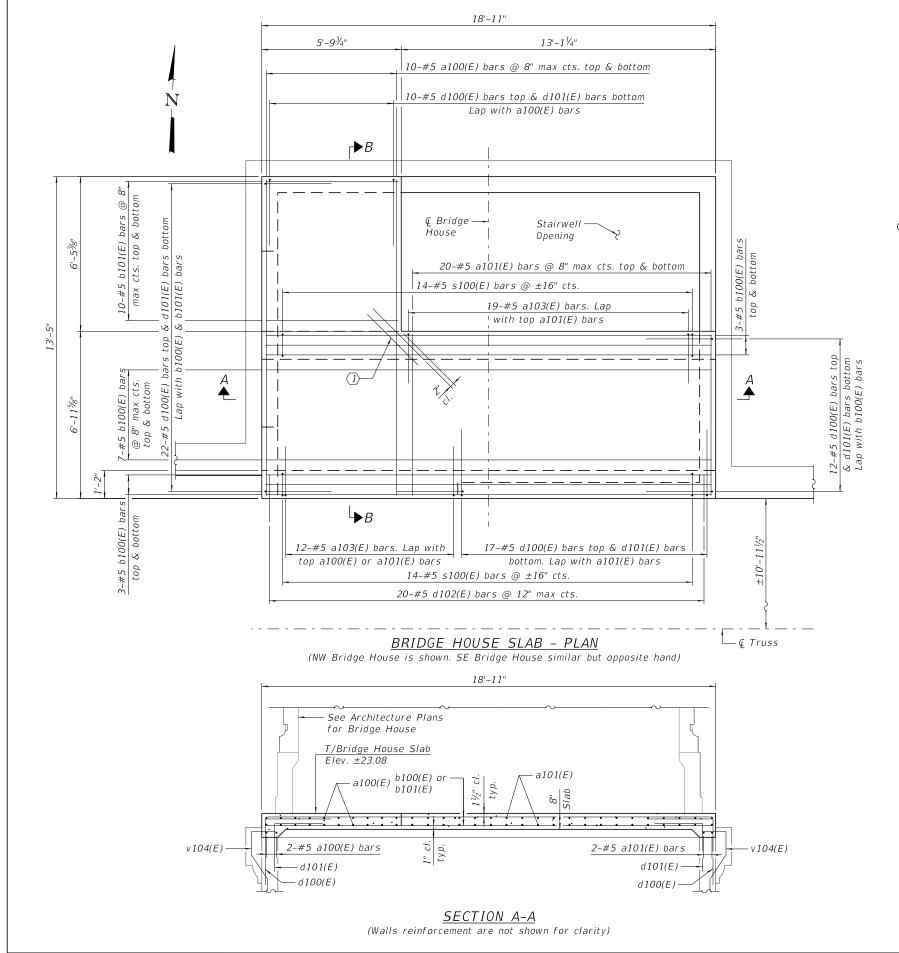


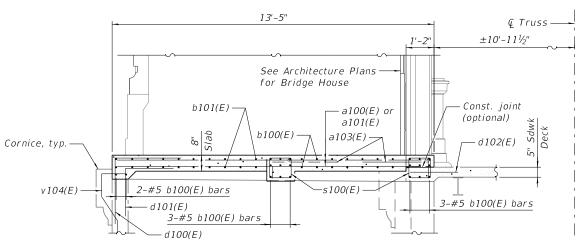
½" ∅ 8" Fiberglass reinforced plastic rebar

USER NAME = IBRAHIMM	DESIGNED - MI	REVISED -
	CHECKED - PJL	REVISED -
PLOT SCALE = N.T.S.	DRAWN - MI	REVISED -
PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -

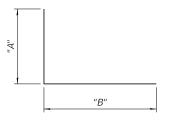
RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-88
CDOT	PROJECT NO. E-1-525		131 of 210

¾" ∅ S.S. Stud Bolts, threaded 3" for insert end and 6" for clamp end, with 2 locknuts. PIPE SUPPORT DETAIL*



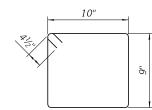


<u>SECTION B-B</u> (Walls reinforcement are not shown for clarity)



BARS a103(E), d100(E), & d101(E)

Bar	"A"	"B"
a103(E)	10"	2'-0"
d100(E)	2'-9"	2'-9"
d101(E)	2'-0"	2'-0"



BAR s100(E)

BILL OF MATERIAL

Bar	No.	Size	Length	Shape	
a100(E)	44	#5	13'-2"		
a101(E)	84	#5	6'-8"		
a102(E)	8	#5	3'-0"		
a103(E)	62	#5	2'-10"		
b100(E)	64	#5	18'-7"		
b101(E)	44	#5	5'-6"		
d100(E)	122	#5	5'-6"		
d101(E)	122	#5	4'-0"		
d102(E)	40	#5	2'-0"		
s100(E)	56	#4	3'-11"		
Reinforceme	ent Bars,		Pound	4,340	
Epoxy Coated					
High Perfor	rmance	Cu. Yd.	10.6		
Concrete St	ructures				
Quantity cho	wn includ	toc MM	and SE h	nous os	

Quantity shown includes NW and SE houses.

1 2-#5 a102(E) bars @ 4" cts. top & bottom.

Notes:

1. See sheets S-91 to S-93 for Bridge House connection details.

WSP USA Inc.
30 N. LASALLE STREET
SUITE 4200
CHICAGO, IL 60602
TEL: (312) 782-8150
FAX: (312) 782-1684

 USER NAME = IBRAHIMM
 DESIGNED - MI
 REVISED

 CHECKED - PJL
 REVISED

 PLOT SCALE = N.T.S.
 DRAWN - MI
 REVISED

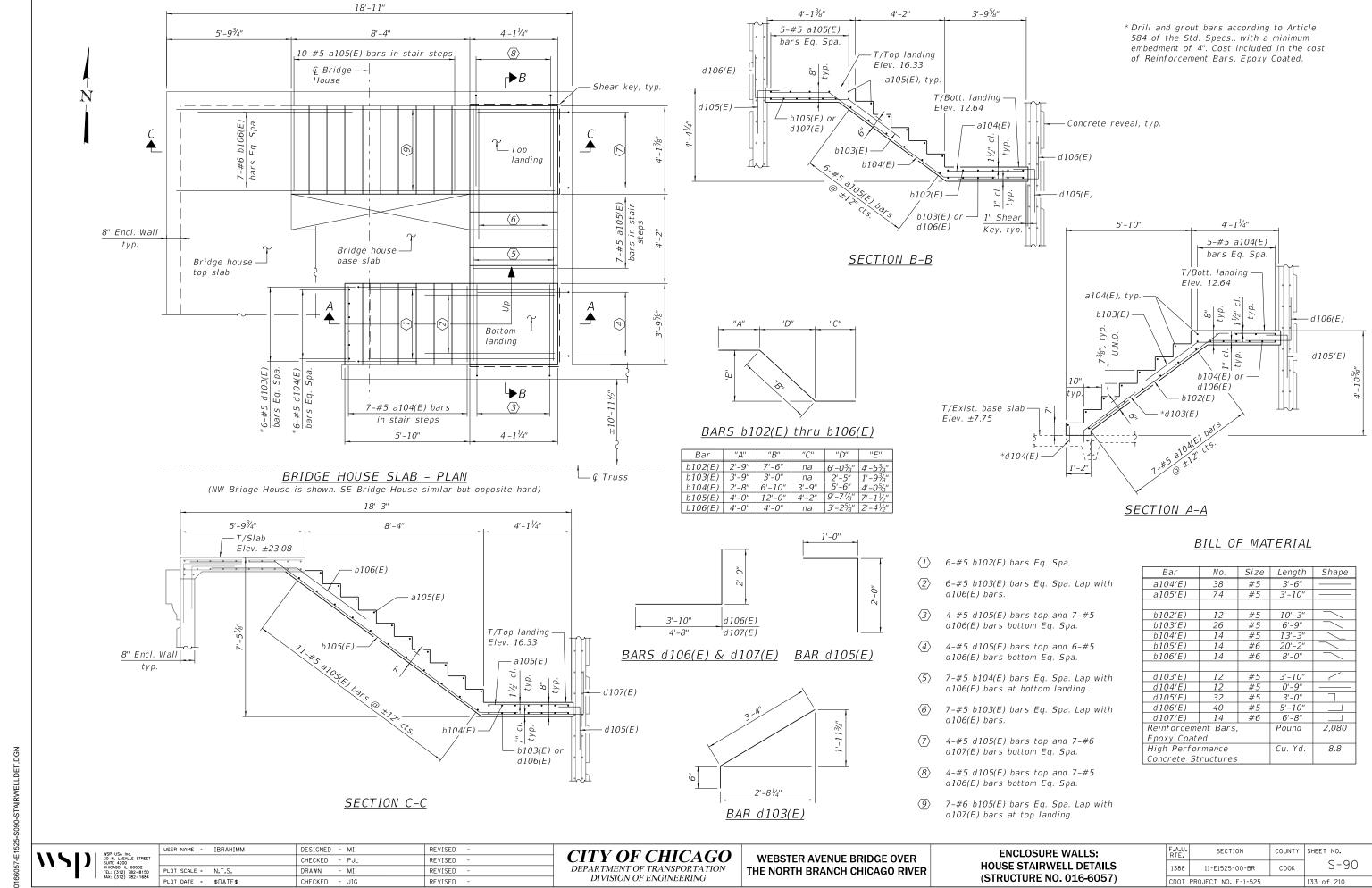
 PLOT DATE = \$DATE\$
 CHECKED - JIG
 REVISED

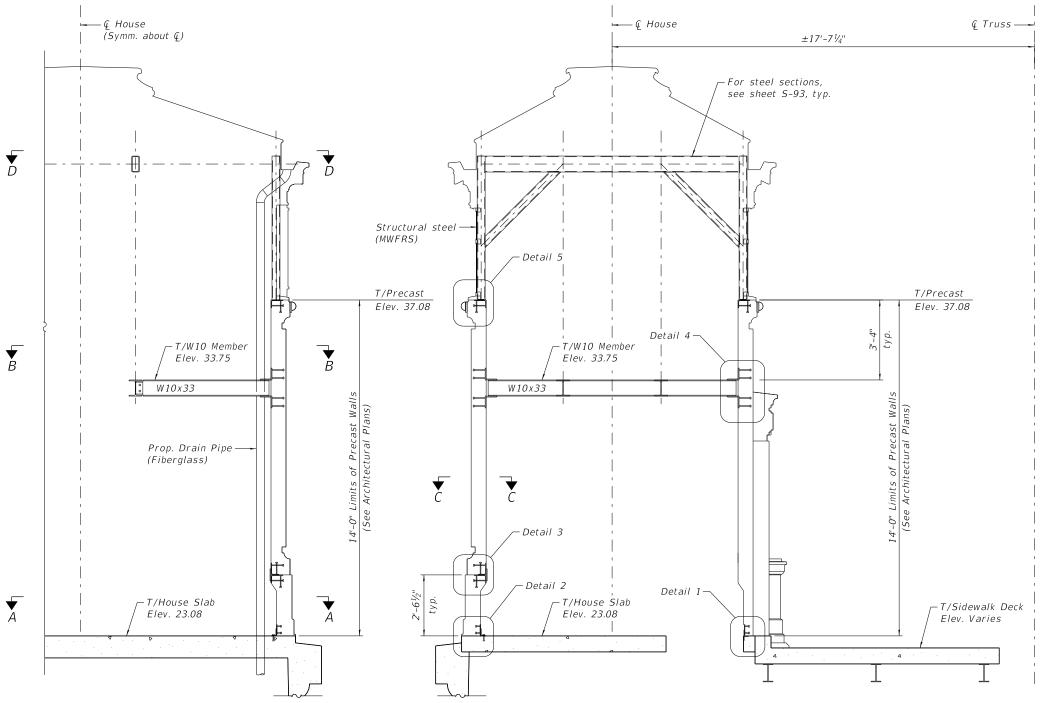
CITY OF CHICAGO

DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

ENCLOSURE WALLS: HOUSE SLAB DETAILS (STRUCTURE NO. 016-6057)





TYPICAL TRANSVERSE SECTION
THRU BRIDGE HOUSE

(NW House looking east, SE House looking west)

GENERAL NOTES - BRIDGE HOUSE

STRUCTURAL STEEL (MWFRS)

- 1. Cost of bridge house structural steel included in the cost of Furnishing and Erecting Structural Steel.
- 2. The structural design for the bridge house structural steel, Main Wind Force Resisting System (MWFRS) is based on the 2017 AASHTO LRFD Bridge Design Specification, 8th Edition and the following wind loading:

Basic wind speed, strength
Basic wind speed, service
Wind exposure category

Basic wind speed, service
70 mph (25-year MRI)
B

3. The intermediate bracing is based on wind loads above and dead load of 50 psf, superimposed dead load of 10 psf, and live load of 10 psf.

COLD FORMED STEEL (CFS) FRAMING

- Cost of cold formed steel framing shall not be measured for payment but considered part of Precast Concrete Wall item.
- 2. The CFS framing shown is for information purposes only. The CFS framing is to support vertical loads from the upper portion of bridge house which shall include dead load, roof live load, and superimposed dead load.
- 3. The Contractor shall follow Architectural Detailed Specification Section 054000 Cold-Formed Metal Framing and the structural performance and standards setforth within the specification.

PRECAST CONCRETE WALLS

- Cost of precast concrete walls included in the cost of Precast Concrete Wall. See architectural special provisions.
- Precast concrete walls shall be designed for a minimum of the connection forces shown in the plans and the loads specified by the manufacturer. Precast manufacturer shall work with cold formed steel framing (CFS) delegated designer and account for additional vertical forces from CFS framing.

Notes:

- 1. Drainage system shall connect through existing lower slab. See sheet S-89 for details. See special provisions.
- 2. See sheet S-88 for pipe support detail.
- 3. See sheet S-92 for Section A-A, B-B & C-C and Detail 1 thru 5.
- 4. See sheet S-93 for Section D-D.

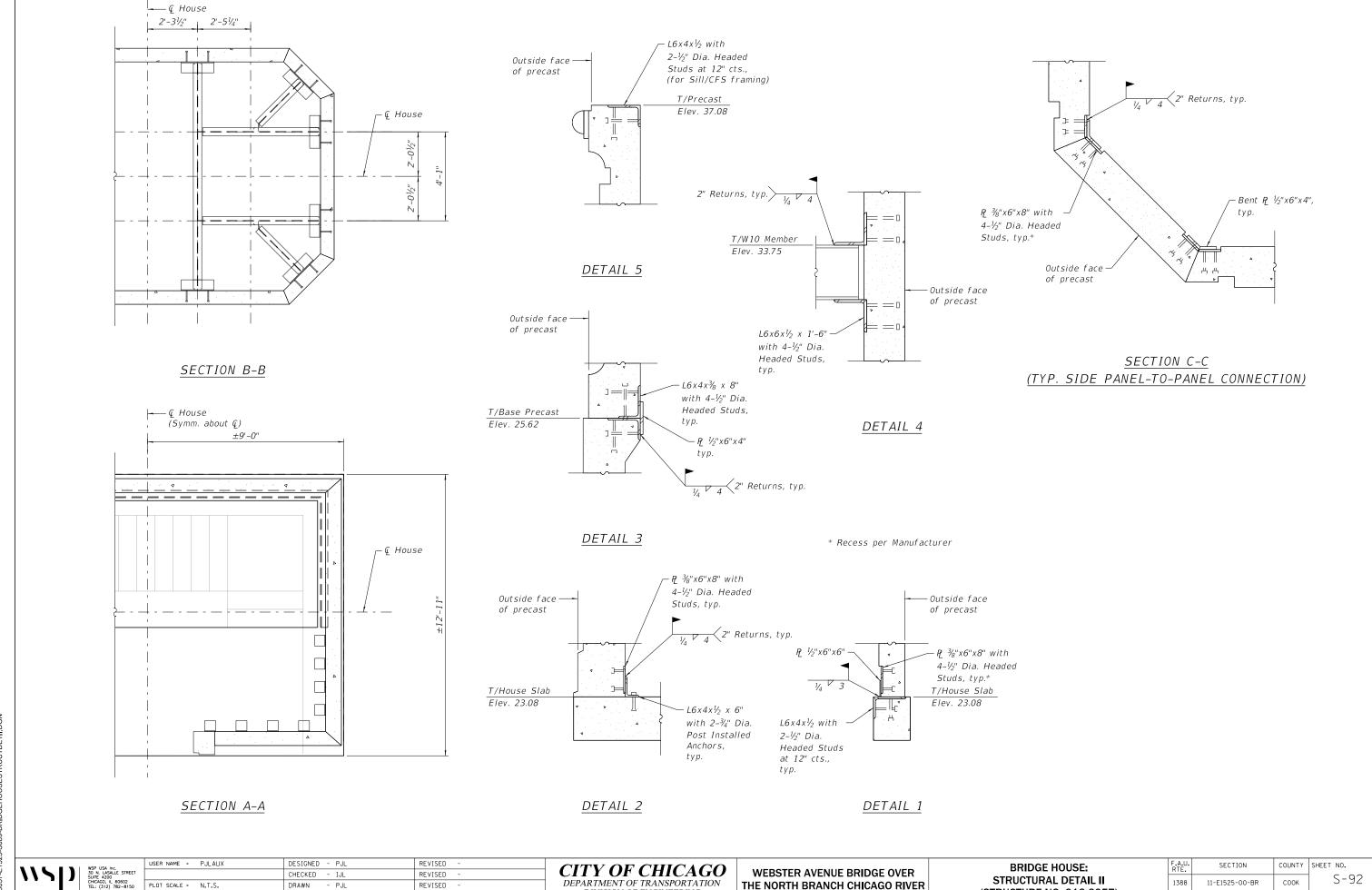
BILL OF MATERIAL

Item	Unit	Quantity	
Drainage System	L. Sum	1	

HALF LONGITUDINAL SECTION

THRU BRIDGE HOUSE

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-91
CDOT	PROJECT NO. E-1-525		134 of 210



DIVISION OF ENGINEERING

(STRUCTURE NO. 016-6057)

CDOT PROJECT NO. E-1-525

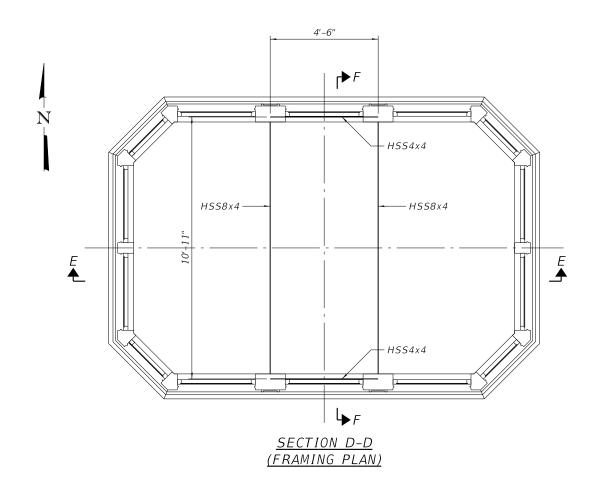
135 of 210

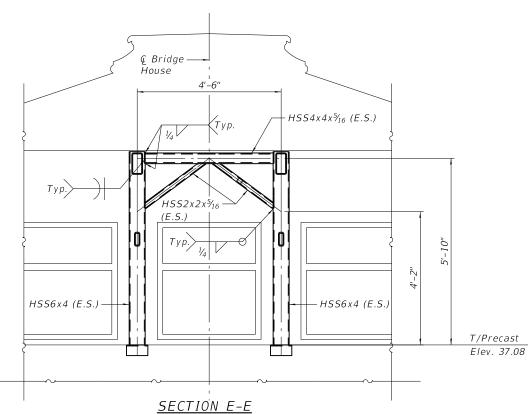
0166057-E1525-S089-BRIDGEHOUSEST

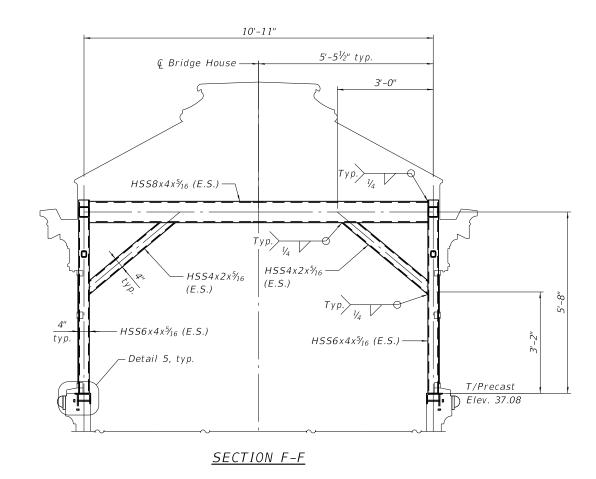
PLOT DATE = \$DATE\$

CHECKED - JIG

REVISED







HOUSE CFS FRAMING TABLE

CFS Member	No. – Size Type (Configuration)	Min. Thickness	Spacing	I.D. (Min. Size)
Base Track	1 – 4x2 Track	0.0451"	_	400T200-43
Jamb	2 - 4x1.625 Studs with	0.0451"	-	4005162-43
	1 – 4x1.25 Track (Box)	0.0451"		400T125-43
Head Track	2 - 4x1.25 Tracks with	0.0451"	-	400T125-43
	2 - 4x1.625 Studs (Box)	0.0451"		4005162-43
Header	1 – 4x2 Track	0.0451"	-	400T200-43
Roof Rafter	1 - 8x2 Studs	0.0451"	at 16"	8005200-43
Roof Joist	1 - 4x1.625 Studs	0.0451"	at 16"	4005162-43
Ridge Joist	1 - 8x2 Studs	0.0451"	-	8005200-43

Note: See Architectural plans for locations and details, and see Special Provisions.

- 1. Load carrying components designated "NTR" shall conform to the Impact Testing Requirement, Zone 2.
- 2. See sheet S-91 for location of Section D-D.
- 3. See sheet S-92 for Detail 5.

wsp	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684
-----	---

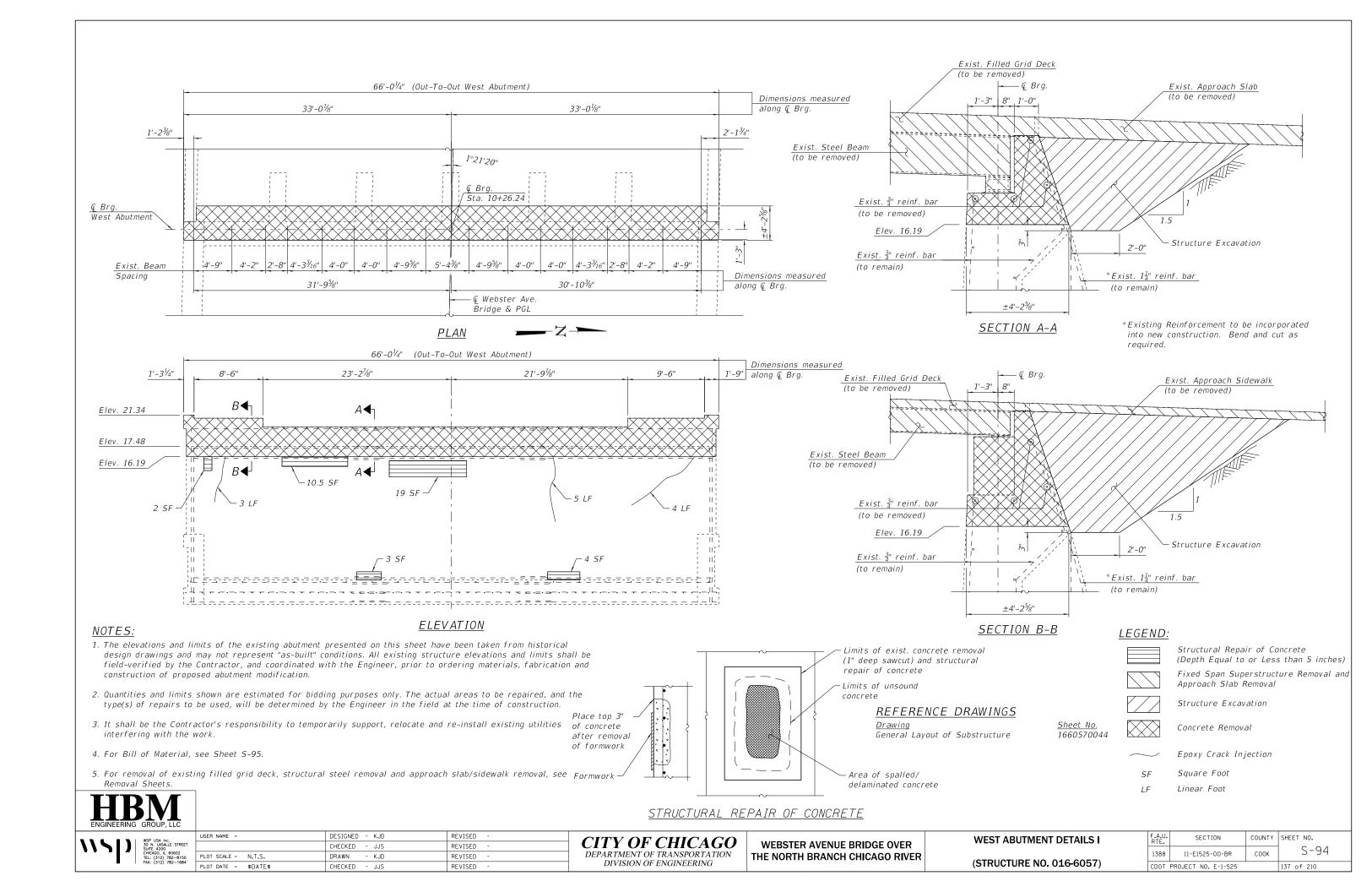
USER NAME = PJLAUX DESIGNED - PJL REVISED CHECKED - IJL REVISED PLOT SCALE = N.T.S. DRAWN - PJL REVISED PLOT DATE = 10/5/2020 CHECKED - JIG REVISED

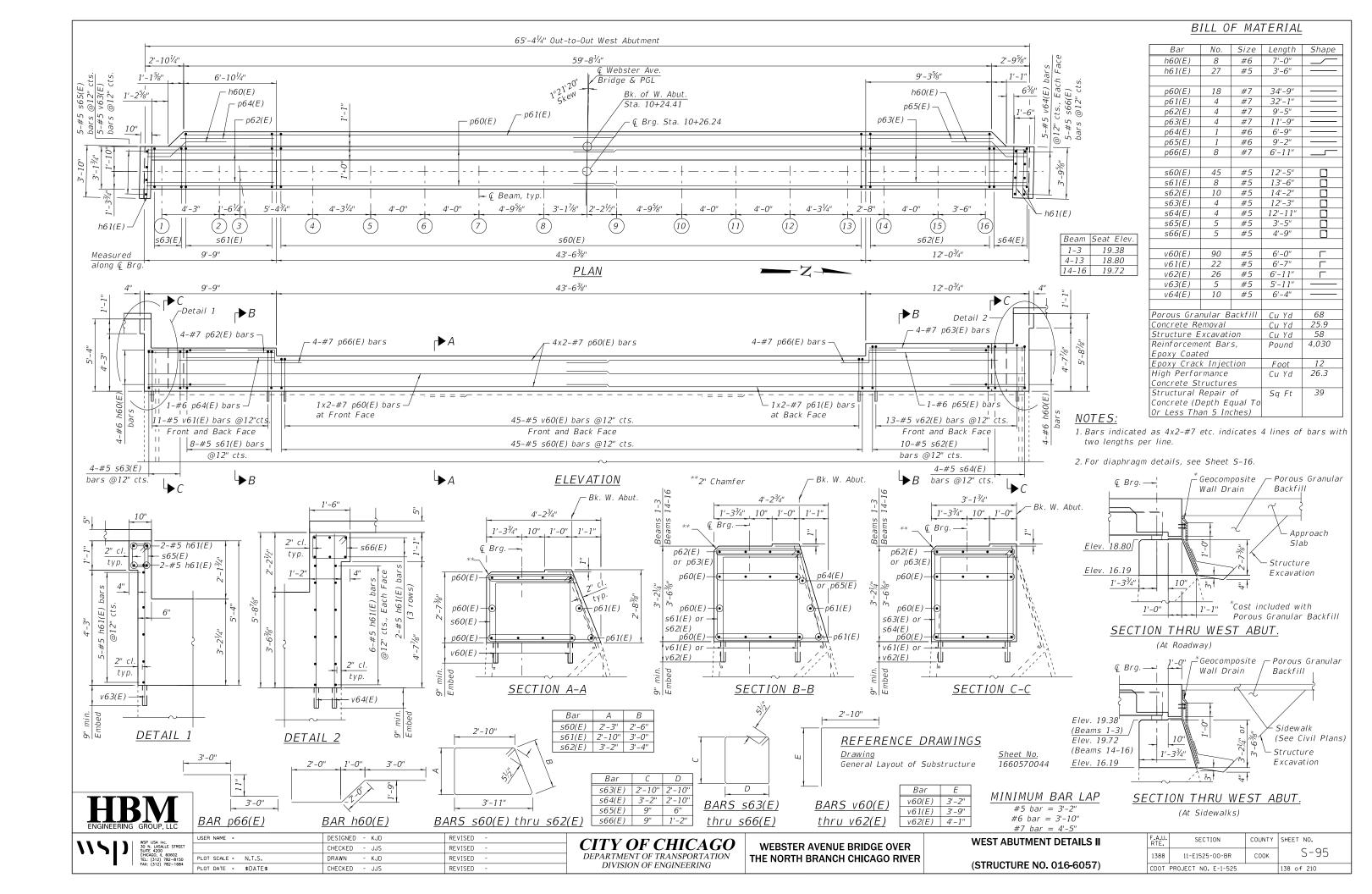
CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

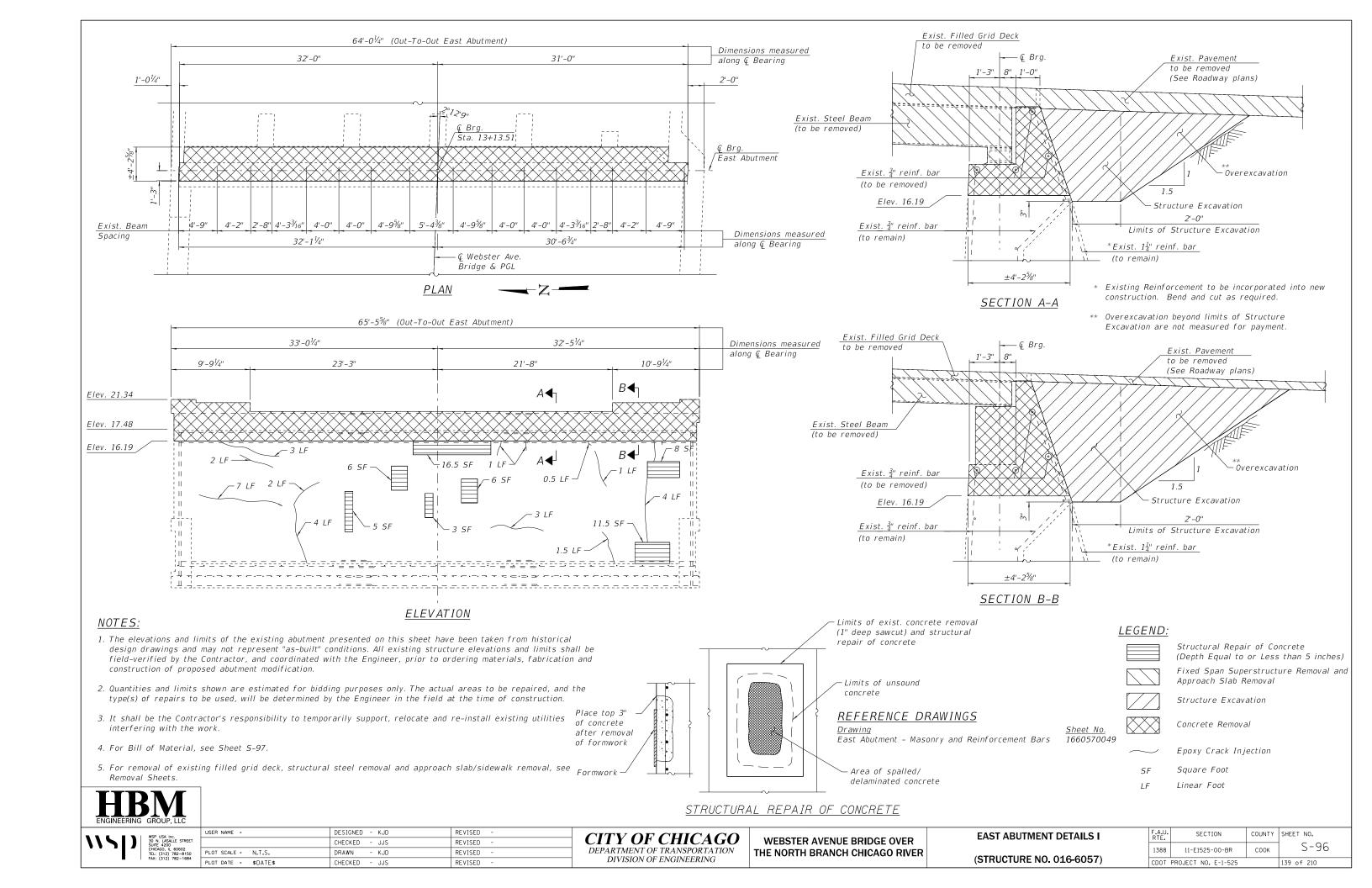
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

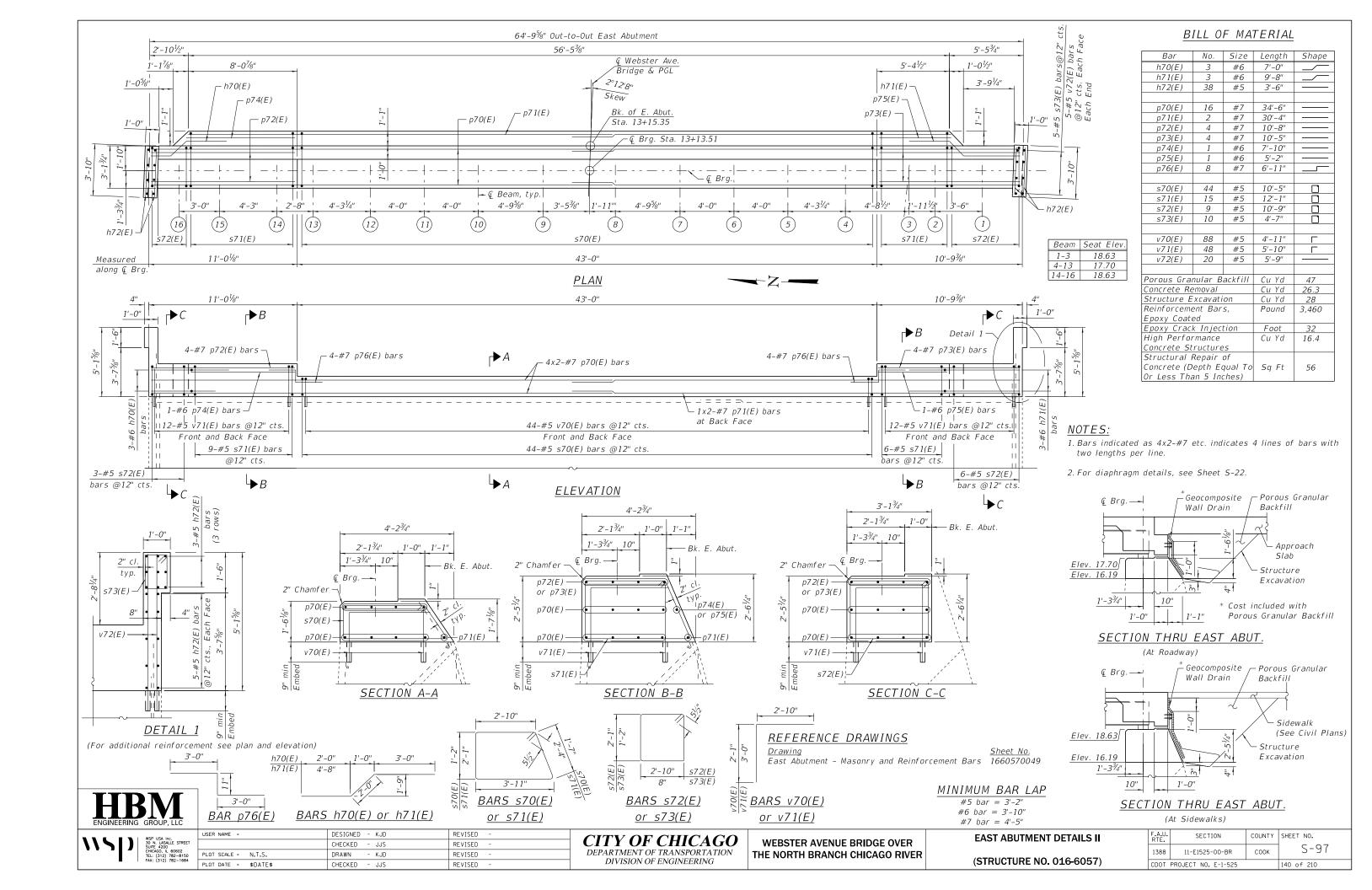
BRIDGE HOUSE: STRUCTURAL DETAILS III (STRUCTURE NO. 016-6057)

COUNTY SHEET NO. SECTION S-93 11-E1525-00-BR соок CDOT PROJECT NO. E-1-525 136 of 210







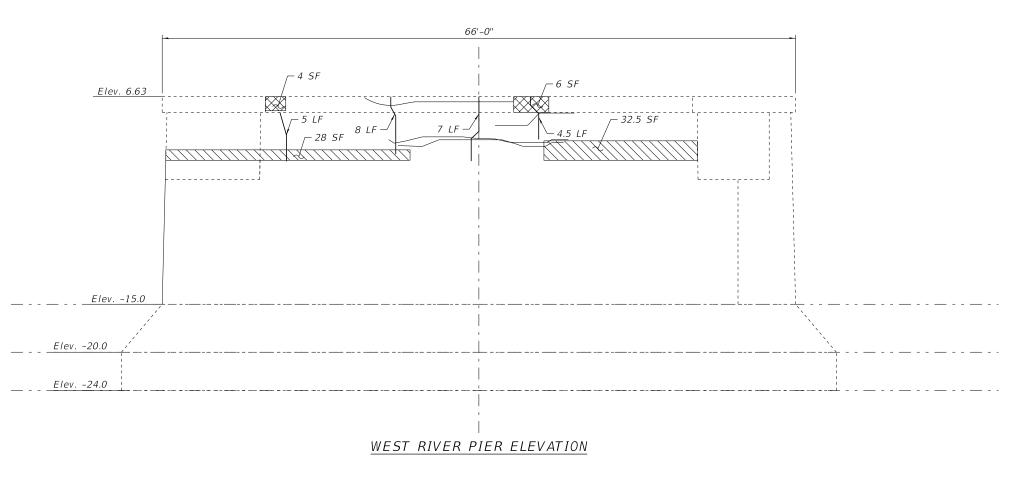


NOTES:

- 1. Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contactor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not because for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 2. The presented elevations and dimensions have been taken from historical design drawings and may not present "as-built" condition. All exisiting structure limits shall be field verified by the contractor and coordinated with the engineer prior to ordering materials, fabrication and construction of the proposed wall modification.
- 3. Exisiting Utilities in conflict with new construction shall be abandoned, protected or relocated according to directions given on roadway plans.

BILL OF MATERIAL

Item	Unit	Quantity
Epoxy Crack Injection	Foot	25
Structural Repair of Concrete (Depth		
Equal To or Less Than 5 Inches)	Sq. Ft.	61
Structural Repair of Concrete (Depth		
Greater Than 5 Inches)	Sq. Ft.	10



<u>REFERENCE DRAWI</u>NGS

<u>Drawing</u> General Layout of Substructure Main Piers West Piers - Reinforcement Bars <u>Sheet No.</u> 1660570044 1660570045 1660570046





- Structural Repair of Concrete (Depth Equal To or Less Than 5 Inches)



Structural Repair of Concrete (Depth Greater Than 5 Inches)



- Epoxy Crack Injection



- Hairline Crack (HL) (Width <0.06") (For Information Only)

SF

- Square Foot

l F

- Linear Foot



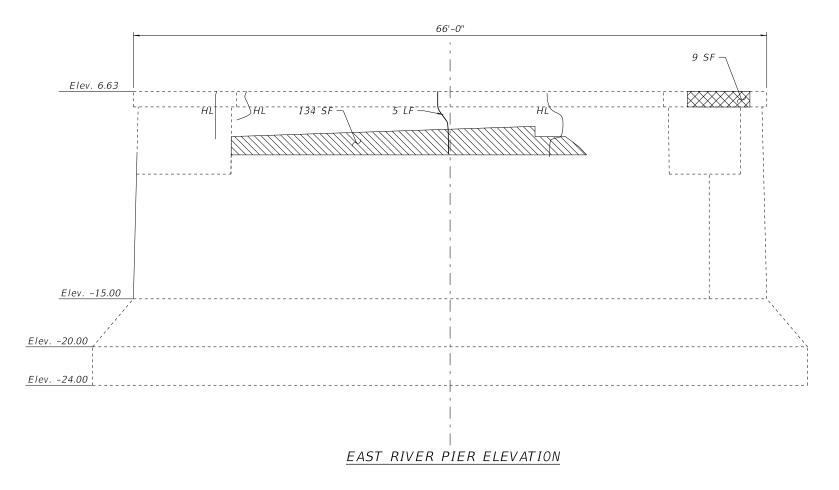


USER NAME =	DESIGNED - AMI	REVISED -
	CHECKED - MI	REVISED -
PLOT SCALE = N.T.S.	DRAWN - AMI	REVISED -
PLOT DATE = \$DATE\$	CHECKED - MI	REVISED -

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-98
CDOT	PROJECT NO. E-1-525		141 of 210

NOTES:

- 1. Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contactor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not because for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 2. The presented elevations and dimensions have been taken from historical design drawings and may not present "as-built" condition. All exisiting structure limits shall be field verified by the contractor and coordinated with the engineer prior to ordering materials, fabrication and construction of the proposed wall modification.
- 3. Exisiting Utilities in conflict with new construction shall be abandoned, protected or relocated according to directions given on roadway plans.



REFERENCE DRAWINGS

<u>Drawing</u> General Layout of Substructure Main Piers West Piers - Reinforcement Bars <u>Sheet No.</u> 1660570044 1660570045 1660570046

BILL OF MATERIAL

Item	Unit	Quantity
Epoxy Crack Injection	Foot	5
Structural Repair of Concrete (Depth	Sq. Ft.	134
Equal To or Less Than 5 Inches)		
Structural Repair of Concrete (Depth	Sq. Ft.	9
Greater Than 5 Inches)		

LEGEND



- Structural Repair of Concrete (Depth Equal To or Less Than 5 Inches)



- Structural Repair of Concrete (Depth Greater Than 5 Inches)



- Epoxy Crack Injection



- Hairline Crack (HL) (Width <0.06") (For Information Only)

SF - Square Foot

.F – Linear Foot

HBM ENGINEERING GROUP, LLC



SER NAME =	DESIGNED - AMI	REVISED -	
	CHECKED - MI	REVISED -	
LOT SCALE = N.T.S.	DRAWN - AMI	REVISED -	
OT DATE = \$DATE\$	CHECKED - MI	REVISED -	

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-99
CDOT	PROJECT NO. E-1-525		142 of 210

30'-3" 30'-3" \bot A 61x2-#5a(E) at 12" TOP

#5b(E) 4"-0" POROUS GRANULAR BACKFILL EXISTING PIER SECTION A-A **EXISTING** ABUTMENT

BILL OF MATERIAL WEST FIXED SPAN

Bar	No.	Size	Length	Shape
a(E)	61	#5	14'-2"	
b(E)	15	#5	30'-7''	
POROUS GE	RANULAR	BACKFILL	CU YD	130
CLASS SI CONCRETE		CU YD	16.3	
(MISCELLANEOUS)				
METAL LADDER		EACH	2	

BILL OF MATERIAL EAST FIXED SPAN

Bar	No.	Size	Length	Shape
a(E)	61	#5	14'-2"	
b(E)	15	#5	30'-7''	
POROUS GRANULAR BACKFILL			CU YD	130
CLASS SI CONCRETE			CU YD	16.3
(MISCELLANEOUS)				
METAL LADDER			EACH	2

Note: Cost of Reinforcement bars, Epoxy Coated included with Class S1 Concrete (Miscellaneous).

½" SIDE RAIL BAR Q ¾" φ EXP. BOLT -BENT BAR ¾"x3"x11"

SECTION C-C

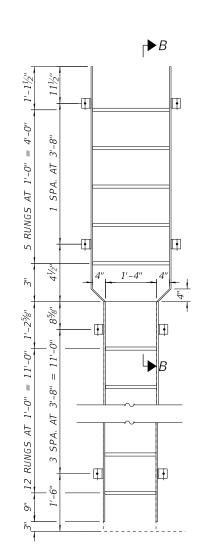
NOTE:

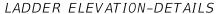
1. After removal of existing access slab, the Contactor shall field verify POROUS GRANUAL BACKFILL amount needed and make necessary approved adjustments prior to construction of proposed access slab or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

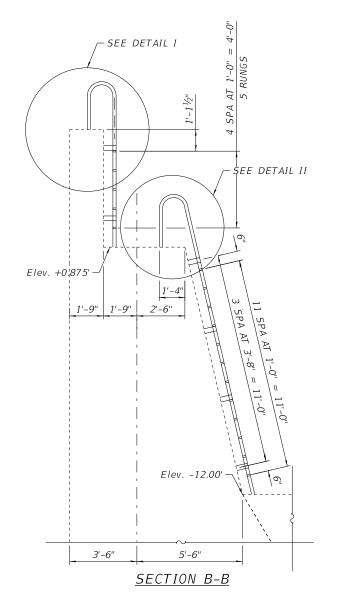
REFERENCE DRAWINGS

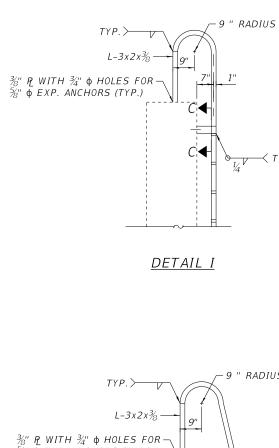
METERICE BIOWINGS	
<u>Drawing</u>	Sheet No.
General Layout of Substructure	1660570044
Main Piers	1660570045
West Piers - Reinforcement Bars	1660570046
East Piers - Reinforcement Bars and Bar Tables	1660570047

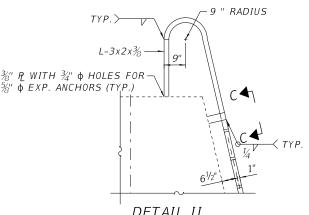
EAST AND WEST PLAN-PIT ACCESS SLAB













wsp	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684
-----	---

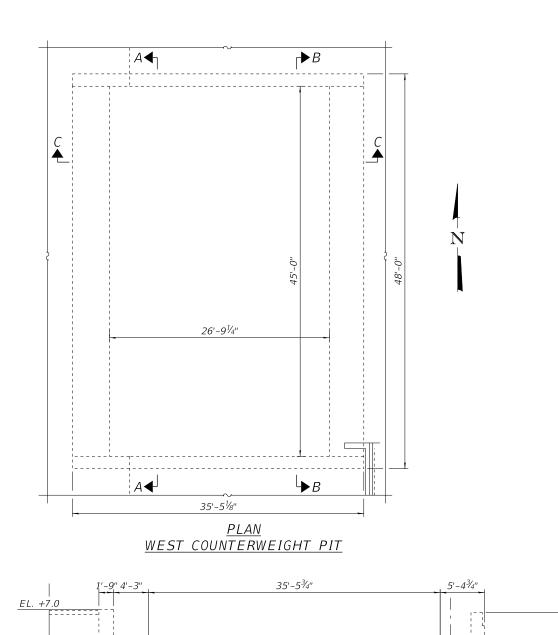
	USER NAME =	DESIGNED - MA	REVISED -
		CHECKED - MI	REVISED -
	PLOT SCALE = N.T.S.	DRAWN - AMS	REVISED -
	PLOT DATE = \$DATE\$	CHECKED - MA	REVISED -
ī			

CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER PLATFORM AND LADDER DETAILS WEST AND EAST FIXED SPANS (STRUCTURE NO. 016-6057)

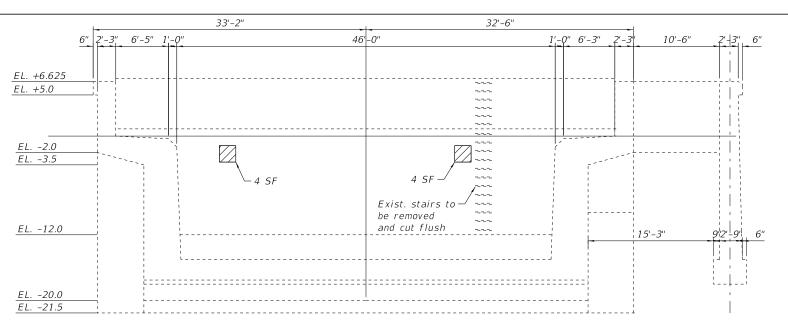
F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-100
CDOT	PROJECT NO. E-1-525		143 of 210

DETAIL II

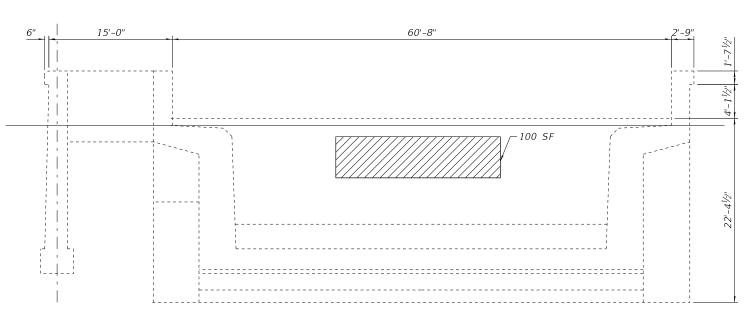


EL. +7.0 EL. +7.0 EL. +1.46 EL. +1.46 EL. +1.50 EL. -20.0 EL. -21.5 13-0" 22'-0" 15-6"

SECTION C-C



SECTION A-A



SECTION B-B

BILL OF MATERIAL

ITEM	UNIT	TOTAL QUANTITY
Structural Repair of Concrete (Depth Equal to or Less than 5 Inches)	Sq. Ft.	121

REFERENCE DRAWINGS

<u>Drawing</u> General Layout of Substructure Main Piers West Piers - Reinforcement Bars <u>Sheet No.</u> 1660570044 1660570045 1660570046

LEGEND:

Structural Repair of Concrete
(Depth Equal to or Less than 5 Inches)

SF Square Foot

. F Linear Foot

ENGINEERING GROUP, LLC

 USER NAME
 =
 DESIGNED
 MA
 REVISED

 CHECKED
 MI
 REVISED

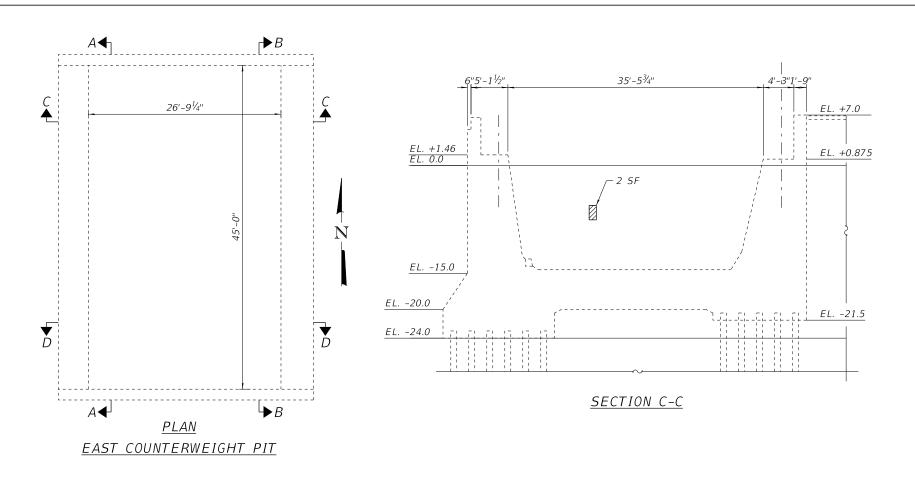
 PLOT SCALE =
 N.T.S.
 DRAWN
 AMS
 REVISED

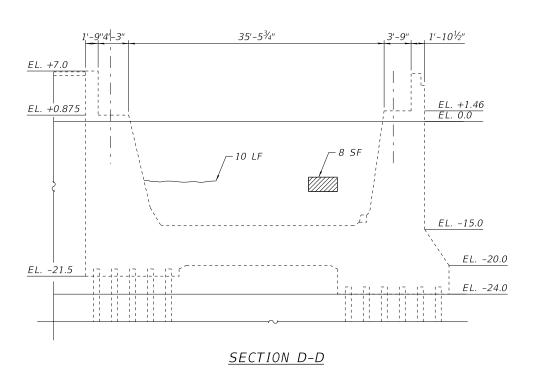
 PLOT DATE =
 \$DATE\$
 CHECKED
 MAA
 REVISED

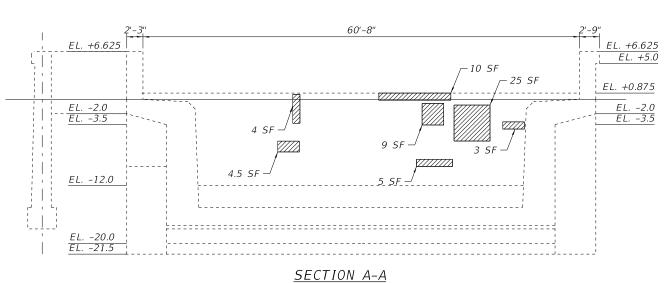
CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

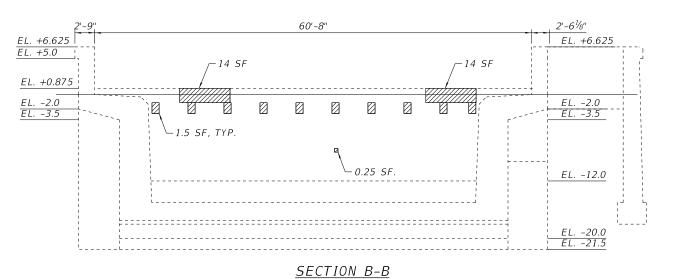
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

WEST PIT REPAIR DETAILS
(STRUCTURE NO. 016-6057)









BILL OF MATERIAL

ITEM	UNIT	TOTAL QUANTITY
Epoxy Crack Injection	Foot	10
Structural Repair of Concrete (Depth	Sq. Ft.	114
Equal to or Less Than 5 Inches)		

REFERENCE DRAWINGS

DrawingSheet No.General Layout of Substructure1660570044Main Piers1660570045East Piers - Reinforcement Bars and Bar Tables1660570047

LEGEND:

Structural Repair of Concrete (Depth Equal to or Less than 5 Inches)

Low Pressure Epoxy Injection

SF Square Foot LF Linear Foot



///> 	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684
-----------------	---

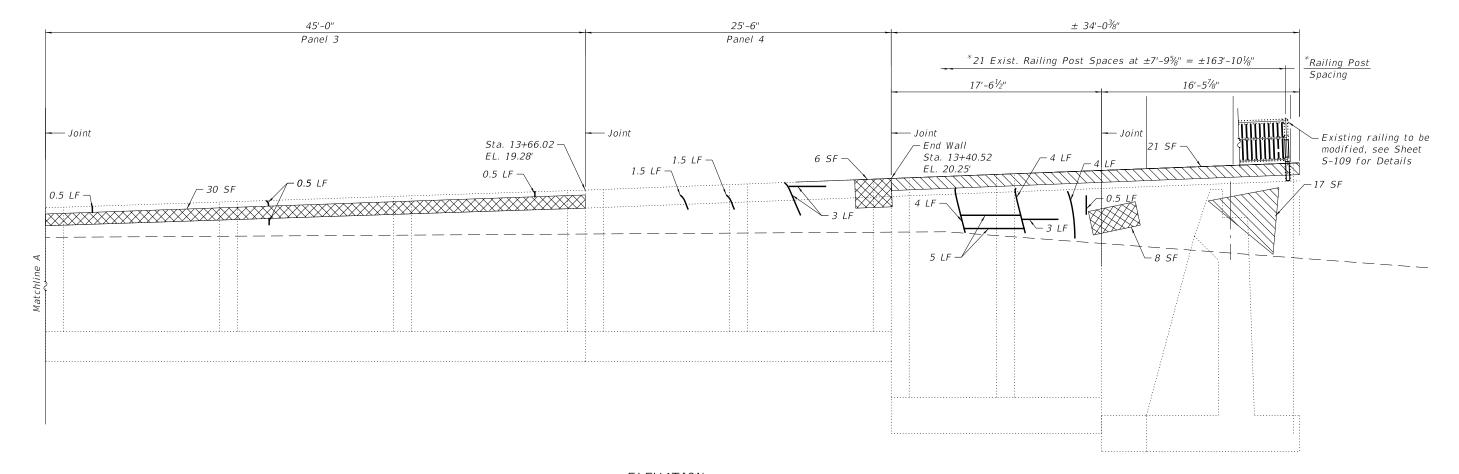
USER NAME =	DESIGNED - MA	REVISED -
	CHECKED - MI	REVISED -
PLOT SCALE = N.T.S.	DRAWN - AMS	REVISED -
PLOT DATE = \$DATE\$	CHECKED - MAA	REVISED -

CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

EAST PIT REPAIR DETAILS
(STRUCTURE NO. 016-6057)

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-102
CDOT	PROJECT NO. E-1-525		145 of 210



<u>ELEVATION</u>

BILL OF MATERIAL

·		
Item	Unit	Quantity
Epoxy Crack Injection	Foot	47
Structural Repair of Concrete (Depth	Sq. Ft.	38
Equal To or Less Than 5 Inches)		
Structural Repair of Concrete (Depth	Sq. Ft.	45.3
Greater Than 5 Inches)		

<u>NOTES:</u>

- 1. Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contactor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not because for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 2. The presented elevations and dimensions have been taken from historical design drawings and may not present "as-built" condition. All exisiting structure limits shall be field verified by the contractor and coordinated with the engineer prior to ordering materials, fabrication and construction of the proposed wall modification.
- Exisiting Utilities in conflict with new construction shall be abandoned, protected or relocated according to directions given on roadway plans.

REFERENCE DRAWINGS

Drawing
Survey Layout
General Layout of Substructure
East Approach - Retaining Wall Details and Reinforcement Bars
1660570051

<u>LEGEND</u>

- Structi

Structural Repair of Concrete (Depth Equal To or Less Than 5 Inches)



- Structural Repair of Concrete (Depth Greater Than 5 Inches)



- Epoxy Crack Injection



- Hairline Crack (HL) (Width <0.06") (For Information Only)

SF - Square Foot

F – Linear Foot



USER NAME =	DESIGNED - AMI	REVISED -
	CHECKED - MI	REVISED -
PLOT SCALE = N.T.S.	DRAWN - AMI	REVISED -
PLOT DATE = 10/5/2020	CHECKED - MAA	REVISED -

CITY OF CHICAGO

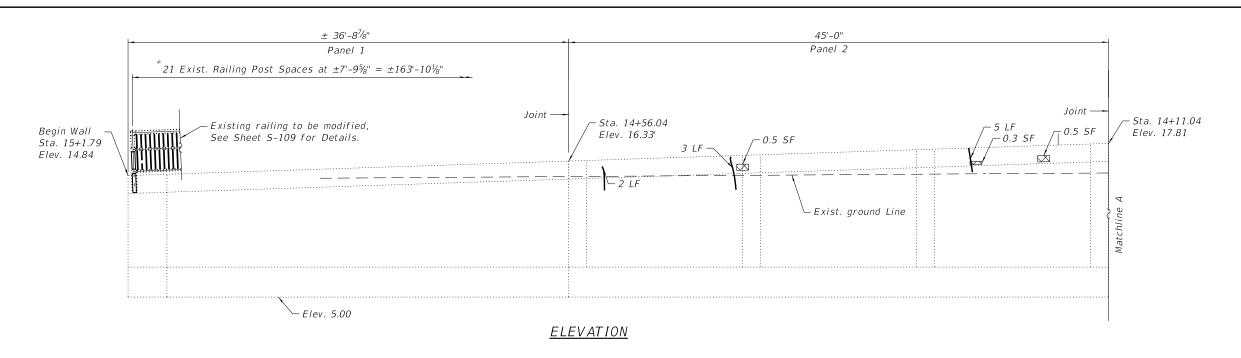
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

NORTHEAST RETAINING WALL PLAN AND ELEVATION I (STRUCTURE NO. 016-6057)

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	COOK	S-103
CDOT	PROJECT NO. E-1-525		146 of 210

^{*} Contractor shall determine/ verify the existing railing post spaces. See Sheet S-108 and S-109 for more details.



*Contractor shall determine/ verify the existing railing post spaces. See Sheets S-108 and S-109 for more details.

<u>NOTE:</u>

1. For notes, see Sheet S-103.

REFERENCE DRAWINGS

Drawing Sheet No.

Survey Layout 1660570042
General Layout of Substructure 1660570044
East Approach - Retaining Wall Details and Reinforcement Bars 1660570051

<u>LEGEND</u>

-

- Structural Repair of Concrete (Depth Greater Than 5 Inches)

- Epoxy Crack Injection

SF - Square Foot

LF - Linear Foot

ENGINEERING GROUP, LLC

wsp	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684
-----	---

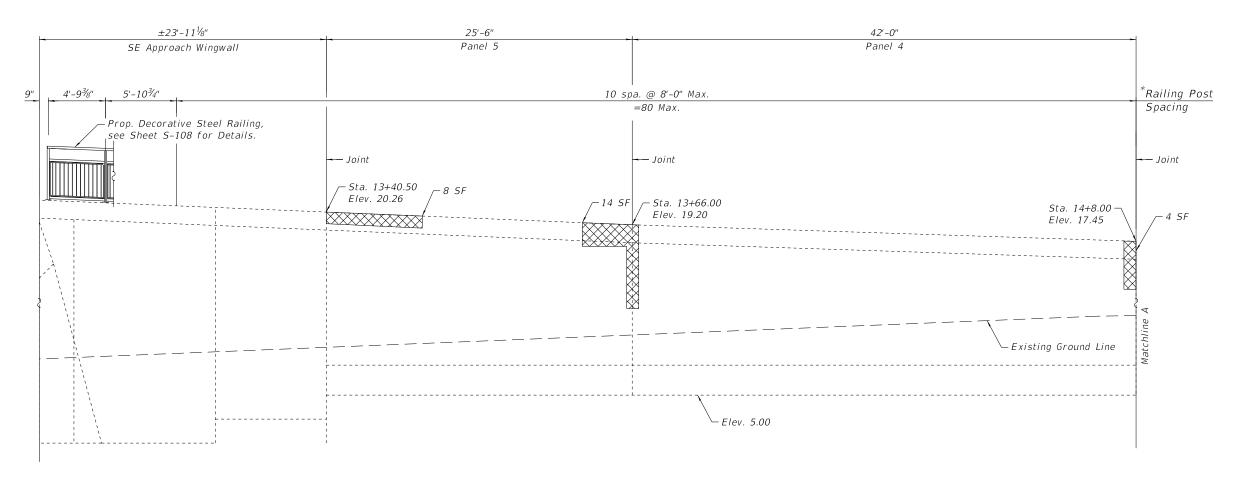
ER NAME =	DESIGNED - AMI	REVISED -	CITYOI
	CHECKED - MI	REVISED -	CITY OF
OT SCALE = N.T.S.	DRAWN - AMI	REVISED -	DEPARTMENT C
OT DATE = 10/5/2020	CHECKED - MAA	REVISED -	DIVISION C



WEBSTER AVENUE BRIDGE OVER	
THE NORTH BRANCH CHICAGO RIV	/ER

NORTHEAST RETAINING WALL
PLAN AND ELEVATION II
(STRUCTURE NO. 016-6057)

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-104
CDOT	PROJECT NO. E-1-525		147 of 210



<u>ELEVATION</u>

*Contractor shall determine final post spacing. See Sheet S-108.

BILL OF MATERIAL

Item	Unit	Quantity
Epoxy Crack Injection	Foot	45
Structural Repair of Concrete (Depth	Sq. Ft.	26
Equal to or Less Than 5 Inches)		
Structural Repair of Concrete (Depth	Sq. Ft.	34
Greater Than 5 Inches)		

LEGEND



Sheet No.

1660570042

1660570044

1660570051

- Structural Repair of Concrete (Depth Greater Than 5 Inches)

SF -Square Foot

<u>NOTE:</u>

1. For notes, see Sheet S-103.

HBM ENGINEERING GROUP, LLC

USER NAME	=	DESIGNED	-	

USER NAME =	DESIGNED - AMI	REVISED -
	CHECKED - MI	REVISED -
PLOT SCALE = N.T.S.	DRAWN - AMI	REVISED -
PLOT DATE = \$DATE\$	CHECKED - MAA	REVISED -

CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

<u>Drawing</u>

REFERENCE DRAWINGS

General Layout of Substructure

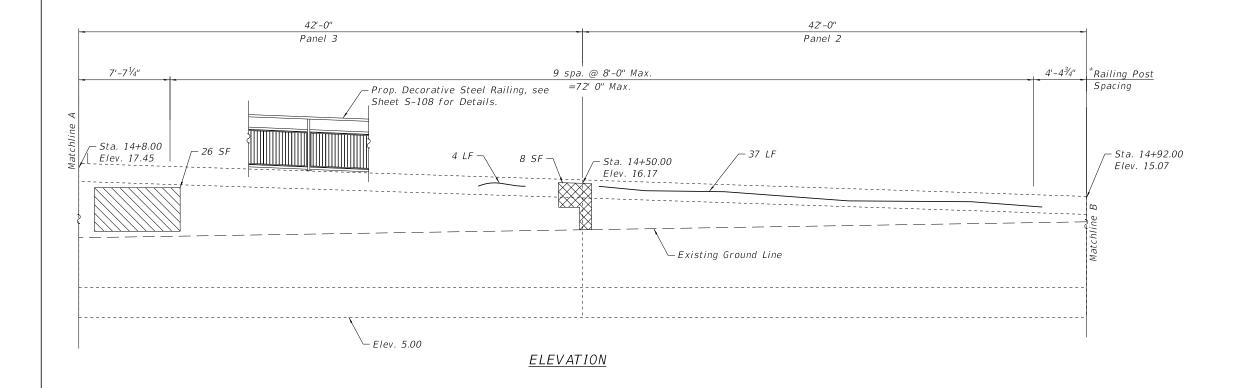
East Approach - Retaining Wall Details and Reinforcement Bars

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

SOUTHEAST RETAINING WALL PLAN AND ELEVATION I (STRUCTURE NO. 016-6057) F.A.U. SECTION COUNTY SHEET NO.

1388 11-E1525-00-BR COOK S-105

CDOT PROJECT NO. E-1-525 148 of 210



*Contractor shall determine final post spacing. See Sheet S-108.

REFERENCE DRAWINGS

<u>Drawing</u> Survey Layout General Layout of Substructure East Approach - Retaining Wall Details and Reinforcement Bars

<u>LEGEND</u>



- Structural Repair of Concrete (Depth Equal To or Less Than 5 Inches)



Structural Repair of Concrete (Depth Greater Than 5 Inches)

- Epoxy Crack Injection

- Square Foot

- Linear Foot

1. For notes, see Sheet S-103.

NOTE:

USER NAME =	DESIGNED - AMI	REVISED -
	CHECKED - MI	REVISED -
PLOT SCALE = N.T.S.	DRAWN - AMI	REVISED -
PLOT DATE = \$DATE\$	CHECKED - MAA	REVISED -
122		

CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

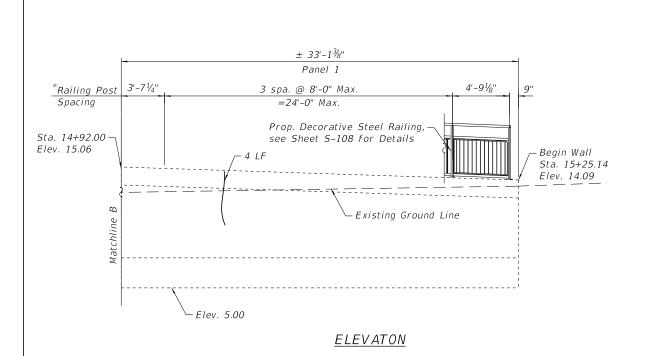
<u>Sheet No.</u> 1660570042

1660570044

1660570051

SOUTHEAST RETAINING WALL PLAN AND ELEVATION II (STRUCTURE NO. 016-6057)

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	COOK	S-106
СДОТ	PROJECT NO. E-1-525		149 of 210



*Contractor shall determine final post spacing. See Sheet S-108.

REFERENCE DRAWINGS

<u>NOTE:</u>

1. For notes, see Sheet S-103.

DrawingSheet No.Survey Layout1660570042General Layout of Substructure1660570044East Approach - Retaining Wall Details and Reinforcement Bars1660570051

<u>LEGEND</u>

— - Epoxy Crack Injection

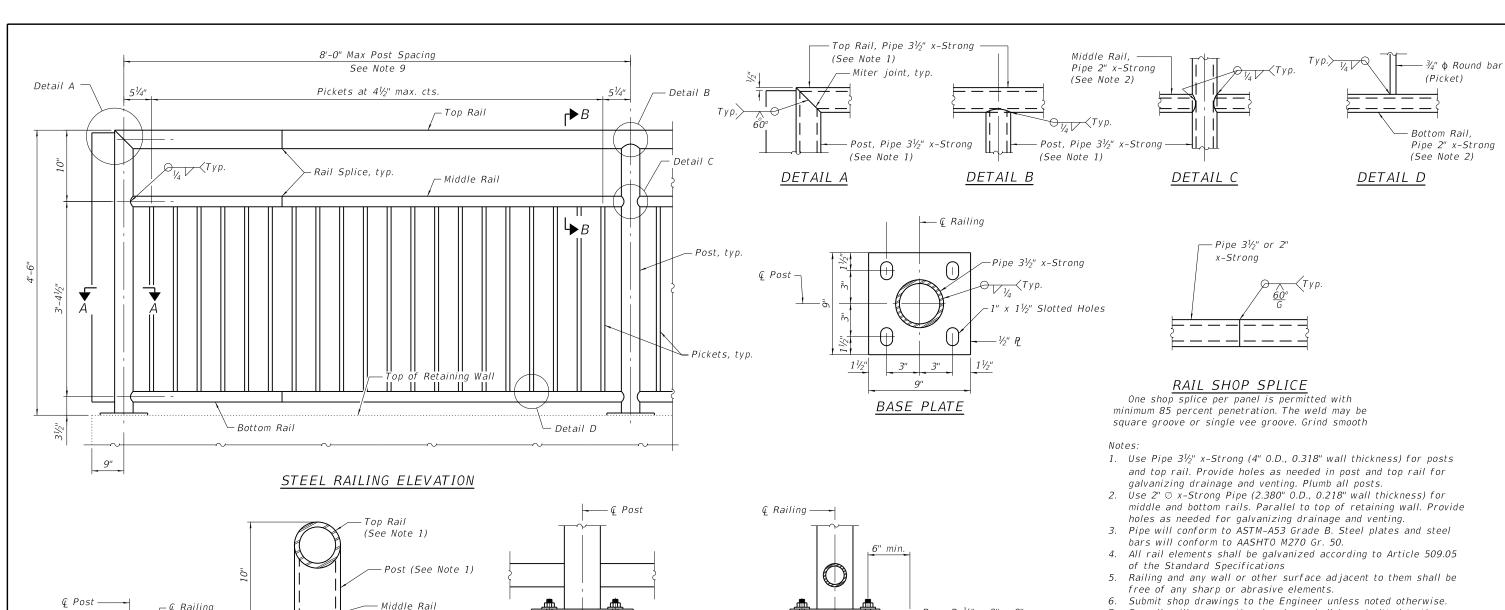
LF - Linear Foot

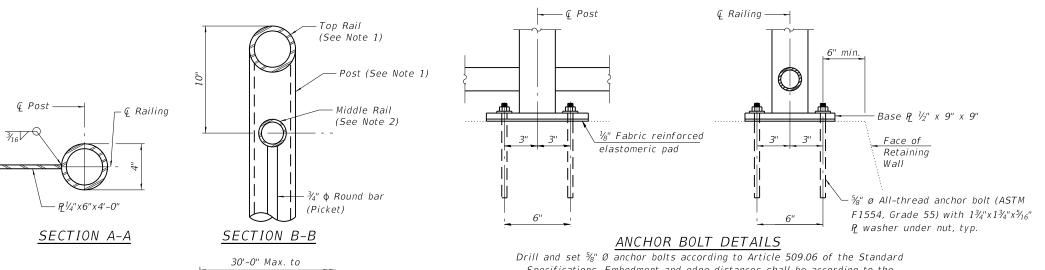


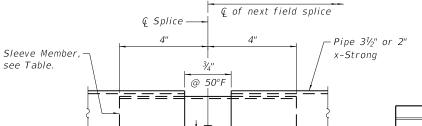


USER NAME =	DESIGNED - AMI	REVISED -
	CHECKED - MI	REVISED -
PLOT SCALE = N.T.S.	DRAWN - AMI	REVISED -
PLOT DATE = \$DATE\$	CHECKED - MAA	REVISED -

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-107
CDOT	PROJECT NO. E-1-525		150 of 210







RAIL FIELD SPLICE

SLEEVE MEMBER

Rail Size	Sleeve Member
3½" x−Strong	$3\frac{1}{4}$ " \oslash MT Pipe $(3\frac{1}{4}$ " 0.D., 0.120" wall thickness)
2" x–Strong	$1^{13}/_{16}$ " \oslash MT Pipe $(1^{13}/_{16}$ " 0.D., 0.120" wall thickness)

Mechanical tubing (MT) will conform to ASTM A513 Grade 1015 or higher.

REVISED

REVISED

REVISED

REVISED

Specifications. Embedment and edge distances shall be according to the manufacturer's specifications. Galvanize upper half of anchor bolt length and hardware in accordance with AASHTO M232. Cost of anchor bolts, hardware, galvanizing, drilling and setting is included in the cost of Decorative Steel Railing.

REFERENCE DRAWINGS

East Approach - Retaining Wall Details and Reinforcement Bars

Sheet No. 1660570051

- For all railings, erection drawings shall be submitted to the Commissioner for approval to ensure proper installation. Drawings shall show field splice locations, railing expansion joints (placed at the existing joint locations in the reinforced concrete wall), post spacing, anchor bolt drilling and setting procedures, profile slope, splice joint locations, shim plates, and railing lengths with identification showing where each railing goes on the layout.
- Railing shall be fabricated such that longitudinal elements are parallel to the top of sidewalk and posts are plumb. Contractor shall field verify sidewalk profile slope.
- All exposed edges will be rounded or chamfered to approximately $\frac{1}{8}$ " by grinding.
- All existing railing removals and wall repairs shall be completed with the existing post connections marked on walls prior to installation of new decorative railings. Any existing anchors not fully removed shall be removed at least 2" below the surface of the new wall top. Installation of new anchors shall not begin until concrete repairs have reached a minimum of 3,000 psi. Contractor shall determine final post spacing to miss location of existing railing post anchor bolts and submit shop drawings to the Commissioner for approval prior to railing Construction.

BILL OF MATERIAL

Item	Unit	Quantity
Decorative Steel Railing	Foot	210
Steel Railing Removal	Foot	206

USER NAME = USERNAME DESIGNED - IJL HECKED - NBR DRAWN - IJL PLOT DATE = 10/5/2020 CHECKED - JIG

 \searrow 1/4" \oslash Pin. Drive fit pin in

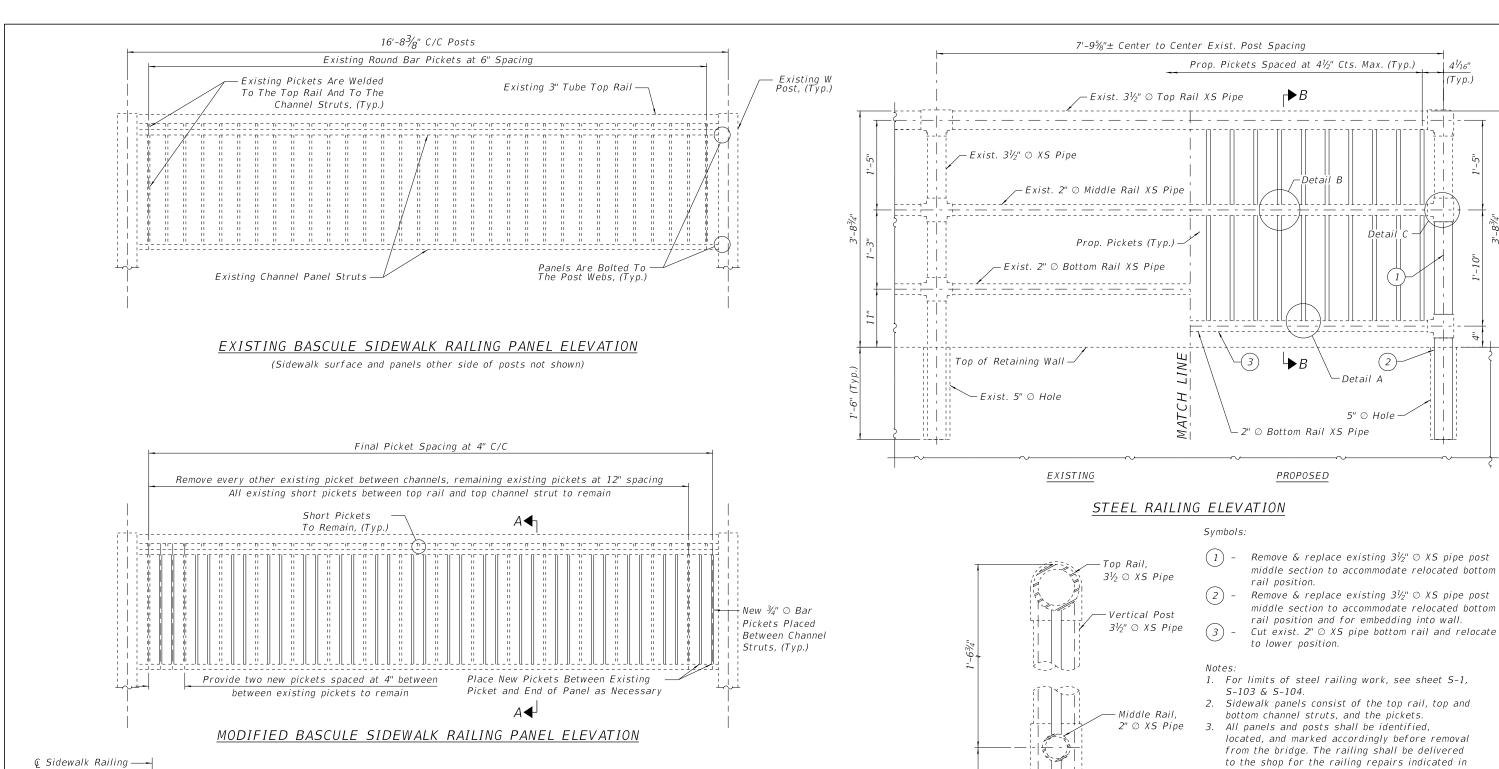
of Sleeve Member.

pre-drilled hole in bottom

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER **EAST RETAINING WALLS RAILING DETAILS** (STRUCTURE NO. 016-6057)

SECTION COUNTY SHEET NO. S-108 11-E1525-00-BR соок CDOT PROJECT NO. E-1-525 151 of 210



¾" **¢** Round bar (Picket) (Typ.)

the specification for the item Steel Railing (Special). 4. Prior to performing the work in this item, the panels shall be unbolted from the posts. Tack welds between the top rail and posts may be present and shall be ground. All welds on the

struts after the picket removals shall be ground. All work to clean railings, modify the panels, furnish and fabricate steel pickets, painting, delivery to and from the shop, reassembling the panels to posts at the same locations, and any other incidental work necessary to complete the work as described in Steel Railing (Special), is included in that item for payment.

Steel Railing Foot 506	Item	Unit	Quantity
(Special)	Steel Railing (Special)	Foot	506

SECTION A-A

SECTION B-B

Item	Unit	Quantity
Steel Railing (Special)	Foot	506



New Steel ¾" ∅ -

Channel Strut, (Typ.) -

Bar Picket

Top Rail -

USER NAME = PJLAUX	DESIGNED - IJL	REVISED -
	CHECKED - NBR	REVISED -
PLOT SCALE = N.T.S.	DRAWN - IJL	REVISED -
PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -

DETAIL A

Exist. Bottom Rail,

2" ∅ XS Pipe

·Prop. ¾" φ Round Typ.>—

Exist. Middle Rail, -

2" ⊘ XS Pipe

bar (Picket)

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

-Prop. ¾" ♦ Round

bar (Picket) (Typ.)

DETAIL B

Middle Rail,

2" ∅ XS Pipe

DETAIL C

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

Vertical Post

Exist. Iron

Fittina

3½" ∅ XS Pipe

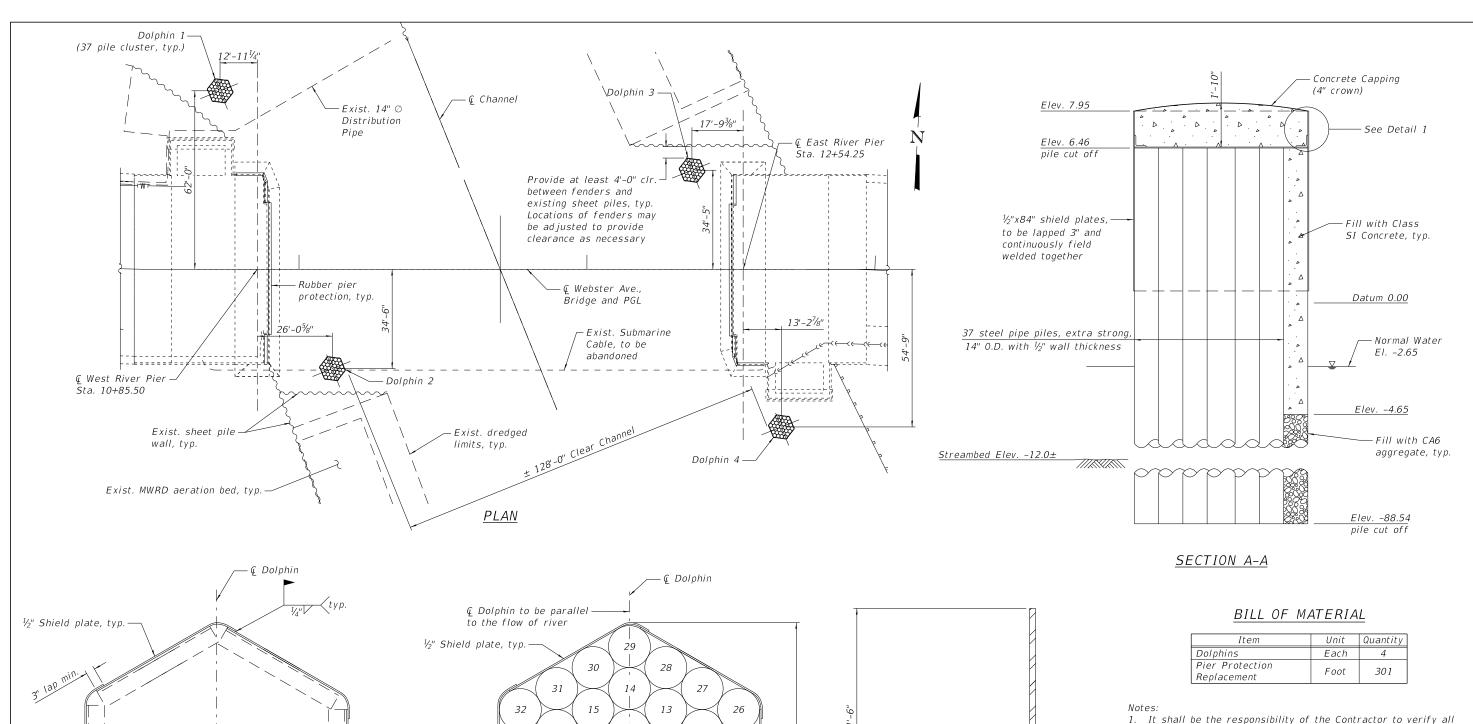
STEEL RAILING DETAILS (STRUCTURE NO. 016-6057)

Bottom Rail,

-Top of Retaining Wall

2" ⊘ XS Pipe

SECTION COUNTY SHEET NO. S-109 11-E1525-00-BR COOK CDOT PROJECT NO. E-1-525 152 of 210



25

24

23

- dimensions and conditions existing in the field prior to construction and ordering of materials.
- See sheet S-111 for the layout plan of the rubber pier protection.
- 5. Existing pier protection and dolphins are heavily deteriorated and not easily visible above the surface of the water. It is the responsibility of the Contractor to locate existing pier protection timber piles and dolphins for removal.
- 6. Pile driving at Dolphin 2 may conflict with existing submarine cables which are to be abandoned in place. Contractor shall plan this work accordingly.

REFERENCE DRAWINGS

<u>Drawing</u> Substructure Pier Protection Substructure Plan Substructure Pier Protection

Sheet No. 1660570043 1660570091 1660570096

DOLPHIN - TOP PLAN

USER NAME = NBROMAN DESIGNED - NBR REVISED CHECKED - IJL REVISED PLOT SCALE = N.T.S. REVISED PLOT DATE = \$DATE\$ CHECKED - JIG REVISED

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

21

±7'-41/4"

PILE DRIVING SEQUENCE

16

18

19

37

34

© Dolphin -

© Dolphin

 $\frac{1}{2}$ " wall, typ.

Concrete capping

- L6x6x¾ typ.

14" Pipe pile with

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

typ. > 1/4 V

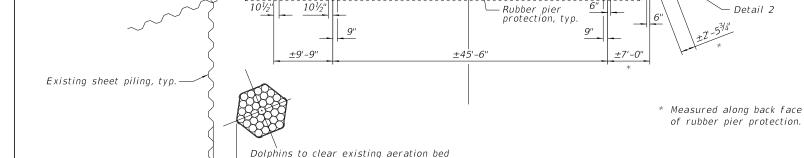
 $L6x6x\frac{3}{4}$ typ.

DETAIL 1

DOLPHINS AND PIER PROTECTION I (STRUCTURE NO. 016-6057)

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	S-110
CDOT	PROJECT NO. E-1-525		153 of 210

- 2. Number on the piles denotes driving sequence.
- Removal of existing dolphins and fenders, and the placement of the new dolphins and rubber pier protection shall be included in the cost of Dolphins. See the Special Provisions.



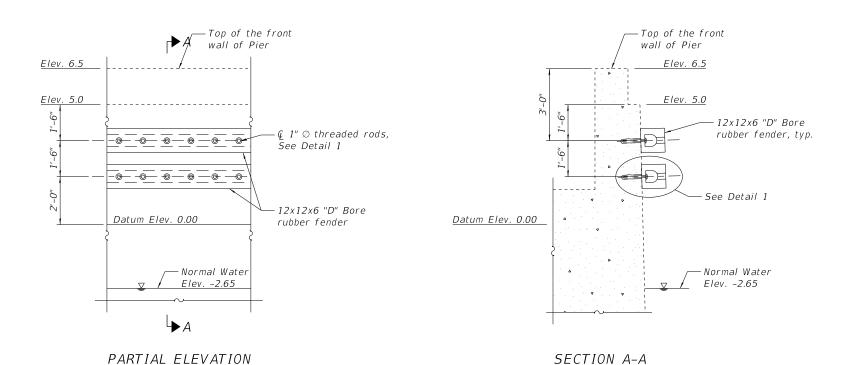
sheet piles by at least 4'-0"

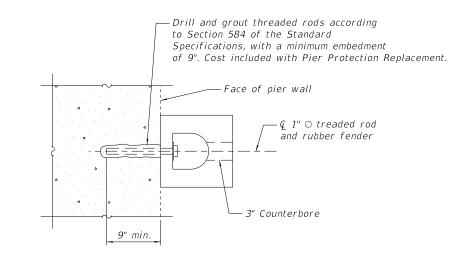
101/2"

101/2

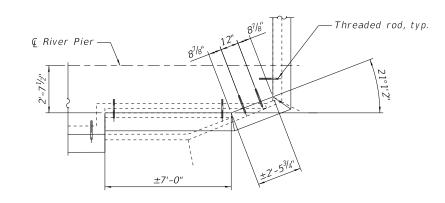
PIER PROTECTION - LAYOUT PLAN

(West Pier shown, East Pier similar)





DETAIL 1



DETAIL 2

(West Pier shown, East Pier similar)

- 1. For Dolphin layout, see sheet S-110.
- 2. The rubber fender shall be a marine type, extruded rubber. Readily available lengths of rubber fender may be used for the long front face of pier as long as threaded rods are provided at 12" spaces and the minimum edge distances shall not be less than 9". Provide no more than 1" clearance between runs of rubber fenders. The costs for furnishing and erecting rubber fenders and all hardware are included with Pier Protection Replacement.
- 3. All hardware, rods, nuts, and plate washers shall be galvanized in accordance with AASHTO M 232.
- 4. The Contractor shall coordinate installation of the rubber pier protection with the structural repair of concrete on the river piers.



USER NAME = NBROMAN DESIGNED - NBR REVISED CHECKED - IJL REVISED REVISED PLOT DATE = \$DATE\$ CHECKED - JIG REVISED

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

dolphin, typ.

Detail 2

DOLPHINS AND PIER PROTECTION II (STRUCTURE NO. 016-6057)

SECTION COUNTY SHEET NO. S-111 11-E1525-00-BR соок CDOT PROJECT NO. E-1-525 154 of 210



SOIL BORING LOG

Page <u>1</u> of <u>4</u>

Date 4/22/15

ROUTE F.A.U. Route 1388 DESCRIPTION						habili	tation-	-Webster Ave over Chicago River LOGGED BY JJR
SECTION 11-E1525-00-B	BR	_ LC	CAT	ON N	orthsi	de of	bridge	Northing 1914745.514 Easting 1165501.335
COUNTY Cook D	RILLING	MET	HOD			Н	SA	HAMMER TYPE AUTO
STRUCT. NO. 016-6057 Station 11+69.88 BORING NO. B-1 Station NA Offset NA			BLOWS 6	C S	M O I S T	DRY DWZW-HY	ORGANIC	Surface Water Elev. 576.54 ft Stream Bed Elev. 567.19 ft Groundwater Elev.: 571.0 ft First Encounter 571.0 ft Upon Completion None ft After NA Hrs. NA ft
Ground Surface Elev. 588.00	ft	(11)	(/6') (tsf)	(%)	(pcf)	(%)	NOTES:
6 inches of Topsoil Black, Wet			<u>''</u>					
FILL: SAND, with gravel, brick and cinders	d		<u>4</u>					
Cinders			5		19			
		-8	∅ 5					
			₩ 7		18			
		<u>-5</u>	5	-				
	582.00	-8						
Brown and Black, Moist FILL: CLAY, with brick, cinders, gravel and wood			3 3 5	2.0 P	15			
		-8						
			3	<u> </u>				
		-10	5 5	2.0 P	20			
		-10		† ·				_
			2					
		-8	2	1.3	18			
		$\overline{}$	4	Р				
Bassas and Once Military	574.00		2	1.5	0.1			
Brown and Gray, Moist to Very Moist		_ -15	1 2	1.5 P	21			
FILL: CLAY, with gravel, and wood		-10						
			 1					
	571.00	V	2	1.8	30			
Brown, Wet FILL: SAND, with wood, concrete		- 🗟	10	P				
fragments		-						
		$\overrightarrow{-}$	0/3	"	20			
		-20			30			
								1

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

Date 4/22/15

tel. 5	12./33.6262 • Ta	X. 312.7	33.30	12						Date <u>4/22/15</u>
ROUTEF.A.U. Ro	oute 1388	_ DE:	SCRI	PTI	ON _	Bridg	ge Re	habilit	ation-	Webster Ave over Chicago River LOGGED BY JJR
SECTION 11-E	E1525-00-BR		_ L	.oc	ATIC	N No	orthsi	de of	oridge	Northing 1914745.514 Easting 1165501.335
COUNTY Cook	DRI	LLING	ME	THC	DD _				SA	HAMMER TYPEAUTO
STRUCT. NO. 0 Station 1 BORING NO.	1+69.88 B-1 NA		D E P T H	GRAPH-C LOG	B L O W S	O C & Qu	M O I S T	AH-WZMD KWD	ORGANIC	Surface Water Elev. 576.54 ft Stream Bed Elev. 567.19 ft Groundwater Elev.: 571.0 ft First Encounter 571.0 ft Upon Completion None ft After NA Hrs. NA ft
Ground Surface Elev.	588.00	_ ft	(ft)	G	(/6")	(tsf)	(%)	(pcf)	(%)	NOTES:
Brown, Wet FILL: SAND, with wood fragments (continued)	•			$\overset{\circ}{\otimes}$	4					
Stiff to Very Stiff Gray, Moist to Very Mo		566.50			6	2.1 B	17			
CLAY, trace gravel (CL	-)					В				_
					2	4.5	00			
			_ - <u>25</u>		3	1.5 P	22			_
					2	1.3	25			
					3	Р				_
			-		1					
			-30		2	1.3 P	25	105.4		
			-30							_
					2					
					2	1.3 B	23			
Very Stiff to Hard Gray, Moist to Very Mo		553.00	-35 _		-	D				
SILTY CLAY, trace gra	vel									
					10					
					12 18	6.0 P	12			
			-40		10	۲				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)

USER NAME =	DESIGNED - PJL	REVISED -	
	CHECKED - IJL	REVISED -	
PLOT SCALE = N.T.S.	DRAWN - IJL	REVISED -	
PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -	



SOIL BORING LOG

Page <u>3</u> of <u>4</u>

Date 4/22/15

ROUTE	F.A.U. Rou	te 1388	_ DES	SCRI	PTI	ON	Bridge Rehabilitation-Webster Ave over Chicago River LOGGED BY JJR					
											Northing 1914745.514 Easting 1165501.335	
COUNTY	Cook	DR	RILLING	ME	THO	DD .			Н	SA	HAMMER TYPEAUTO	
STRUCT. NO Station BORING NO. Station Offset	E	3-1 NA	_	D E P T H	GRAPH-C LOG		U S Qu	M O I S T	41-WZMG 420	ORGAN-C	Surface Water Elev. 576.54 ft Stream Bed Elev. 567.19 ft Groundwater Elev.: 571.0 ft First Encounter None ft Upon Completion None ft After NA Hrs. NA ft	
Ground Sur	_	588.00	ft	(ft)	G	(/6")	(tsf)	(%)	(pcf)	(%)	NOTES:	
Very Stiff to F Gray, Moist to SILTY CLAY, (CL/ML) (con	o Very Mois , trace grave			- - -45		4 7 8	2.5 B	18				
						7 11 14	5.5 P	14				
						4 6 10	2.5 B	20				
			529.00	-60		18 22 20		21				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



SOIL BORING LOG

Page <u>4</u> of <u>4</u>

tel. 312.733.0202 • 1	dx. 512./	33.30	12						Date <u>4/22/15</u>
ROUTE F.A.U. Route 1388	_ DE	_ DESCRIPTION _			Bridg	Bridge Rehabilitation-Webster Ave over Chicago River LOGGED BY JJR			
SECTION 11-E1525-00-B	₹	L	.oc	ATI <u>C</u>	N No	N Northside of bridge Northing 1914745.514 Easting 1165501.335			
COUNTY Cook DF	RILLING	ME.	THC	DD _			Н	SA	HAMMER TYPEAUTO
STRUCT. NO. 016-6057 Station 11+69.88 BORING NO. B-1 Station NA Offset Ground Surface Elev. 588.00	_	D E P T H	GEAPI-C LOG	B ~ O € O ⊢ E	၁၀၈ d ဦ	1 % - 0 M	ה≺יושביים אסם	O-ZPOZO	Surface Water Elev. 576.54 ft Stream Bed Elev. 567.19 ft Groundwater Elev.: First Encounter 571.0 ft Upon Completion None ft After NA Hrs. NA ft
Ground Surface Elev. 588.00 Hard to Very Hard Gray, Moist SILT, trace gravel (ML) (continued)	ft		43	21 46 0/4" 28 23 32	(tsf)	12	(pcf)	(%)	NOTES:

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer) The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206)



USER NAME =	DESIGNED - PJL	REVISED -
	CHECKED - IJL	REVISED -
PLOT SCALE = N.T.S.	DRAWN - PJL	REVISED -
PLOT DATE = \$DATE\$	CHECKED - JIG	REVISED -

GENERAL NOTES - DEMOLITION

- THE CONTRACT IS TO PERFORM THE DEMOLITION SCOPE OF WORK AS REQUIRED TO COMPLETE NEW CONSTRUCTION. COORDINATE ARCHITECTURAL DEMOLITION WITH CIVIL, STRUCTURAL, MECHANICAL AND ELECTRICAL DEMOLITION SCOPE OF WORK.
- THE CONTRACTOR MUST VISIT THE SITE AND BE KNOWLEDGEABLE OF ALL CONDITIONS. HE/SHE
 MUST INVESTIGATE, VERIFY, AND BE RESPONSIBLE FOR ALL CONDITIONS OF THE PROJECT AND
 MUST NOTIFY THE COMMISSIONER OF ANY CONDITIONS REQUIRING MODIFICATION BEFORE
 DEDOCEDING WITH THE WORK.
- THE CONTRACTOR MUST PERFORM ALL WORK IN ACCORDANCE WITH ALL APPLICABLE CITY OF CHICAGO, COUNTY, STATE, AND FEDERAL LAWS AND REGULATIONS.
- 4. DUMPSTER PLACEMENT LOCATIONS MUST BE APPROVED BY THE COMMISSIONER.
- TO THE BEST OF COMMISSIONER'S KNOWLEDGE, THESE DRAWINGS REPRESENT THE EXTENT OF THE EXISTING CONDITIONS. EXISTING CONSTRUCTION TO BE REMOVED, RELOCATED, OR REMAIN SHALL BE VERIFIED AT THE SITE BY THE CONTRACTOR AND COMMISSIONER.
- 6. THE CONTRACTOR MUST NOTIFY THE COMMISSIONER IN WRITING OF ANY DISCREPANCIES ON THE DRAWING OR THE UNCOVERING OF HIDDEN CONDITIONS WHICH MAY AFFECT THE WORK,
- ITEMS OF CONSTRUCTION SHOWN AND NOTED TO BE REMOVED SHALL REPRESENT ALL SIMILAR
 CONDITIONS AND CONSTRUCTION UNLESS NOTED OTHERWISE.
- 8. DO NOT REMOVE OR ALTER ANY EXISTING STRUCTURAL MEMBER OR PORTION OF THE STRUCTURAL FLOOR SYSTEM UNLESS SPECIFICALLY NOTED OR SHOWN ON THE CONTRACT DOCUMENTS.
- 9. THE CONTRACTOR MUST NEATLY SAWCUT ALL CONCRETE WALLS AND CONCRETE FLOORS IN A CLEAN AND STRAIGHT MANNER.
- 10. CONTRACTOR MUST PROVIDE CONTAINMENT INCLUDING CONSTRUCTION BARRICADES OR OTHER DUST COLLECTION METHODS TO PREVENT DUST GENERATED FROM DEMOLITION OR CONSTRUCTION FROM ENTERING THE PUBLIC AREAS.
- 11. CONTRACTOR MUST PAY ALL APPLICABLE PERMIT FEES AND COSTS RELATED TO REMOVAL AND INSTALLATION.
- 12. ANSI A-10.6 "SAFETY REQUIREMENTS FOR DEMOLITION" WILL GOVERN EXCEPT AS OTHERWISE MODIFIED HEREIN. WHERE THE REQUIREMENTS SPECIFIED HEREIN OR CONTAINED IN THE ANSI STANDARD DIFFER FROM OTHER APPLICABLE RULES, REGULATIONS, AND CODES, THE MORE STRINGENT REQUIREMENTS SHALL GOVERN THE WORK UNDER CONTRACT.
- 13. BEFORE COMMENCING ANY WORK, SUBMIT TO THE COMMISSIONER FOR REVIEW A SCHEDULE SHOWING THE COMMENCEMENT OF WORK, THE ORDER, AND THE COMPLETION DATES FOR THE VARIOUS PARTS OF THIS WORK. THE CONTRACTOR MUST OBTAIN COMMISSIONER APPROVAL IN WRITING BEFORE PROCEEDING WITH THIS WORK.
- 14. PROVIDE, ERECT, AND MAINTAIN TEMPORARY WORK INCLUDING, BUT NOT LIMITED TO BARRICADES, WARNING SIGNS, ETC. AS REQUIRED FOR PROPER PROTECTION OF THE PUBLIC AND PROPERTY DURING REMOVAL OPERATIONS.
- 15. THE CONTRACTOR WILL BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PHASING OR PROCEDURES AND SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK. ALL WORK SHALL MEET THE QUALITY REQUIREMENTS NOTED IN THE CONTRACT DOCUMENTS.
- 16. COMMISSIONER WILL NOT BE RESPONSIBLE FOR NOR WILL HAVE CONTROL OVER OR CHARGE OF CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PHASING OR PROCEDURES, SAFETY PRECAUTIONS, AND PROGRAMS IN CONNECTION WITH THE WORK, NOR WILL THEY BE RESPONSIBLE FOR THE CONTRACTOR'S FAILURE TO CARRY OUT THE WORK IN ACCORDANCE WITH THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. COMMISSIONER WILL NOT BE RESPONSIBLE FOR, OR HAVE CONTROL OR CHARGE OVER THE ACTS OR OMISSIONS OF THE CONTRACTOR, SUBCONTRACTORS, THEIR AGENTS OR EMPLOYEES, OR ANY OTHER PERSON PERFORMING ANY OF THE WORK.
- 17. ALL DEMOLITION INDICATED TO BE REMOVED MUST BE REMOVED CAREFULLY. ANY ADJACENT MATERIAL DAMAGED SHALL BE REPLACED IN KIND AT THE CONTRACTOR'S EXPENSE TO THE SATISFACTION OF THE COMMISSIONER.
- 18. MATERIAL AND/OR ITEMS REMOVED AND NOT DESIGNATED TO BECOME THE PROPERTY OF THE COMMISSIONER WILL BECOME THE PROPERTY OF THE CONTRACTOR AND REMOVED FROM THE JOB SITE. DELIVER ALL ITEMS DESIGNATED AS "SALVAGE" OR "REMIT" TO STORAGE SITE DESIGNATED BY COMMISSIONER. COORDINATE DELIVERY WITH COMMISSIONER.
- 19. ANY EXISTING COMPONENT REMOVED OR DAMAGED TO ALLOW NEW WORK TO OCCUR MUST BE REINSTALLED OR REPLACED AT THE COMPLETION OF THE NEW WORK UNLESS NOTED OTHERWISE.
- 20. NO BURNING OF MATERIAL ON THE PREMISES WILL BE PERMITTED. COMBUSTIBLE FUELS ARE PROHIBITED.
- 21. UPON COMPLETION OF REMOVAL WORK, REMOVE ALL TOOLS, MATERIALS, APPARATUS, AND

- RUBBISH OF ANY SORT. THE PORTION OF THE JOB SITE THAT CAN BE SEEN BY THE PUBLIC MUST BE
- REFER TO CONTRACT DOCUMENTS PLANS AND DETAILS INCLUDING APPLICABLE STRUCTURAL, CIVIL, MECHANICAL AND ELECTRICAL SCOPE OF WORK FOR EXISTING CONDITIONS AND NEW WORK COORDINATION.
- 23. NOTIFY COMMISSIONER IF ANY MATERIAL IS FOUND WHICH IS NOTED TO BE A HAZARDOUS MATERIAL. 13.
- 24. THE FOLLOWING ITEMS/EQUIPMENT/SYSTEMS ARE TO BE REMOVED FROM THE PROJECT AND DELIVERED TO A SITE AS INDICTED BY THE COMMISSIONER UNLESS NOTED OTHERWISE:
- ALL BRIDGE EQUIPMENT AT OPERATOR'S LEVEL THAT IS NOT GOING TO BE REUSED.
- 25. THE FOLLOWING AREAS ARE TO BE CLEANED:
- COMPLETE CLEANING OF BRIDGE HOUSES, COST INCLUDED IN THE LUMP SUM PRICE FOR BRIDGE HOUSE PAY ITEMS.

GENERAL NOTES - SCOPE OF WORK

THE ARCHITECTURAL SCOPE OF WORK AND ASSOCIATED PAY ITEMS GENERALLY CONSISTS OF THE REHABILITATION OF THE BRIDGE HOUSES AND ABUTMENTS CLOSE TO THEIR ORIGINAL HISTORICAL CONDITION AND THE REPLACEMENT OF BRIDGE RAILINGS AND GUARDRAILS WITH HISTORIC RAILINGS AS INDICATED ON THE DRAWINGS.

GENERAL NOTES - CONSTRUCTION

- THE COMMISSIONER EXPRESSLY DISCLAIMS ANY RESPONSIBILITY ARISING FROM ANY UNAUTHORIZED USE OF THESE DRAWINGS, PLANS, AND NOTES. ANY AUTHORIZATIONS MUST BE IN WRITING. THESE DRAWINGS MAY HAVE BEEN REPRODUCED AT A SIZE DIFFERENCE THAT ORIGINALLY DRAWN.
- 2. THE COMMISSIONER WILL NOT BE RESPONSIBLE FOR NOR WILL HAVE ANY CONTROL OVER OR CHARGE OF CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PHASING, OR PROCEDURES OF SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK, NOR WILL BE RESPONSIBLE FOR THE CONTRACTOR'S FAILURE TO CARRY OUR THE WORK IN ACCORDANCE WITH THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. THE COMMISSIONER WILL NOT BE RESPONSIBLE FOR OR HAVE ANY CONTROL OR CHARGE OVER THE ERRORS OR OMISSIONS OF THE CONTRACTOR, SUBCONTRACTORS, THEIR AGENTS, EMPLOYEES, OR ANY OTHER PERSON PERFORMING ANY OF THE WORK.
- 3. THE CONTRACTOR SHALL VISIT THE SITE AND BE KNOWLEDGEABLE OF ALL CONDITIONS THEREON. THE CONTRACTOR SHALL INVESTIGATE, VERIEY, AND BE RESPONSIBLE FOR ALL CONDITIONS OF THE PROJECT AND SHALL NOTIFY THE COMMISSIONER OF ANY CONDITIONS REQUIRING MODIFICATION OR CLARIFICATION BEFORE PROCEEDING WITH THE WORK. NOTES APPEAR ON VARIOUS SHEETS FOR DIFFERENT SYSTEMS AND MATERIALS: NOTES REFER TO RELATED STRUCTURAL/MEP AND DETAIL DRAWINGS. THESE SHEETS ARE TO BE REVIEWED AND NOTES ON ANY ONE SHEET ARE TO BE APPLIED ON RELATED DRAWINGS AND DETAILS. WHERE DISCREPANCIES EXIST BETWEEN THE DRAWINGS OF VARIOUS DISCIPLINES, CONSULT THE COMMISSIONER DURING THE BID PERIOD AND PRIOR TO PROCEEDING WITH THE WORK.
- 4. DRAWINGS THAT REPRESENT THE EXISTING PLAN CONDITIONS ARE DIAGRAMMATICALLY SHOWN. EXACT LOCATIONS, SIZES, EXTENT, AND CONDITIONS OF EXISTING CONSTRUCTION TO BE REMOVED, RELOCATED OR TO REMAIN SHALL BE VERIFIED AT THE SITE BY THE CONTRACTOR.
- 5. THE CONTRACTOR MUST PROVIDE ALL REQUIRED PERMITS. THE CONTRACTOR WILL PERFORM ALL WORK IN ACCORDANCE WITH ALL APPLICABLE CITY OF CHICAGO, COUNTY, STATE, AND FEDERAL LAWS, CODES, ORDINANCES, AND REGULATIONS BY MUNICIPAL AUTHORITIES HAVING JURISDICTION, INCLUDING THE OCCUPATIONAL SAFETY AND HEALTH ACT (OSHA) AND REGULATIONS ADOPTED PURSUANT THERETO
- ALL FIRE RATINGS INDICATED FOR WALLS, CEILINGS, AND ROOF ARE TO COMPLY WITH UNDERWRITERS LABORATORIES TEST RATINGS OR AS REGULATED BY CITY OF CHICAGO BUILDING CODE AND NFPA 130 FIXED GUIDE WAY TRANSIT SYSTEMS.
- 7. ANY DETAILS, SYSTEMS, OR MATERIALS (I.E. ARCHITECTURAL, STRUCTURAL, MECHANICAL, ETC.) WHICH ARE PROPOSED TO BE CHANGED OR SUBSTITUTED MUST BE FIRST REVIEWED AND APPROVED BY THE COMMISSIONER PRIOR TO THE PREPARATION AND SUBMITTAL OF SHOP DRAWINGS. THE COMMISSIONER RESERVES THE RIGHT TO REJECT SUBSTITUTIONS. THE CONTRACTOR IS RESPONSIBLE FOR ADDED JOB COSTS DUE TO HIS SUBSTITUTIONS IMPACTED ON OTHER TRADES.
- THE CONTRACTOR MUST BE RESPONSIBLE FOR PROVIDING ALL REQUIRED BLOCKING, SUPPORTS, AND BRACING REQUIRED TO ACHIEVE SPECIFIED REQUIREMENTS AND STANDARDS WHETHER SPECIFICALLY INDICATED OR NOT. FIRE RETARDANT BLOCKING MUST BE PROVIDED.
- ALL NEW GUARDRAILS AND HANDRAILS MUST MEET OSHA, NIOSH, AND ADA GUIDELINES, 200 PSF LIVE LOAD SAFETY CODE HORIZONTAL AND VERTICAL HANDRAIL REQUIREMENTS, UNLESS NOTED OTHERWISE.
- 10. ALL DISSIMILAR METALS MUST BE EFFECTIVELY ISOLATED FROM EACH OTHER TO AVOID GALVANIC ACTION.
- DETAILS SHOWN ARE INTENDED TO BE INDICATIVE OF THE PROFILES AND TYPE OF DETAILING REQUIRED FOR THE WORK. CONDITIONS ARE NOT COVERED BY SPECIFIC DETAILS AND MUST MEET

SPECIFIED DESIGN CRITERIA.

- 12. DETAILS SHOWN MAY NOT NECESSARILY GRAPHICALLY REPRESENT ALL COMPONENTS NECESSARY TO COMPLETE THE TOTAL SYSTEM. THE CONTRACTOR MUST BE RESPONSIBLE TO COORDINATE ALL OF THE REQUIREMENTS SHOWN ON THE DRAWINGS WITH THOSE STATED IN THE APPLICABLE SPECIFICATIONS AND PROJECT NOTES TO PROVIDE A COMPLETE SYSTEM.
- 13. THE CONTRACTOR MUST COORDINATE ALL MECHANICAL AND ELECTRICAL FLOOR AND WALL SLEEVES, FLOOR PENETRATIONS, EMBEDDED CONDUIT AND MECHANICAL DUCTWORK WITH MECHANICAL, PLUMBING, FIRE PROTECTION, ELECTRICAL, STRUCTURAL, AND ARCHITECTURAL DISCIPLINES.
- 14. PROVIDE ACCESS PANELS AS REQUIRED BY APPLICABLE CODES AND AS REQUIRED FOR MECHANICAL AND ELECTRICAL EQUIPMENT. COORDINATE LOCATIONS WITH COMMISSIONER PRIOR TO INSTALLATION.
- 15. ALL PIPES, CONDUITS, AND DUCTWORK THAT PENETRATE WALLS OR ROOF SLABS SHALL BE INSTALLED IN A MANNER THAT WILL PRESERVE THE FIRE RESISTIVE, STRUCTURAL INTEGRITY, AND WATER-TIGHTNESS OF SUCH WALLS OR SLABS AND THE BUILDING. FIRESTOP ALL PENETRATIONS IN WALLS TO MATCH HOURLY RATING OF WALL.
- 16. DETAILS NOT SHOWN ARE SIMILAR IN CHARACTER TO THOSE SHOWN: WHERE SPECIFIC DIMENSIONS, DETAILS, OR DESIGN INTENT CANNOT BE DETERMINED, CONSULT THE COMMISSIONER BEFORE PROCEEDING WITH WORK.
- 17. CONTRACTOR TO REMOVE ANY GRAFFITI WITHIN 24 HOURS DURING CONSTRUCTION.
- 18. REFER TO THE TECHNICAL SPECIFICATIONS FOR ADDITIONAL INFORMATION AND COORDINATION OF THE WORK. IN THE EVENT OF DISCREPANCIES BETWEEN DRAWINGS OR DRAWINGS AND SPECIFICATIONS, CONSULT THE COMMISSIONER FOR CLARIFICATION BEFORE PROCEEDING WITH THE WORK
- 19. THE CONTRACTOR MUST ASSURE ITSELF BY INDEPENDENT SURVEY THAT NO HAZARDOUS MATERIALS ARE HANDLED OR DISTRIBUTED. IF HAZARDOUS MATERIALS ARE ENCOUNTERED, IT MUST BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE COMMISSIONER.
- THE CONTRACTOR WILL BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PHASING, PROCEDURES, SAFETY PRECAUTIONS, AND PROGRAMS IN CONNECTION WITH THE WORK. ALL WORK SHALL MEET THE QUALITY REQUIREMENTS NOTED IN THE CONTRACT DOCUMENTS.
- 21. THE CONTRACTOR IS TO REVIEW AND HAVE A COMPLETE COMMAND OF THE VARIOUS PHASES OF THE CONSTRUCTION WORK PLUS A TOTAL UNDERSTANDING OF THE UNIQUE MANPOWER AND SUPPLY SCHEDULE REQUIREMENTS SPECIFIC TO THE SITE. THE CONTRACTOR MUST COORDINATE ALL CONSTRUCTION RELATED ACTIVITIES WITH BRIDGE LIFT SCHEDULES, OPERATION REQUIREMENTS. AND RESULTING HAZARDOUS CONDITIONS.
- 22. THE CONTRACTOR MUST BE RESPONSIBLE FOR SECURING THE JOB SITE AND THE PROTECTION OF THE GENERAL PUBLIC FROM ALL POSSIBLE HAZARDS OF THE ENTIRE CONSTRUCTION SITE.
- 23. THE GENERAL CONTRACTOR AND/OR TRADE CONTRACTOR MUST PROTECT ALL EXISTING SITE ELEMENTS FROM DAMAGE DUE TO ALTERATIONS AND CONSTRUCTION OPERATIONS, AND REPAIR OR REPLACE ELEMENTS DAMAGED DURING THIS PROJECT AT HIS/HER OWN EXPENSE.
- 24. DRAWINGS ARE TO BE ISSUED TO THE SUBCONTRACTORS BY THE CONTRACTOR IN COMPLETE SETS SO THAT THE EXTENT AND COORDINATION OF WORK IS MADE POSSIBLE.
- THE CONTRACTOR MUST BE SOLELY RESPONSIBLE FOR THE ACCURATE PLACEMENT AND CONDITIONS OF THE WORK.
- THE CONTRACTOR IS TO PROTECT, MOVE, AND STORE THE OWNER'S FIXTURES, FURNISHINGS, AND EQUIPMENT.

DIMENSION NOTES

- ALL DIMENSIONS ARE FACE OF PARTITIONS, COLUMN CENTERLINE, OR FINISHED FACE OF EXTERIOR WALL UNLESS NOTED OTHERWISE.
- ALL DIMENSIONS SHALL BE VERIFIED IN THE FIELD BEFORE PROCEEDING WITH ANY WORK. THE COMMISSIONER SHALL BE NOTIFIED OF ANY DISCREPANCIES OR CORRECTIONS. DIMENSIONS OF NEW WORK ARE BASED ON RECORD DRAWING OF EXISTING WORK. ANY DISCREPANCIES IN EXISTING WORK THAT WILL AFFECT NEW WORK DIMENSIONS, NOTIFY THE COMMISSIONER. CONTRACTOR TO VERIFY EXISTING CONDITIONS.
- ALL DIMENSIONS, ELEVATIONS, REPRESENTATIONS OF THE SITE, EXISTING CONDITIONS, AND AS-BUILT DRAWINGS ARE BASED ON INFORMATION OBTAINED FROM COMMISSIONER. SOME VARIATIONS BETWEEN DRAWINGS AND FIELD CONDITIONS MUST BE ANTICIPATED AND ALLOWED FOR.
- 4. IT IS IMPERATIVE THAT THE CONTRACTOR FULLY FAMILIARIZE ITSELF WITH ALL EXISTING SITE CONDITIONS, SURVEYS AND REPORTS. THE CONTRACTOR MUST FIELD VERIFY ALL DIMENSIONS AND CONDITIONS SHOP OR DESCRIBED TO BE EXISTING BY ITS OWN FIELD SURVEY PRIOR TO THE START OF SHOP DRAWINGS PREPARATION, FABRICATION, OR SITE CONSTRUCTION. NOTIFY THE COMMISSIONER OF DISCREPANCIES IN WRITING BEFORE PROCEEDING WITH WORK.

THE EXISTING COLUMN-TO-COLUMN SPACING VARIES. THE CONTRACTOR MUST CONDUCT ITS OWN
FIELD SURVEY TO ESTABLISH EXISTING DIMENSIONAL PARAMETERS AS RELATED TO THE DESIGN
BEFORE PROCEEDING WITH THE WORK. NOTIFY THE COMMISSIONER IN WRITING OF DISCREPANCIES
WHICH WILL AFFECT EXECUTION OF THE DESIGN AS SHOWN.

PATCHING AND PAINTING NOTES

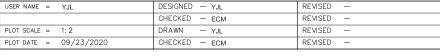
- ALL EXISTING AREAS DAMAGED BY WORK OF THIS PROJECT SHALL BE PATCHED TO MATCH EXISTING ADJACENT SURFACES IN FINISH. COLOR. AND TEXTURE UNLESS OTHERWISE INDICATED.
- ALL EXISTING PAINTED SURFACES DAMAGED OR PATCHED AS PART OF THIS PROJECT SHALL BE
 PAINTED TO MATCH EXISTING ADJACENT COLOR, TEXTURE, AND FINISH. EXISTING PAINT COLOR,
 TEXTURE AND FINISH SHALL BE VERIFIED BY CONTRACTOR IN THE FIELD PRIOR TO CONSTRUCTION
- NEWLY INSTALLED EXPOSED ELECTRICAL ITEMS, INCLUDING BUT NOT LIMITED TO, CONDUIT HANGERS, AND FITTINGS SHALL BE PAINTED TO MATCH EXISTING ADJACENT SURFACES.
- 4. PREFINISHED ITEMS, SUCH AS STAINLESS STEEL, ARE NOT TO BE PAINTED.
- CONTRACTOR IS RESPONSIBLE FOR REMOVAL OF ALL ANCHOR BOLTS, CONDUIT STUBS, AND RELATED PROJECTIONS AND APPURTENANCES FROM FLOOR, WALL, AND CEILING AREAS WHERE ITEMS ARE REMOVED. REMOVE ALL ITEMS TO BELOW FINISH SURFACE PRIOR TO FLOOR PATCHING.
- ALL EXISTING FLOOR SURFACES DAMAGED OR PATCHED AS PART OF THIS PROJECT SHALL BE PATCHED TO MATCH EXISTING ADJACENT COLOR, TEXTURE, AND FINISH. EXISTING FLOOR FINISH SHALL BE VERIFIED BY CONTRACTOR IN THE FIELD PRIOR TO CONSTRUCTION.
- ALL NEW AND EXISTING EXPOSED CONCRETE WALLS TO BE CLEANED AND PAINTED. OR AS INDICATED IN THE DRAWINGS AND SPECIFICATIONS.

GENERAL NOTES - HAZARDOUS MATERIALS

ASBESTOS-CONTAINING BUILDING MATERIALS AND LEAD-BASED PAINT ARE, OR MAY BE, PRESENT IN THIS PROJECT. SEE ENVIRONMENTAL PAY ITEMS AND SPECIFICATION SECTIONS PRIOR TO DISTURBING. NO PERSON MAY DISTURB ASBESTOS-CONTAINING BUILDING MATERIALS UNLESS THAT PERSON IS A LICENSED ASBESTOS WORKER AND CONDUCTS SUCH WORK IN ACCORDANCE WITH SPECIFICATION(S) CONTAINED IN THE PROJECTS DOCUMENTS AND IN COMPLIANCE WITH ILLINOIS DEPARTMENT OF PUBLIC HEALTH RULES AND REGULATIONS.





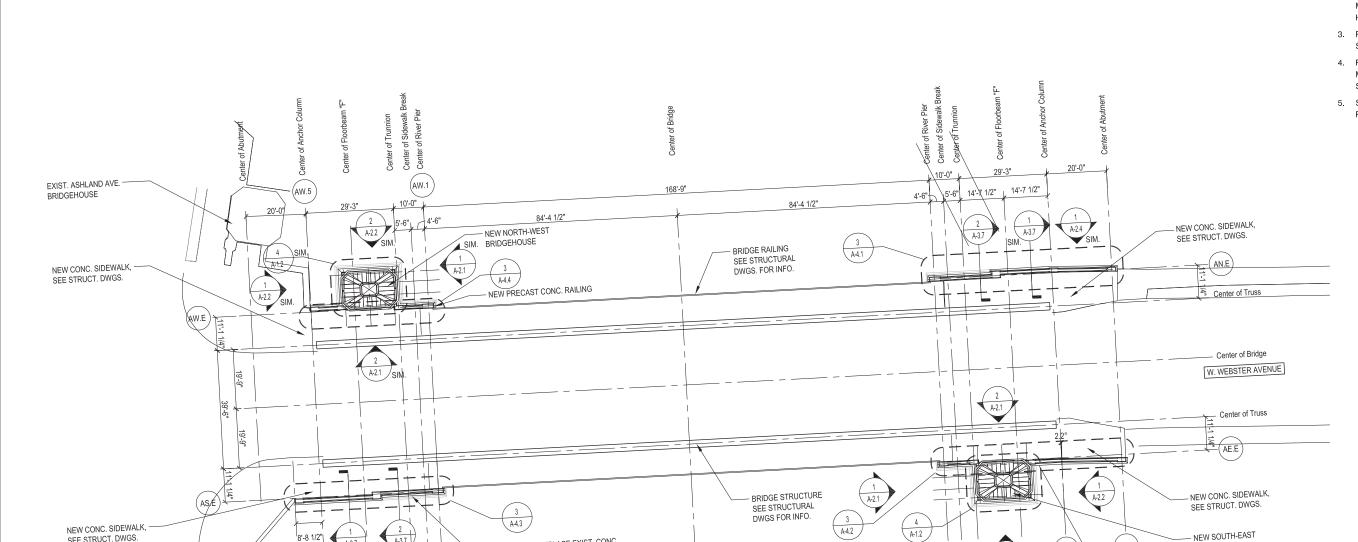


CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

ARCHITECTURAL GENERAL NOTES
(STRUCTURE NO. 016-6057)

| SECTION | COUNTY | SHEET NO. | 1388 | 11-E1525-00-BR | COOK | A-0.0 | CDOT PROJECT NO. E-1-525 | 157 of 210



REMOVE AND REPLACE EXIST. CONC.
OR MISC. RAILINGS WITH NEW
PRECAST CONC. RAILING. TYP. ALL
(4) SIDES OF THE BRIDGE

SHEET NOTES

WEBSTER AVE. BRIDGEHOUSES & RAILINGS

- 1. CONTRACTOR TO VERIFY ALL DIMENSIONS IN THE FIELD
- 2. FIELD VERIFY EXISTING CONDITION, MAINTAIN ALL CLEARANCES AND HOLD ALL DIMENSIONS
- 3. REMOVE EXISTING BRIDGEHOUSE, SEE STRUCT. AND CIVIL DWGS.
- 4. REMOVE EXISTING CONCRETE AND MISCELLANEOUS RAILINGS, SEE STRUCT. AND CIVIL DWGS.
- 5. SEE SHEET A-1.3 FOR LOWER LEVEL PLAN



SEE STRUCT. DWGS.



USER NAME = YJL	DESIGNED — YJL	REVISED —	
	CHECKED — ECM	REVISED —	
PLOT SCALE = 1:2	DRAWN — YJL	REVISED —	
PLOT DATE = $09/23/2020$	CHECKED — ECM	REVISED —	

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

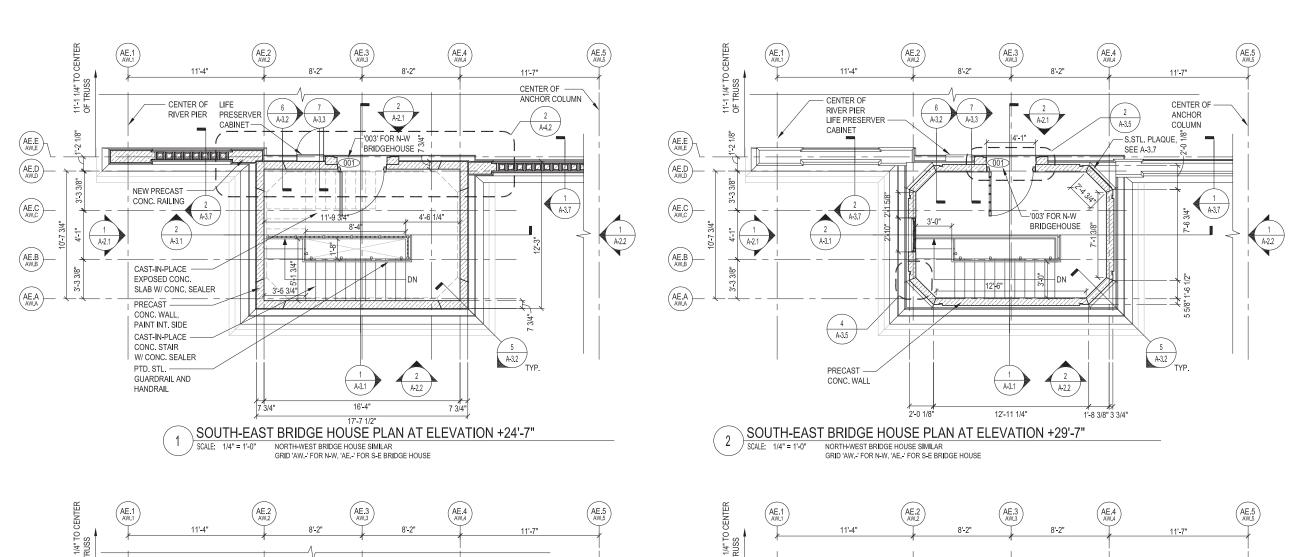
OVERALL SITE PLAN (STRUCTURE NO. 016-6057)

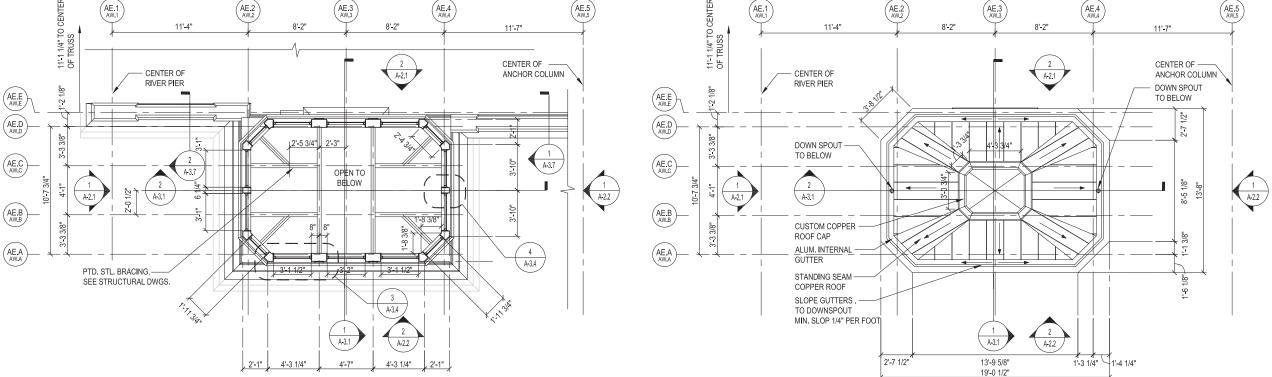
(AE.5

2 A-2.2

BRIDGEHOUSE

COUNTY SHEET NO. SECTION A-1:1 11-E1525-00-BR CDOT PROJECT NO. E-1-525 158 of 210







3 SOUTH-EAST BRIDGE HOUSE PLAN AT ELEVATION +38'-1"

NORTH-WEST BRIDGE HOUSE SIMILAR GRID 'AW.-' FOR N-W, 'AE.-' FOR S-E BRIDGE HOUSE

USER NAME = YJL	DESIGNED — YJL	REVISED —	CITY OF CHICAGO
	CHECKED — ECM	REVISED —	
PLOT SCALE = 1:2	DRAWN — YJL	REVISED —	DEPARTMENT OF TRANSPORTATION
PLOT DATE = 09/23/2020	CHECKED — ECM	REVISED -	DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

BRIDGE HOUSE PLAN
(STRUCTURE NO. 016-6057)

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	COOK	A-1.2
CDOT	PROJECT NO. E-1-525		159 of 210

SHEET NOTES
WEBSTER AVE. BRIDGEHOUSES & RAILINGS

1. CONTRACTOR TO VERIFY ALL
DIMENSIONS IN THE FIELD

2. FIELD VERIFY EXISTING CONDITION, MAINTAIN ALL CLEARANCES AND

3. REMOVE EXISTING BRIDGEHOUSE,

4. REMOVE EXISTING CONCRETE AND

MISCELLANEOUS RAILINGS, SEE

HOLD ALL DIMENSIONS

SEE STRUCT. DWGS.

5. SEE SHEET A-6.1 FOR MORE

INFORMATION ON PRECAST

CONCRETE WALL PANELS.

6. ALL ELEVATIONS ARE CITY OF

CHICAGO DATUM (CCD)

7. PROVIDE A PAINTED GYP. BD.

ABOVE

CEILING AT ELEV +42'-3" CCD.,

SUSPENDED FROM STRUCTURE

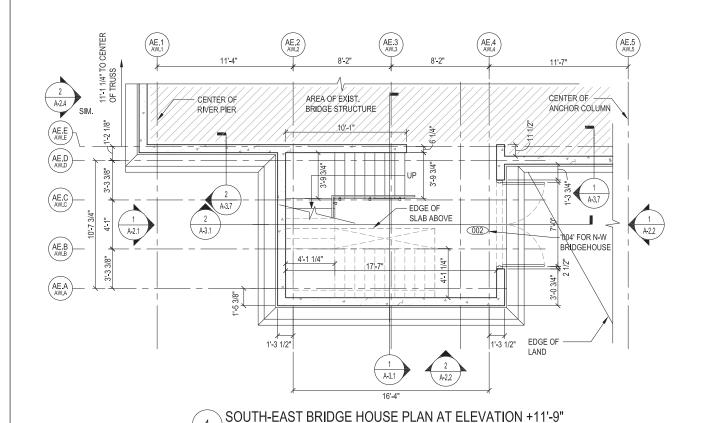
STRUCT. DWGS.

SOUTH-EAST BRIDGE HOUSE ROOF PLAN

SCALE: 1/4" = 1'-0"

' NORTH-WEST BRIDGE HOUSE SIMILAR GRID 'AW.-' FOR N-W, 'AE.-' FOR S-E BRIDGE HOUSE

 WSP USA Inc.	USER NAI
30 N. LASALLE STREET SUITE 4200	
CHICAGO, IL 60602 TEL: (312) 782-8150	PLOT SC
 FAX: (312) 782-1684	PLOT DAT



NORTH-WEST BRIDGE HOUSE SIMILAR GRID 'AW.-' FOR N-W, 'AE.-' FOR S-E BRIDGE HOUSE

SCALE: 1/4" = 1'-0"

SHEET NOTES

WEBSTER AVE. BRIDGEHOUSES & RAILINGS

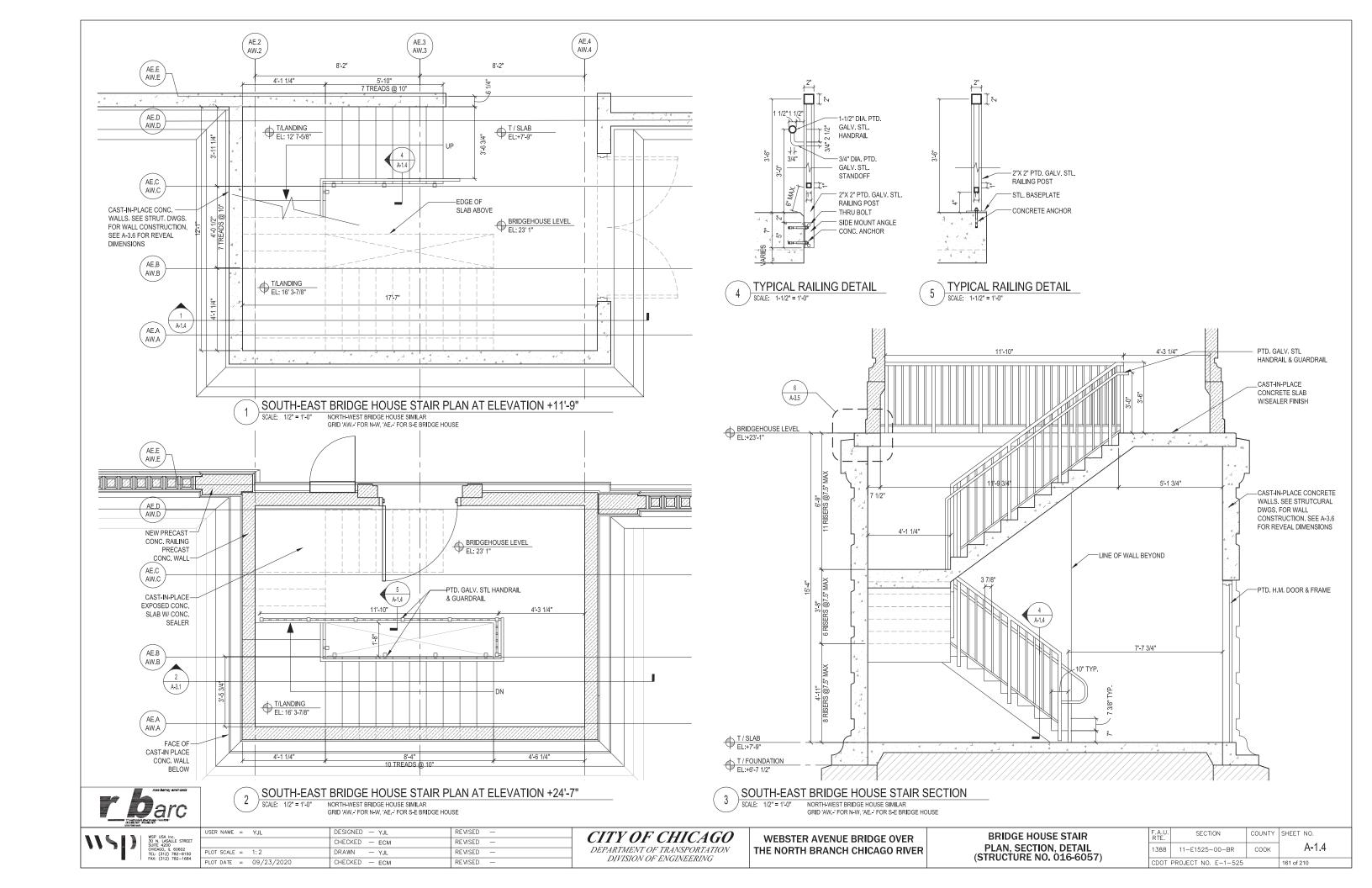
- CONTRACTOR TO VERIFY ALL DIMENSIONS IN THE FIELD
- FIELD VERIFY EXISTING CONDITION,
 MAINTAIN ALL CLEARANCES AND
 HOLD ALL DIMENSIONS
- 3. REMOVE EXISTING BRIDGEHOUSE, SEE STRUCT. DWGS.
- 4. REMOVE EXISTING CONCRETE AND MISCELLANEOUS RAILINGS, SEE STRUCT. DWGS.
- 5. SEE SHEET A-6.1 FOR MORE INFORMATION ON PRECAST CONCRETE WALL PANELS.
- 6. ALL ELEVATIONS ARE CITY OF CHICAGO DATUM (CCD)
- 7. PROVIDE A PAINTED GYP. BD.
 CEILING AT ELEV +42'-3" CCD.,
 SUSPENDED FROM STRUCTURE
 ABOVE

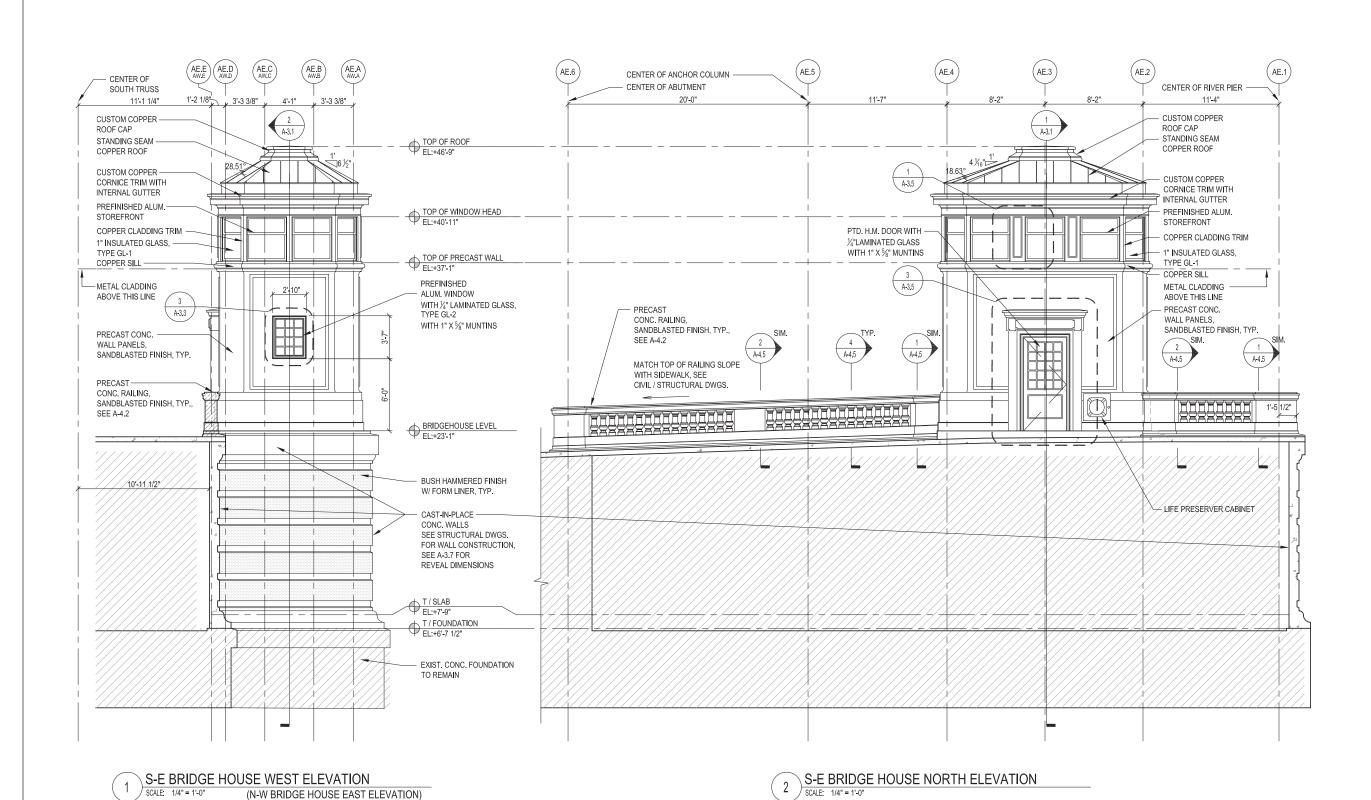


I WSP USA Inc.	USER
30 N. LASALLE STREET SUITE 4200	
CHICAGO, IL 60602 TEL: (312) 782-8150	PLOT
FAX: (312) 782-1684	PLOT

SER NAME = YJL	DESIGNED — YJL	REVISED —	
	CHECKED — ECM	REVISED —	
LOT SCALE = 1:2	DRAWN — YJL	REVISED —	
LOT DATE = $09/23/2020$	CHECKED — ECM	REVISED —	

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.		
1388	11-E1525-00-BR	COOK	A-1.3		
CDOT	PROJECT NO. E-1-525		160 of 210		





L barre, articles

WSP USA Inc. 30 N. IASALLE STREET SUITE 4200 CHICAGO, II. 60602 TEL: (312) 782-8150 FAX: (312) 782-1684

CITY OF CHICAGO

DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

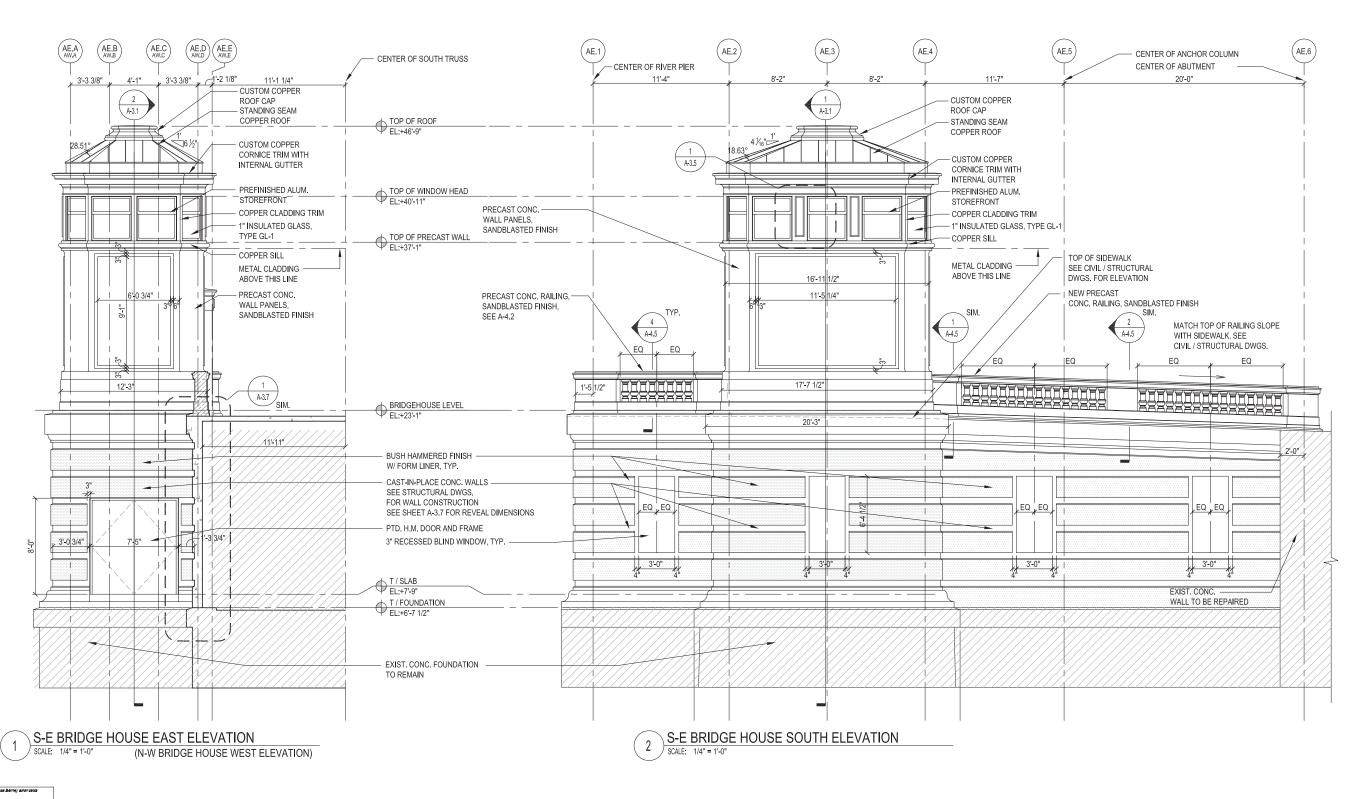
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

SOUTHEAST BRIDGE HOUSE ELEVATION
(STRUCTURE NO. 016-6057)

F.A.U. SECTION COUNTY SHEET NO.

1388 11-E1525-00-BR COOK A-2.1

CDOT PROJECT NO. E-1-525 162 of 210





 USER NAME
 =
 YJL
 DESIGNED
 — YJL
 REVISED
 —

 CHECKED
 —
 ECM
 REVISED
 —

 PLOT SCALE
 =
 1:2
 DRAWN
 — YJL
 REVISED
 —

 PLOT DATE
 =
 09/23/2020
 CHECKED
 — ECM
 REVISED
 —

CITY OF CHICAGO

DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

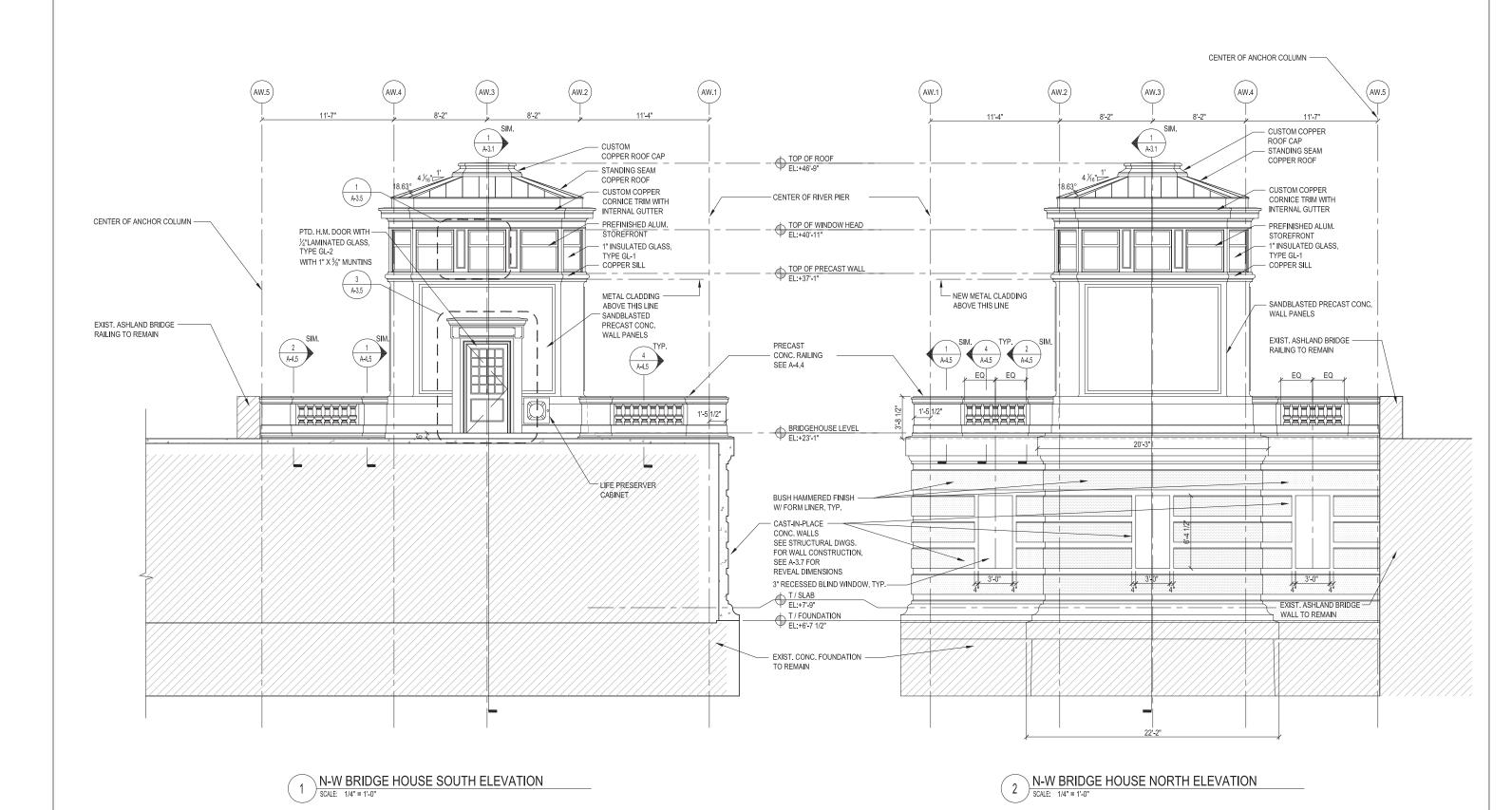
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

SOUTHEAST BRIDGE HOUSE ELEVATION
(STRUCTURE NO. 016-6057)

F.A.U. SECTION COUNTY SHEET NO.

1388 11-E1525-00-BR COOK A-2.2

CDOT PROJECT NO. E-1-525 163 of 210



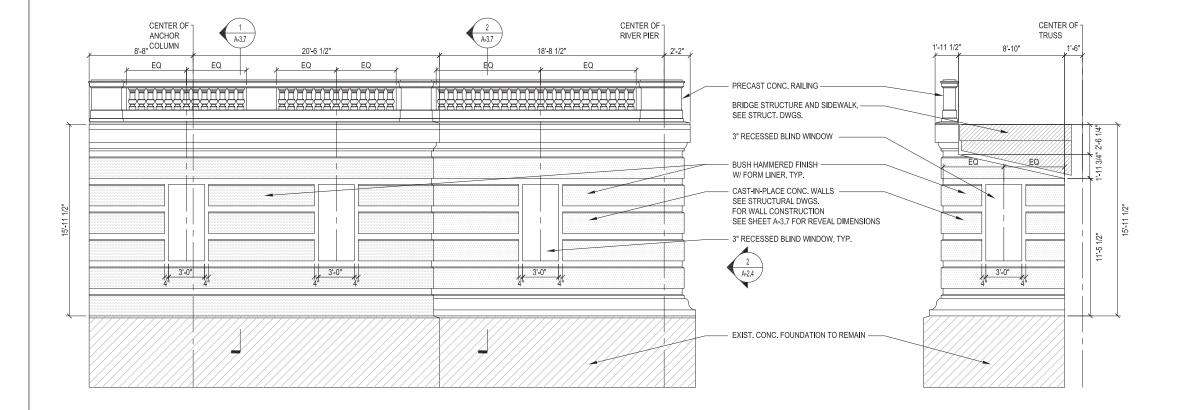
USER NAME = YJL DESIGNED — YJL REVISED CHECKED — ECM DRAWN — YJL PLOT DATE = 09/23/2020CHECKED - ECM

REVISED REVISED REVISED

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER NORTHWEST BRDIGE HOUSE ELEVATION (STRUCTURE NO. 016-6057)

COUNTY SHEET NO. SECTION A-2.3 1388 11-E1525-00-BR COOK CDOT PROJECT NO. E-1-525 164 of 210



1 S-W BRIDGE WALL ELEVATION SCALE: 1/4" = 1'-0"

S-W BRIDGE WALL ELEVATION

SCALE: 1/4" = 1'-0"



USER NAME = YJL	DESIGNED — YJL	REVISED —	
	CHECKED — ECM	REVISED —	
PLOT SCALE = 1:2	DRAWN — YJL	REVISED —	
PLOT DATE = 09/23/2020	CHECKED — ECM	REVISED —	

CITY OF CHICAGO

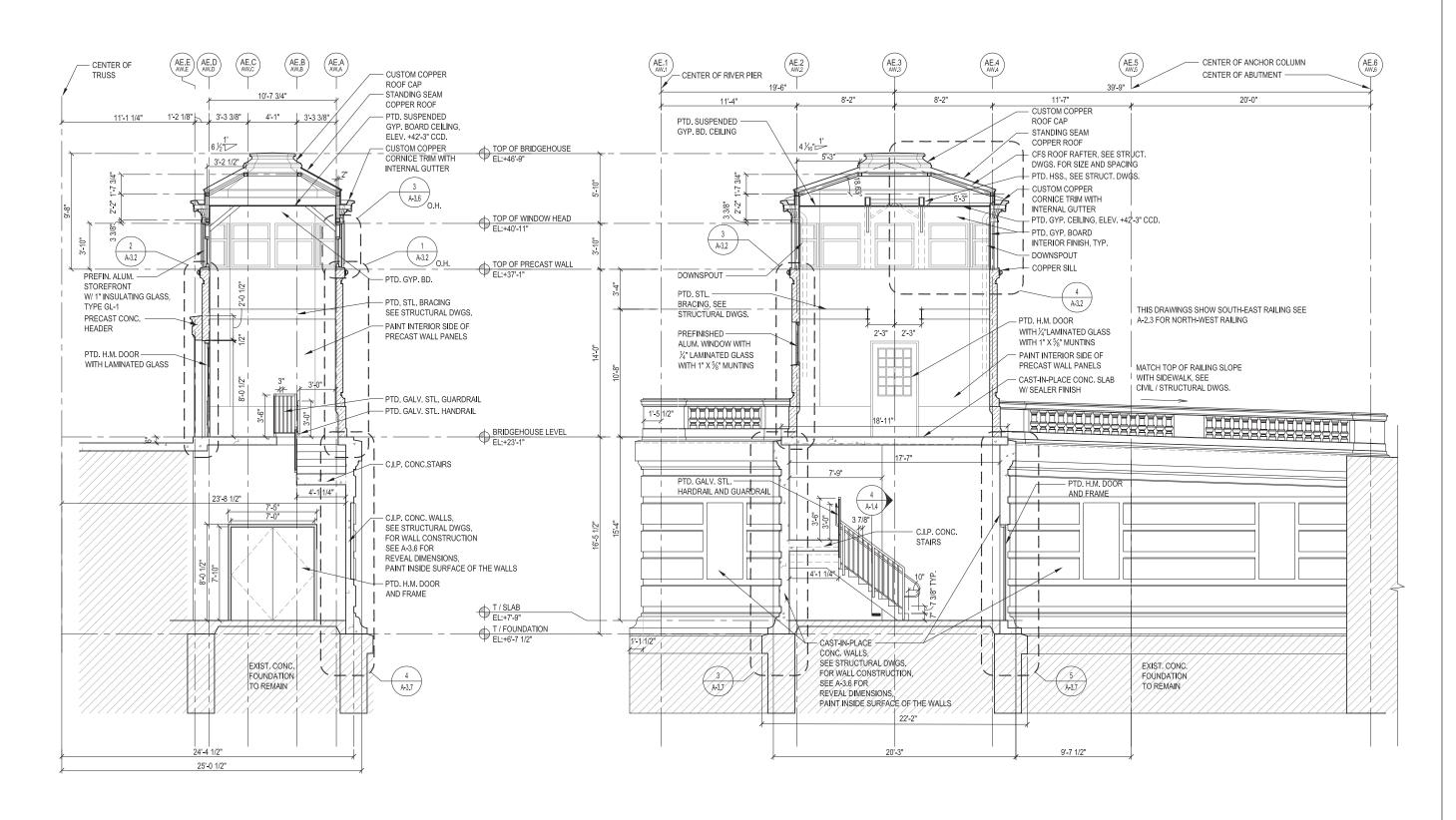
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

SOUTHWEST & NORTHEAST BRIDGE WALL ELEVATION (STRUCTURE NO. 016-6057) F.A.U. SECTION COUNTY SHEET NO.

1388 11-E1525-00-BR COOK A-2.4

CDOT PROJECT NO. E-1-525 165 of 210



S-E BRIDGE HOUSE SECTION

SCALE: 1/4" = 1'-0" (N-W BRIDGE HO

(N-W BRIDGE HOUSE SECTION SIMILAR)

S-E BRIDGE HOUSE SECTION

SCALE: 1/4" = 1'-0" (N-W BRIDGE HO

(N-W BRIDGE HOUSE SECTION SIMILAR)

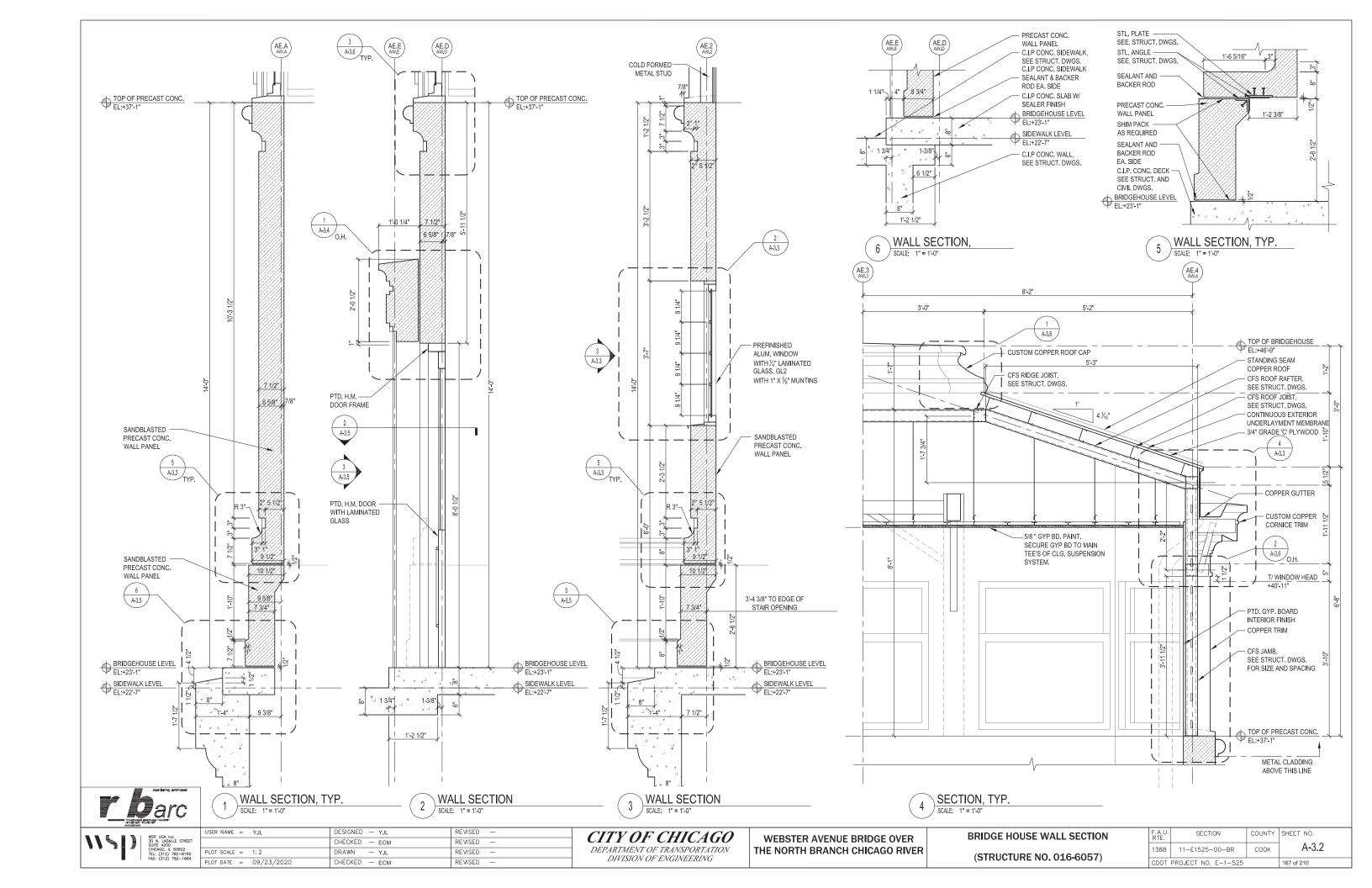


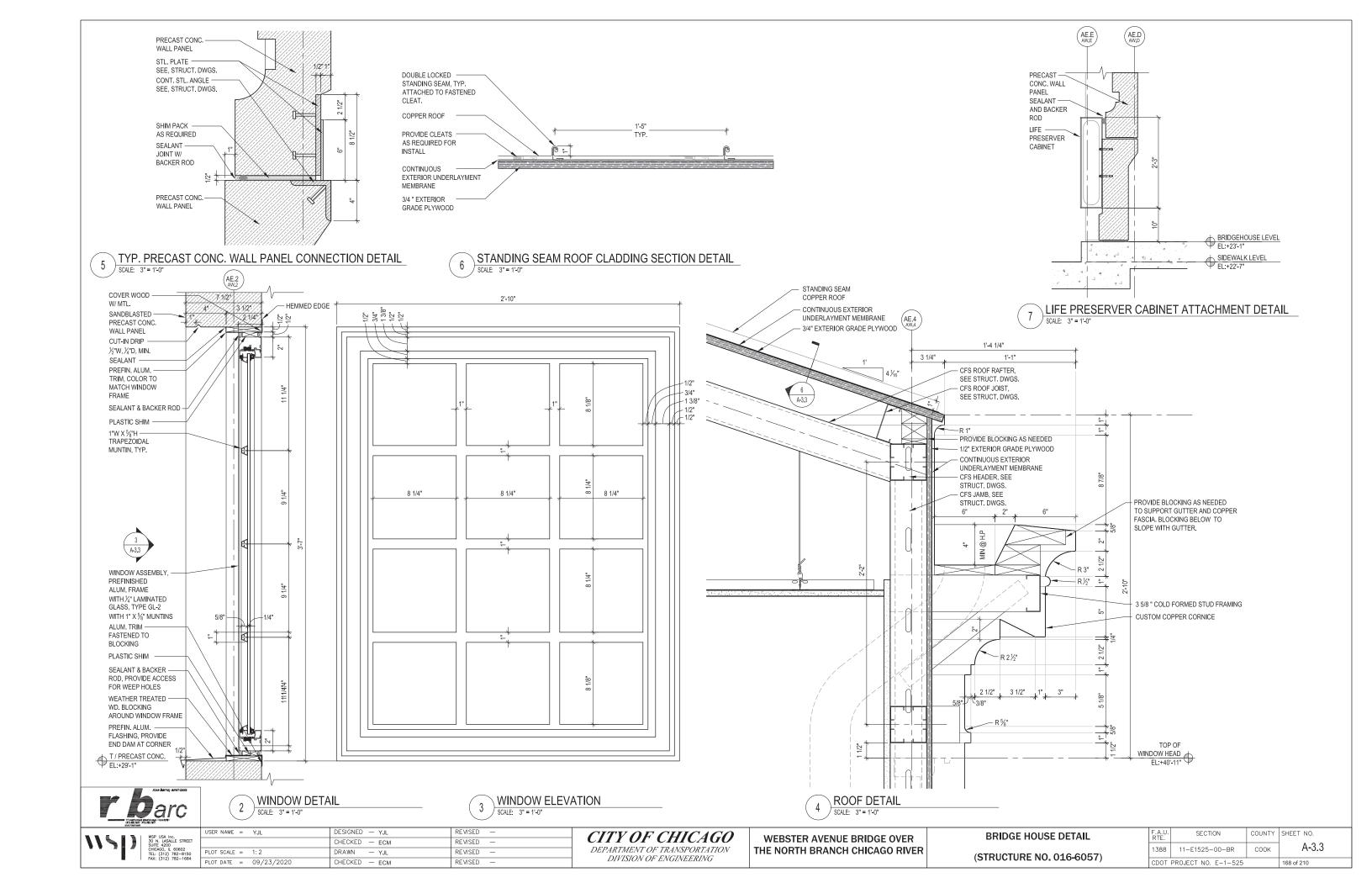
115	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684
-----	---

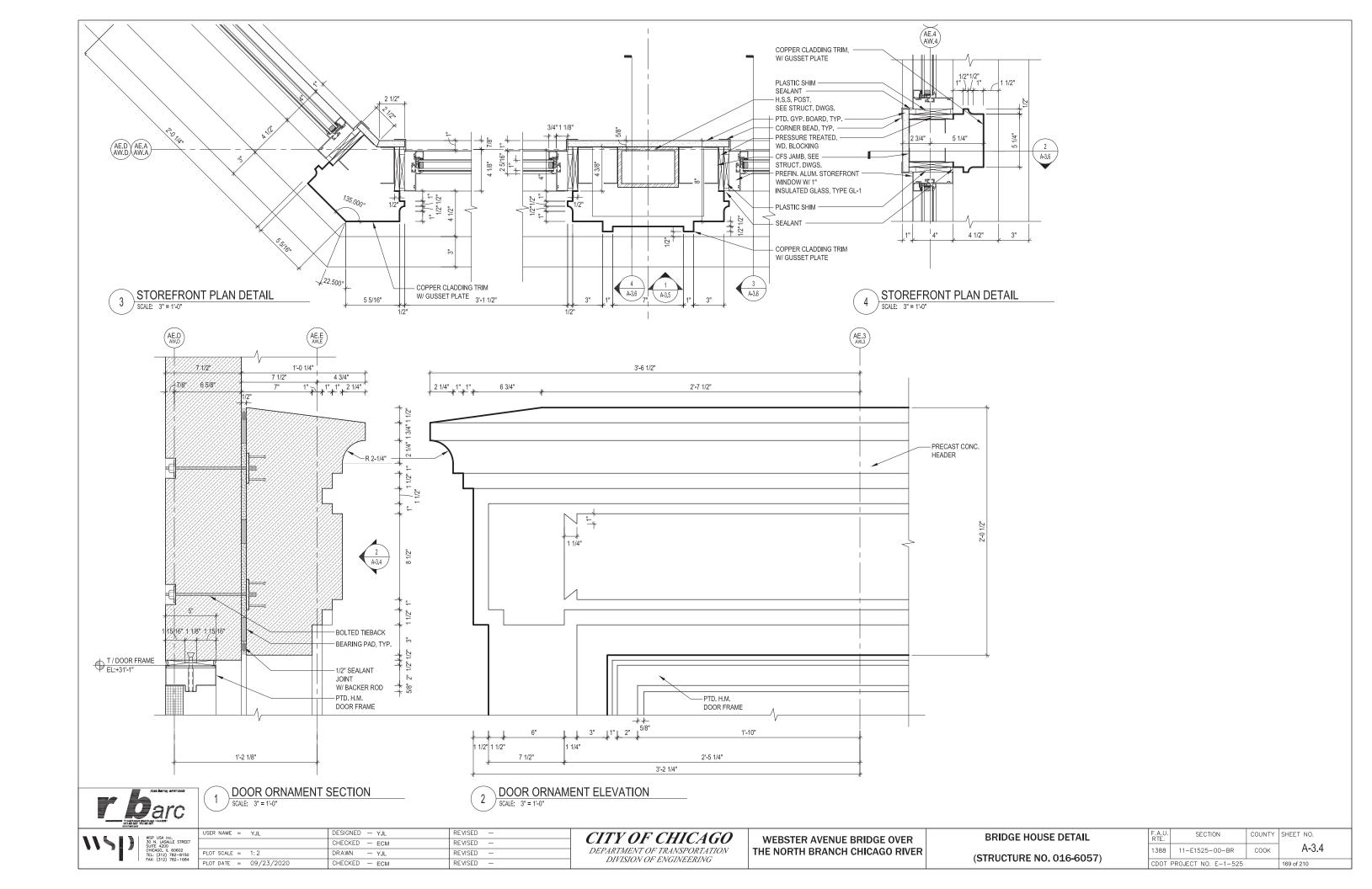
	USER NAME = YJL	DESIGNED — YJL	REVISED —
г		CHECKED — ECM	REVISED —
	PLOT SCALE = 1:2	DRAWN — YJL	REVISED —
	PLOT DATE = 09/23/2020	CHECKED — ECM	REVISED —

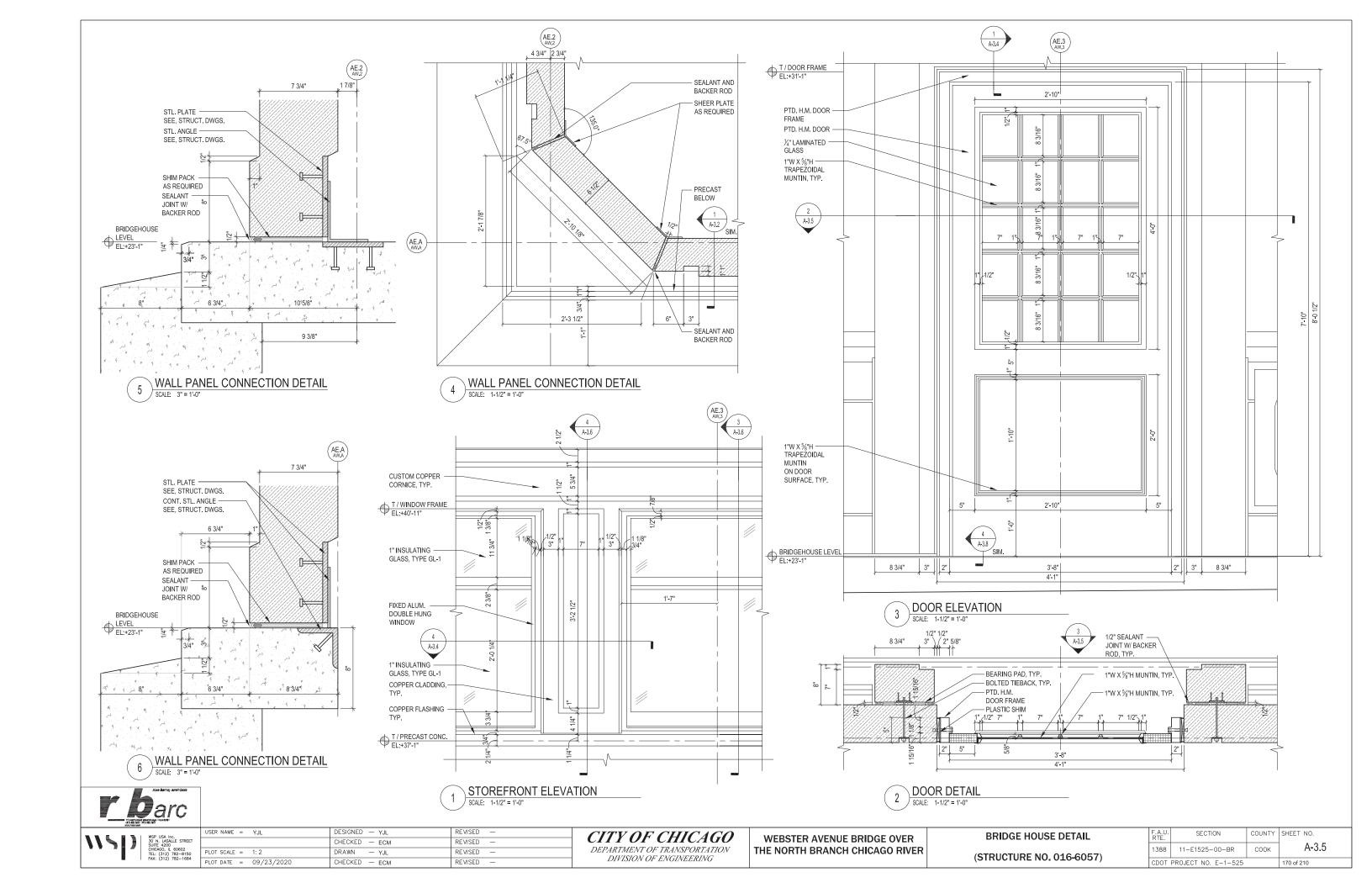
BRIDGE HOUSE SECTION
(STRUCTURE NO. 016-6057)

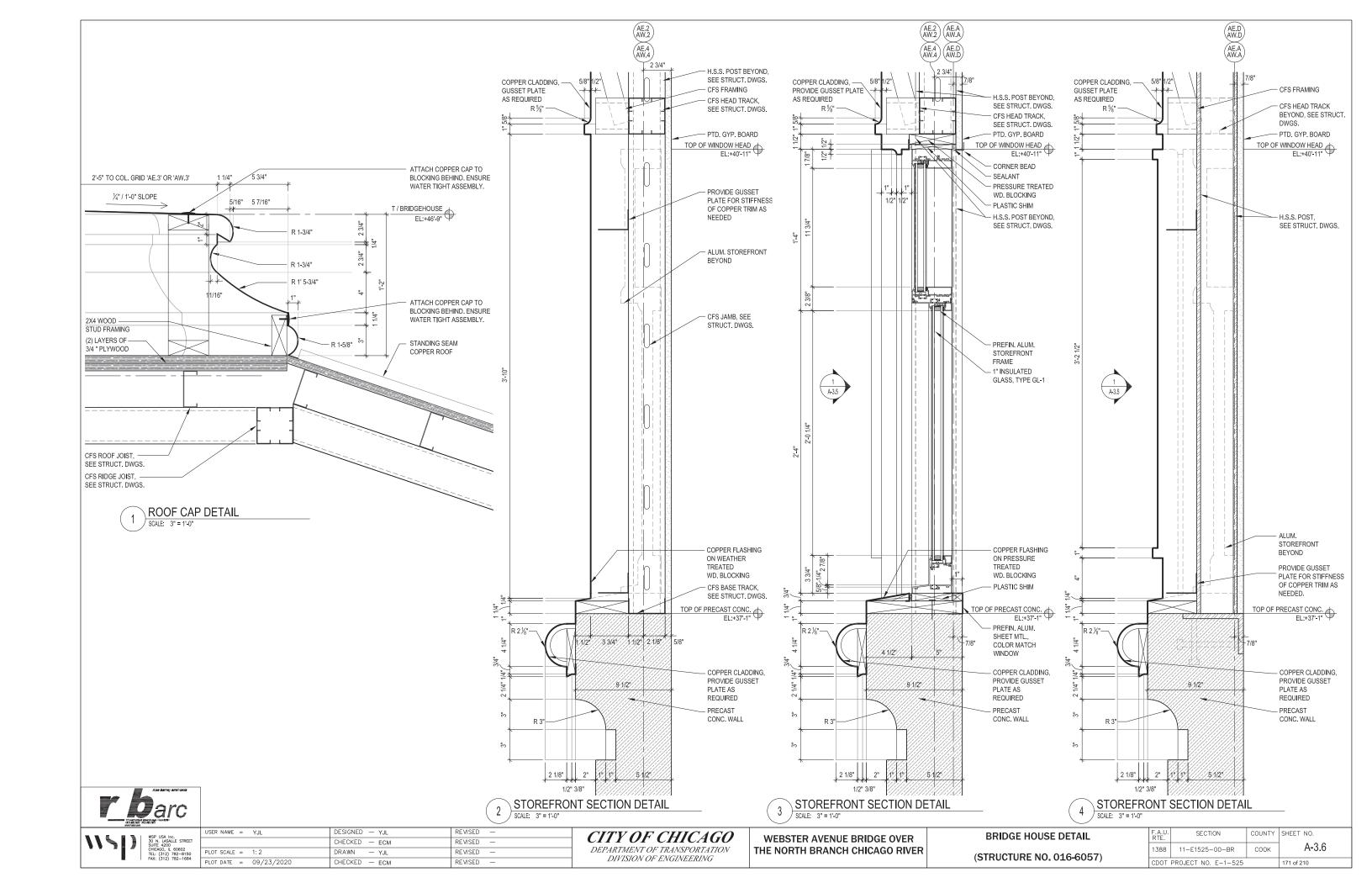
F.A.U. SECTION COUNTY SHEET NO	
1388 11-E1525-00-BR COOK A-3	3.1
CDOT PROJECT NO. E-1-525 166 of 210	

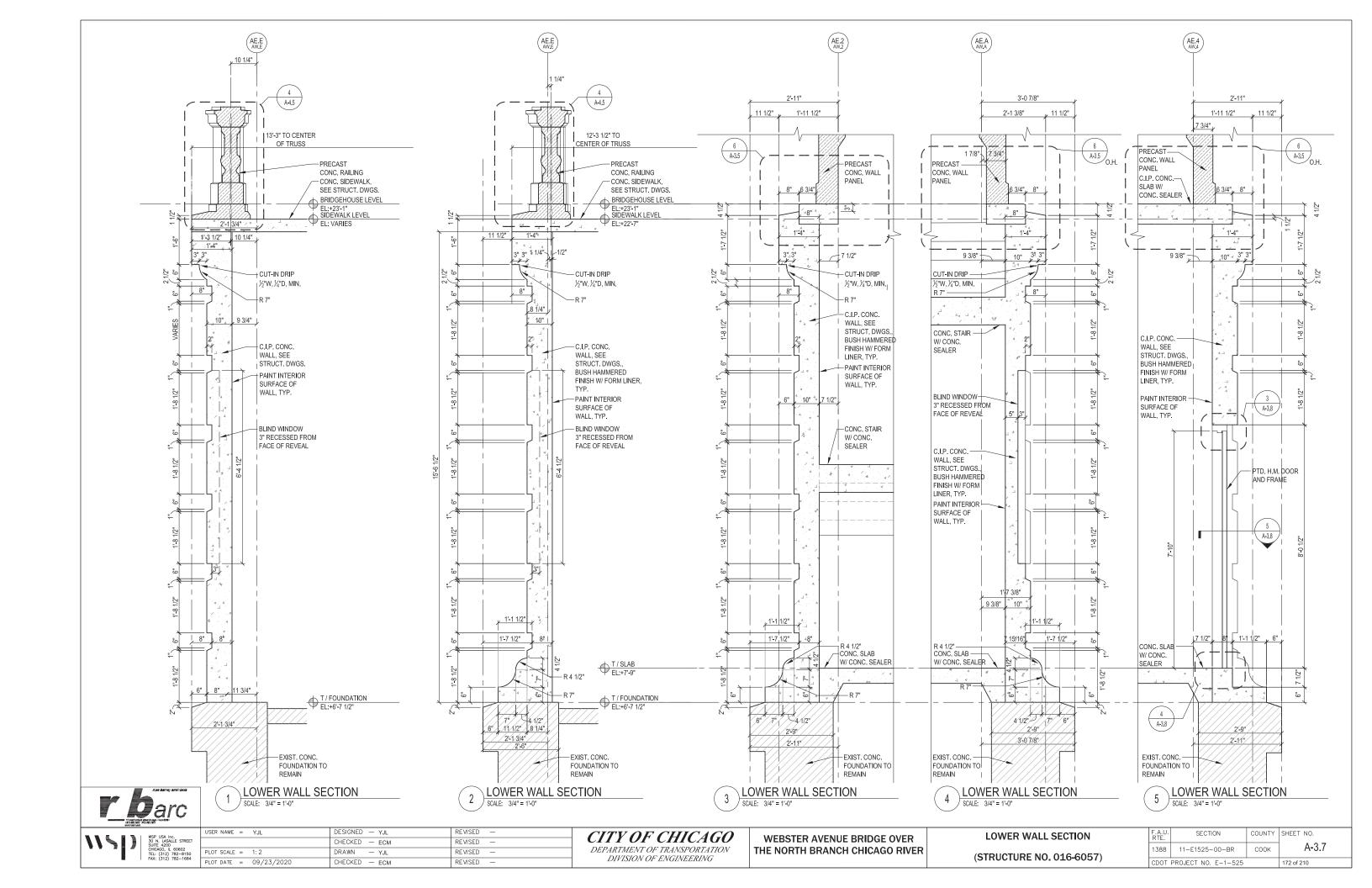








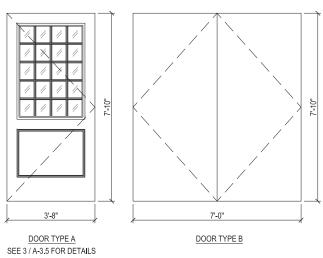


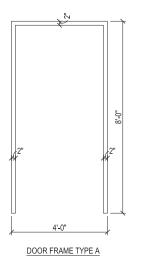


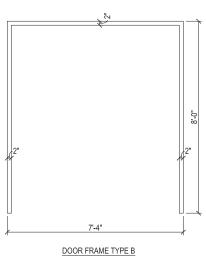
MARK	LOCATION	FIRE RATING	DOOR TYPE	WIDTH	HEIGHT	THICKNESS	DOOR MATERIAL	FRAME TYPE	FRAME MATERIAL	GLAZING	FINISH	HEAD	SILL	JAMB	HARDWARE SET	COMMENT
001	S-E BRIDGE HOUSE / BRIDGE LEVEL	NONE	А	3'-8"	7'-10"	1-3/4"	НМ	Α	НМ	GL-2	PT	1 / A-3.4	4 / A-3.8 SIM.	2 / A-3.5	1	MUNTINS ON GLAZING &
002	S-E BRIDGE HOUSE / DOCK LEVEL	NONE	В	7'-0"	7'-10"	1-3/4"	НМ	В	НМ	NONE	PT	3 / A-3.8	4 / A-3.8	5 / A-3.8	2	
003	N-W BRIDGE HOUSE / BRIDGE LEVEL	NONE	А	3'-8"	7'-10"	1-3/4"	НМ	А	НМ	GL-2	PT	1 / A-3.4	4 / A-3.8 SIM.	2 / A-3.5	1	MUNTINS ON GLAZING &
004	N-W BRIDGE HOUSE / DOCK LEVEL	NONE	В	7'-0"	7'-10"	1-3/4"	НМ	В	НМ	NONE	PT	3 / A-3.8	4 / A-3.8	5 / A-3.8	2	

FINISH MATERIAL LEGEND

1 DOOR SCHEDULE SCALE: NONE



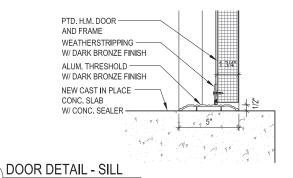


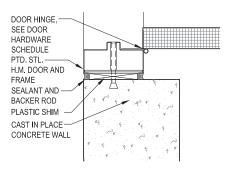


2 DOOR AND DOOR FRAME TYPE
SCALE: NONE CAST IN PLACE — CONCRETE WALL PLASTIC SHIM SEALANT JOINT -W/ BACKER ROD PTD. H.M. DOOR -AND FRAME 1 15/16" 1 1/8" 1 15/16"

DOOR DETAIL - HEAD

SCALE: 3" = 1'-0"





DOOR DETAIL - JAMB 5 SCALE: 3" = 1'-0"

	USER NAME = YJL	DESIGNED — YJL	REVISED —
		CHECKED — ECM	REVISED —
	PLOT SCALE = 1:2	DRAWN — YJL	REVISED —
	PLOT DATE = $09/23/2020$	CHECKED — ECM	REVISED —
_			

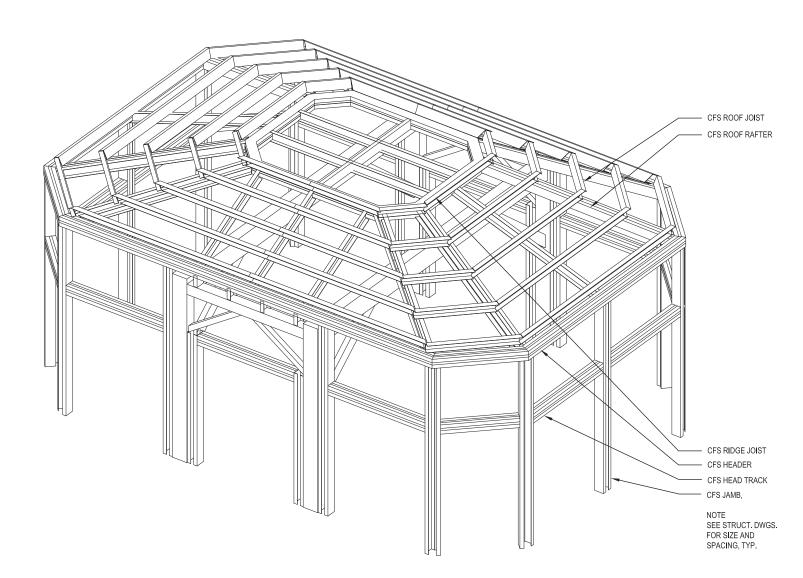
DOOR SCHEDULE AND DETAILS
(STRUCTURE NO. 016-6057)

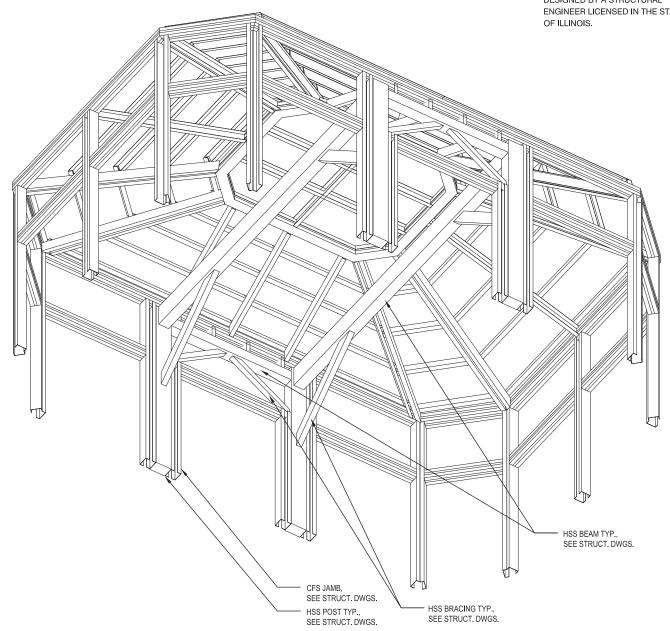
F.A.U. RTE.	SECTION	COUNTY	SHEET NO.		
1388	11-E1525-00-BR	COOK	A-3.8		
CDOT	PROJECT NO. E-1-525		173 of 210		

SHEET NOTES

WEBSTER AVE. BRIDGEHOUSES & RAILINGS

- NOT ALL STUDS AND JOISTS ARE SHOWN FOR CLARITY. THE NECESSARY STUDS, JOISTS, BRACING SHOULD BE PROVIDE AS REQUIRED FOR THE LOADS INDICATED IN THE DOCUMENTS.
- 2. ALL STUD AND JOIST CONNECTIONS, SIZE, SPACING SHALL BE DESIGNED AND DETAILED BY THE FABRICATOR. THE CONNECTIONS SHALL BE DESIGNED BY A STRUCTURAL ENGINEER LICENSED IN THE STATE OF ILLINOIS.





BRIDGE HOUSE ROOF FRAMING - AXONOMETRIC VIEW

SCALE: NONE (REFERENCE ONLY)



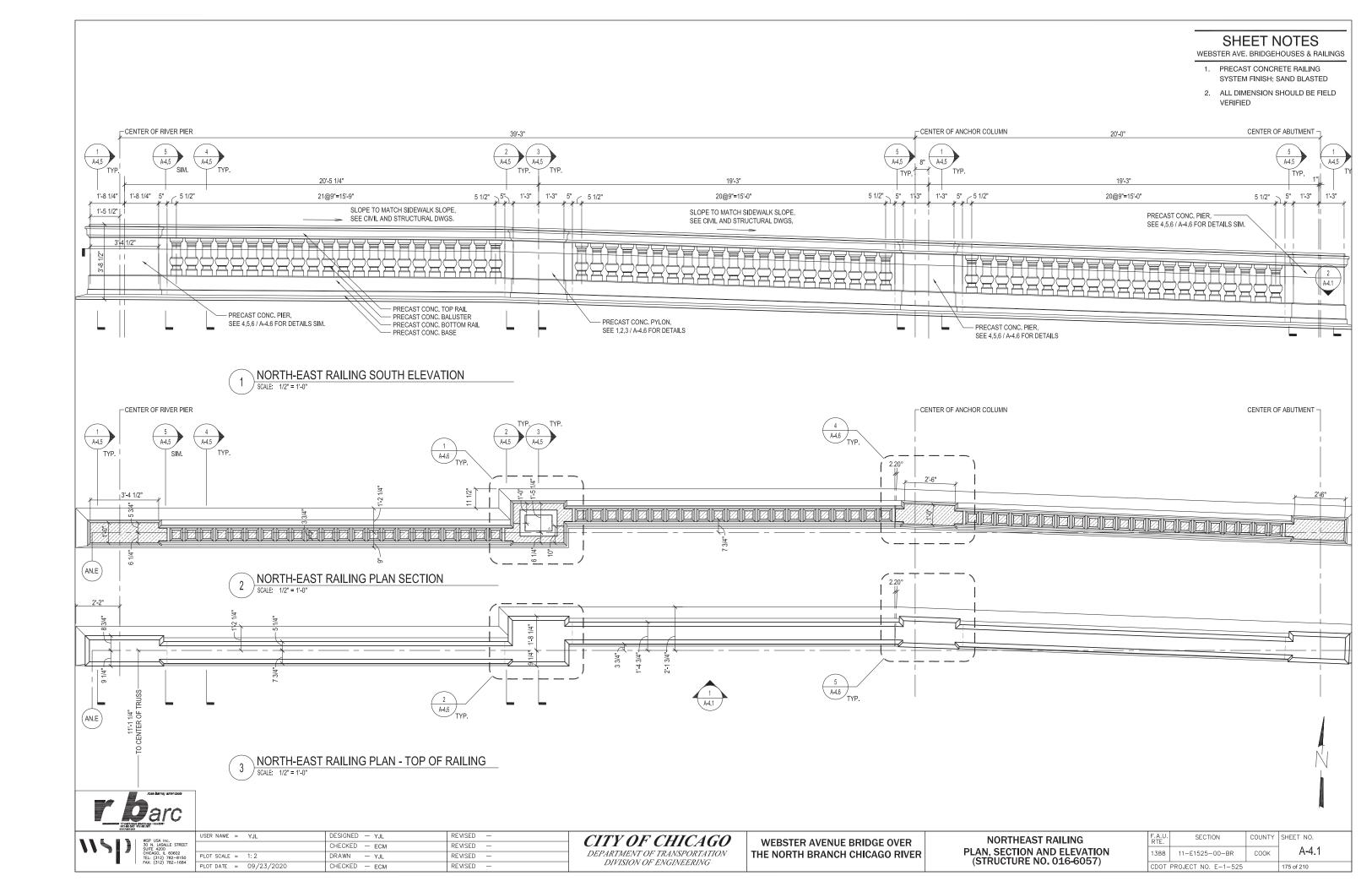
USER NAME = YJL	DESIGNED — YJL	REVISED	_
	CHECKED — ECM	REVISED	_
PLOT SCALE = 1:2	DRAWN — YJL	REVISED	_
PLOT DATE = 09/23/2020	CHECKED — ECM	REVISED	_

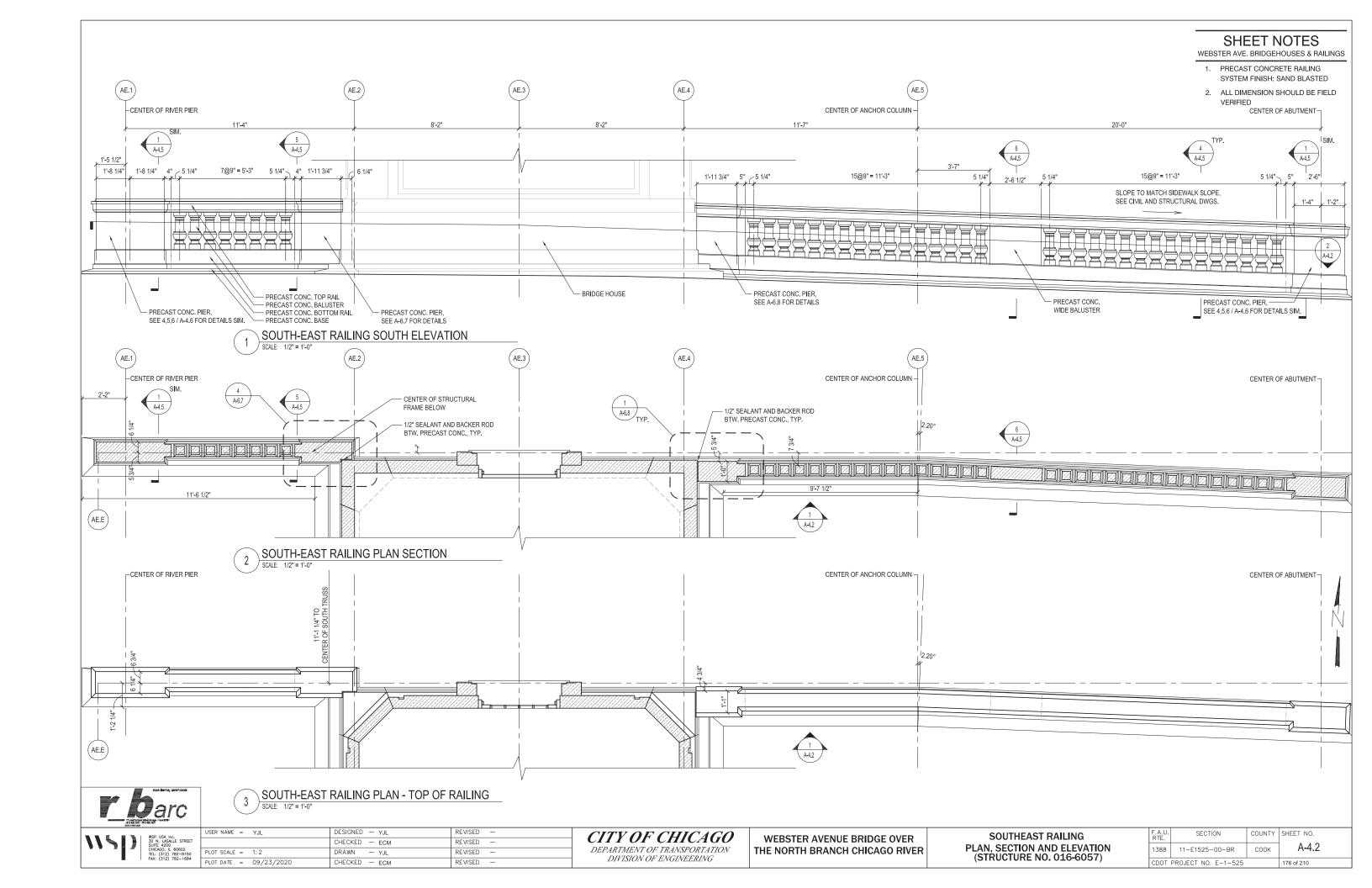
	CITY OF CHICAGO
┪	DEPARTMENT OF TRANSPORTATION
\dashv	DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

BRIDGE HOUSE ROOF FRAMING
AXONOMETRIC VIEW
(STRUCTURE NO. 016-6057)

F.A.U. SECTION RTE.		COUNTY	SHEET NO.
1388	11-E1525-00-BR	COOK	A-3.9
CDOT	PROJECT NO. E-1-525		174 of 210

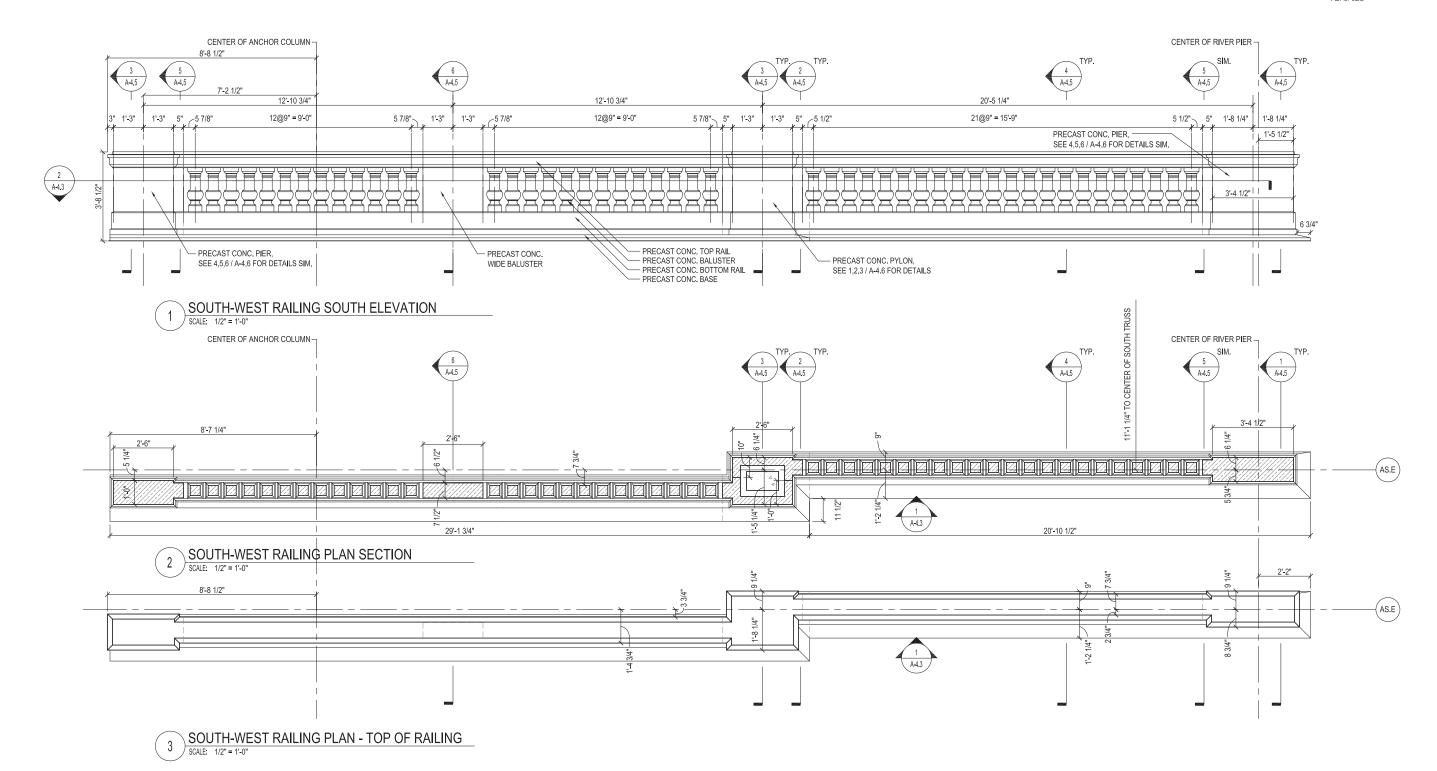




SHEET NOTES

WEBSTER AVE. BRIDGEHOUSES & RAILINGS

- PRECAST CONCRETE RAILING
 SYSTEM FINISH: SAND BLASTED
- 2. ALL DIMENSION SHOULD BE FIELD VERIFIED





WSP USA Inc. 30 N. LASALLE STREET SUITE 4/200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684
--

USER NAME = YJL	DESIGNED — YJL	REVISED —	
	CHECKED — ECM	REVISED —	
PLOT SCALE = 1:2	DRAWN — YJL	REVISED —	
PLOT DATE = $09/23/2020$	CHECKED — ECM	REVISED —	

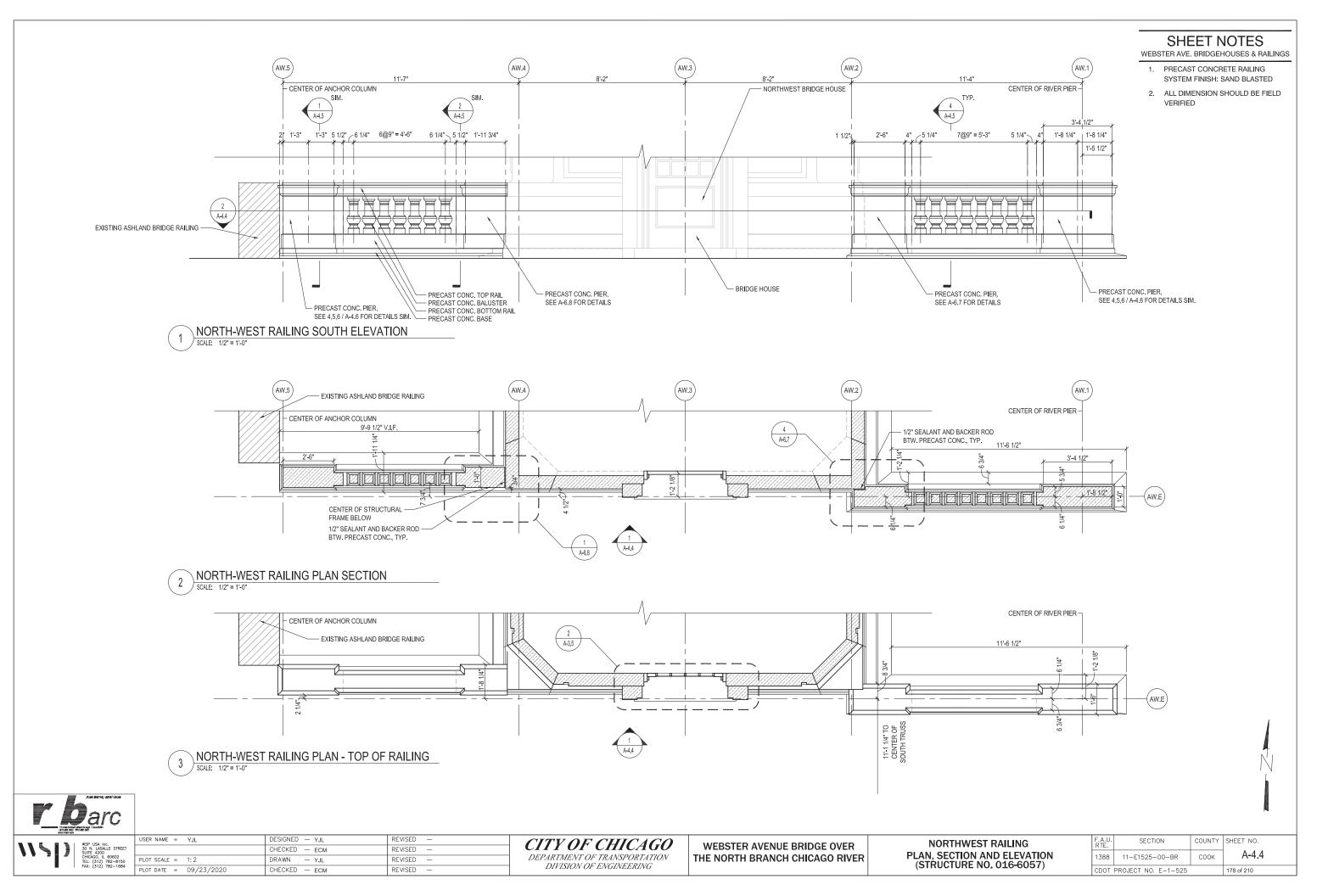
CITY OF CHICAGO

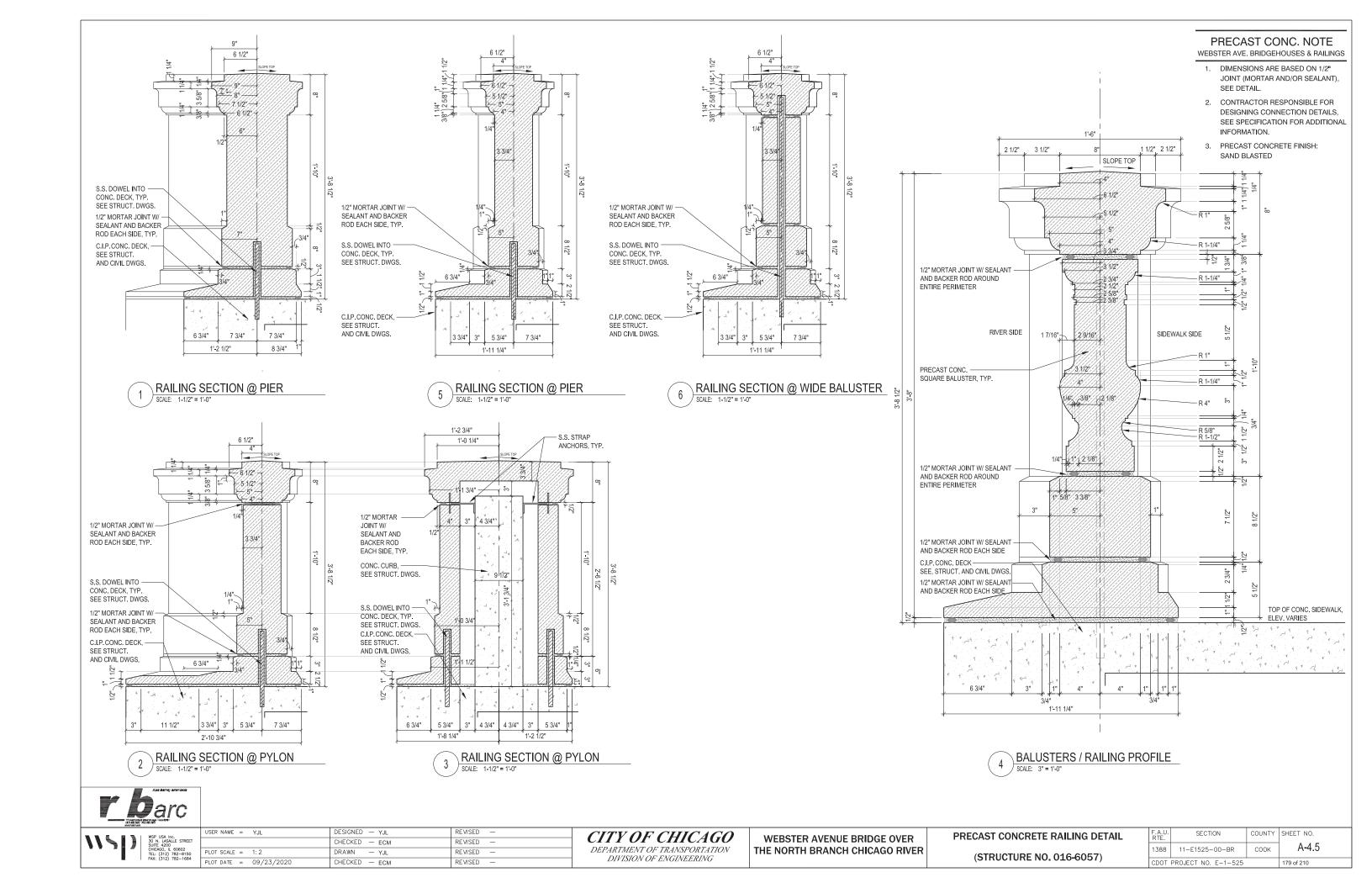
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

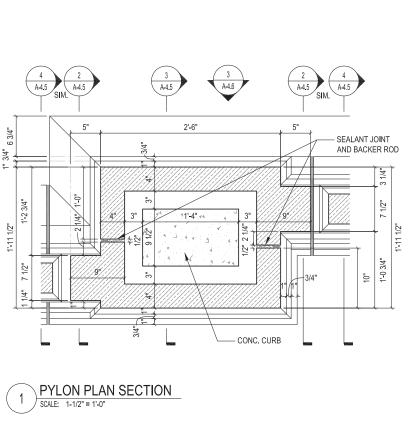
WEBSTER AVENUE BRIDGE OVER
THE NORTH BRANCH CHICAGO RIVER

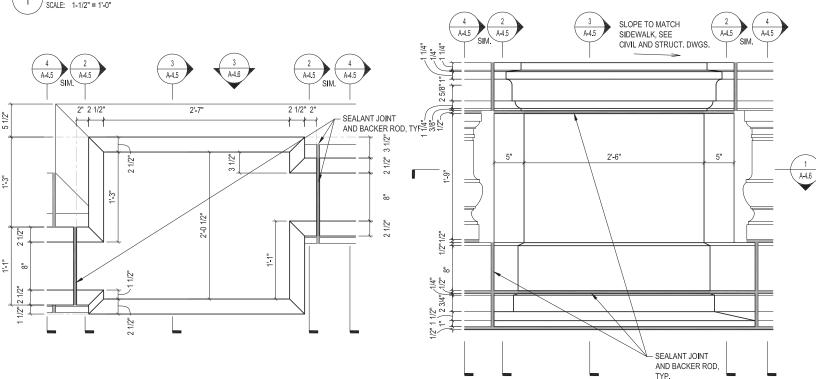
SOUTHWEST RAILING PLAN, SECTION AND ELEVATION (STRUCTURE NO. 016-6057)

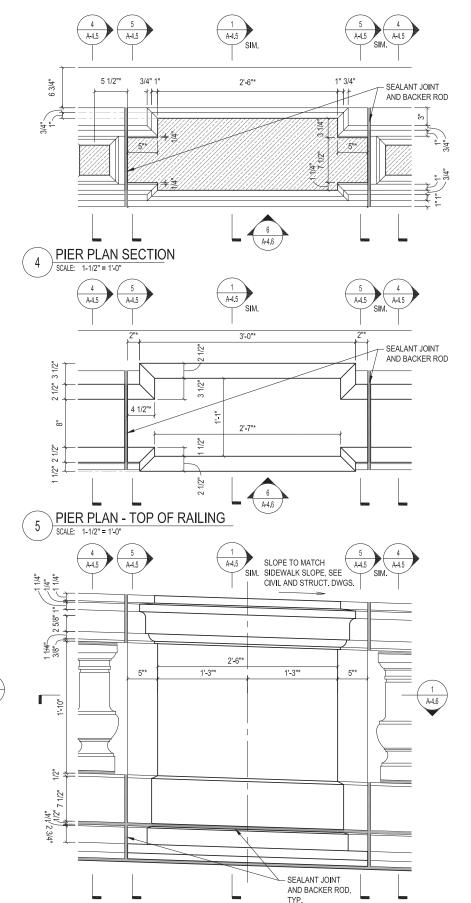
F.A.U. SECTION RTE.		COUNTY	SHEET NO.	
1388	11-E1525-00-BR	COOK	A-4.3	
CDOT	PROJECT NO. E-1-525		177 of 210	











PRECAST CONC. NOTE WEBSTER AVE. BRIDGEHOUSES & RAILINGS

- DIMENSIONS ARE BASED ON 1/2" JOINT (MORTAR AND/OR SEALANT), SEE DETAIL.
- CONTRACTOR RESPONSIBLE FOR DESIGNING CONNECTION DETAILS, SEE SPECIFICATION FOR ADDITIONAL INFORMATION.
- 3. PRECAST CONCRETE FINISH: SAND BLASTED

SHEET NOTES

WEBSTER AVE. BRIDGEHOUSES & RAILINGS

FOR DIMENSIONS FOLLOWED BY " * ", REFER TO ELEVATIONS ON SHEET A-4.1, A-4.2, A-4.3, A-4.4



PYLON PLAN - TOP OF RAILING

SCALE: 1-1/2" = 1'-0"

DESIGNED — YJL

DRAWN — YJL

CHECKED - ECM

CHECKED — ECM

USER NAME = YJL

PLOT DATE = 09/23/2020

PYLON ELEVATION

SCALE: 1-1/2" = 1'-0"

REVISED -

REVISED

REVISED

REVISED

CITY OF CHICAGO

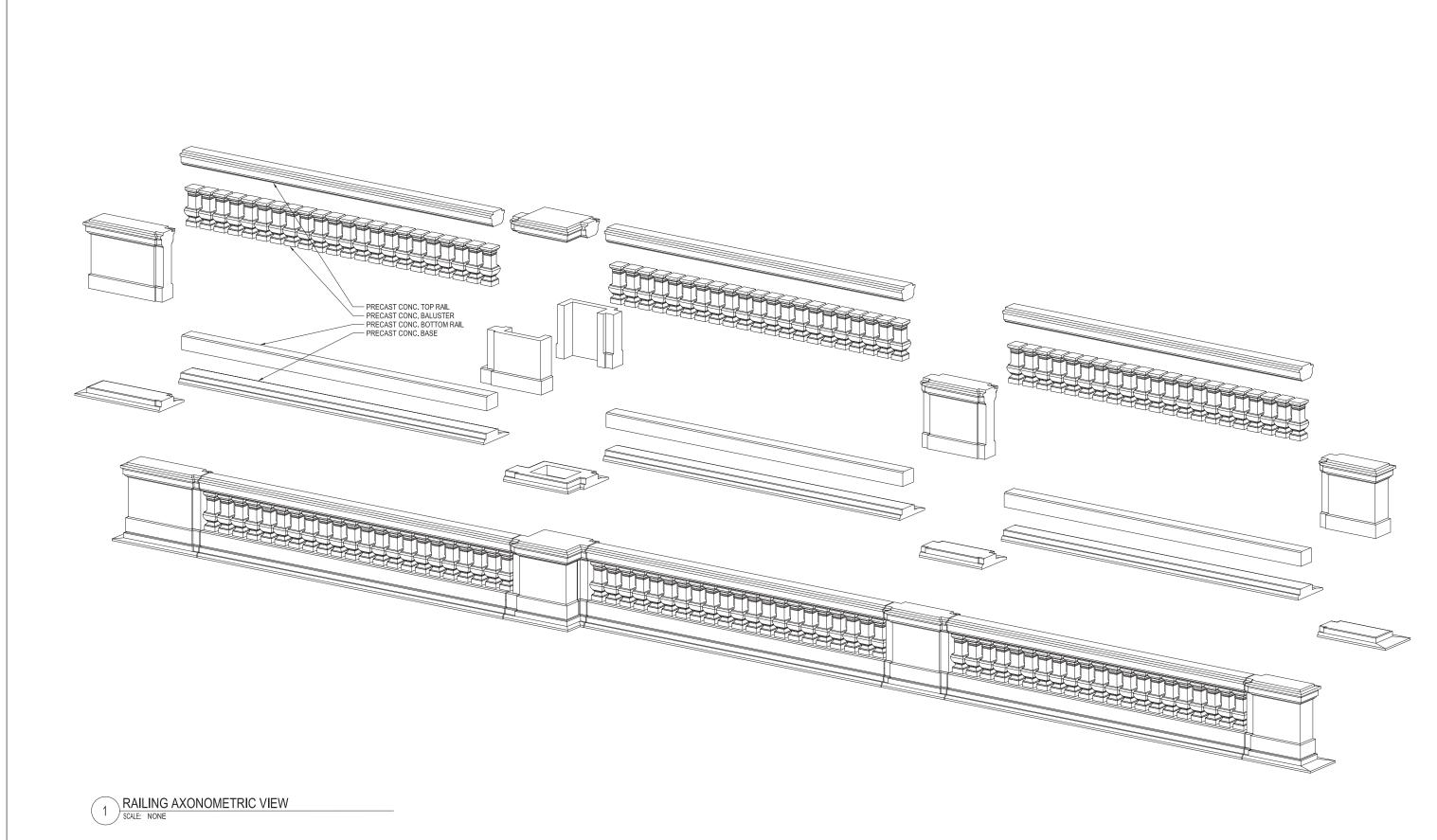
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

PIER ELEVATION
SCALE: 1-1/2" = 1'-0"

PRECAST CONCRETE RAILING DETAIL
(STRUCTURE NO. 016-6057)

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	COOK	A-4.6
CDOT	PROJECT NO. E-1-525		180 of 210



r baro

WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684
 USER NAME
 =
 YJL
 DESIGNED
 — YJL
 REVISED
 —

 CHECKED
 —
 ECM
 REVISED
 —

 PLOT SCALE
 =
 1; 2
 DRAWN
 — YJL
 REVISED
 —

 PLOT DATE
 =
 09/23/2020
 CHECKED
 — ECM
 REVISED
 —

CITY OF CHICAGO

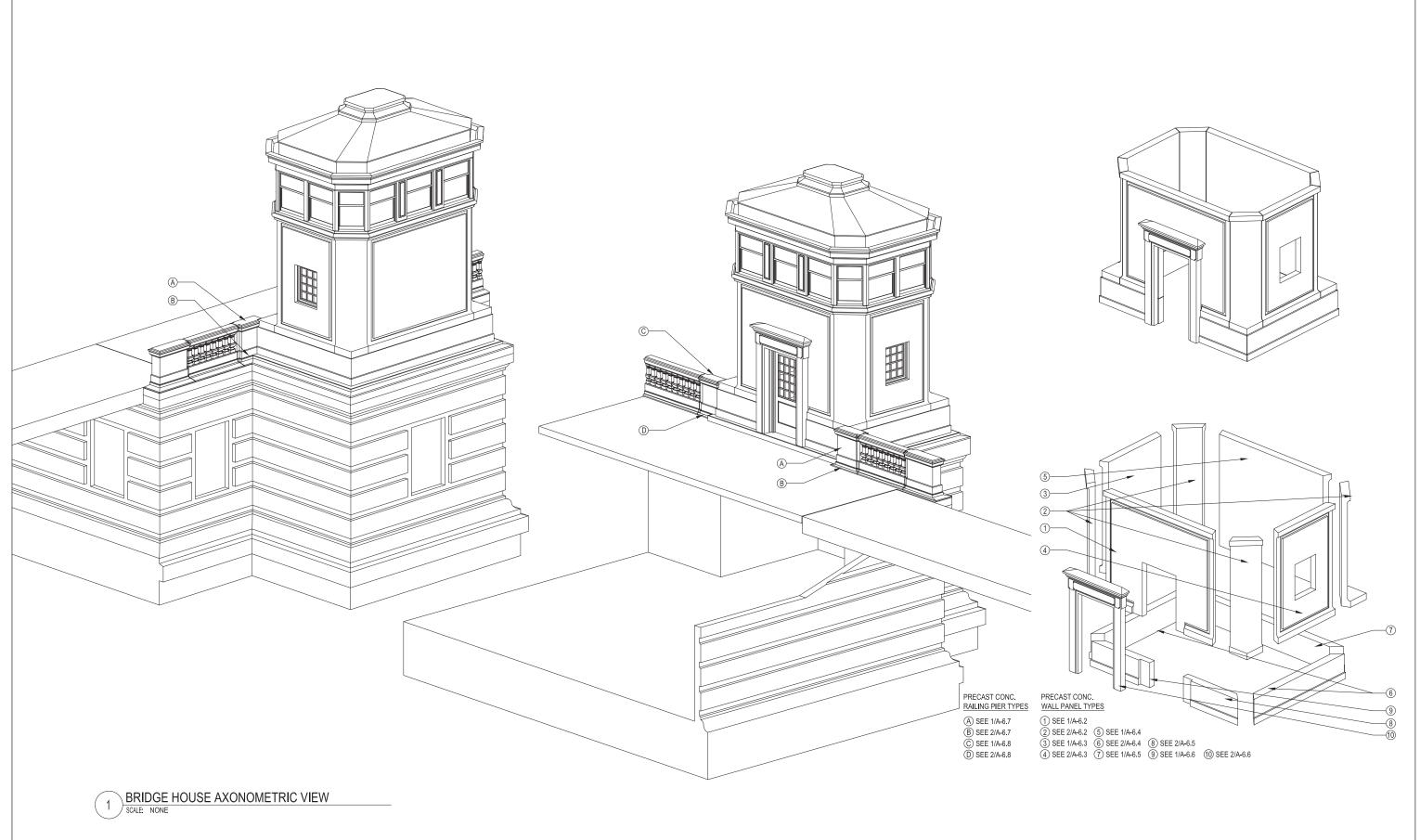
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

PRECAST CONCRETE RAILING AXONOMETRIC VIEW (STRUCTURE NO. 016-6057) F.A.U. SECTION COUNTY SHEET NO.

1388 11-E1525-00-BR COOK A-4.7

CDOT PROJECT NO. E-1-525 181 of 210





WSP USA Inc. 30 N. LASALE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684
 USER NAME
 =
 YJL
 DESIGNED
 — YJL
 REVISED
 —

 CHECKED
 —
 ECM
 REVISED
 —

 PLOT SCALE
 =
 1:2
 DRAWN
 — YJL
 REVISED
 —

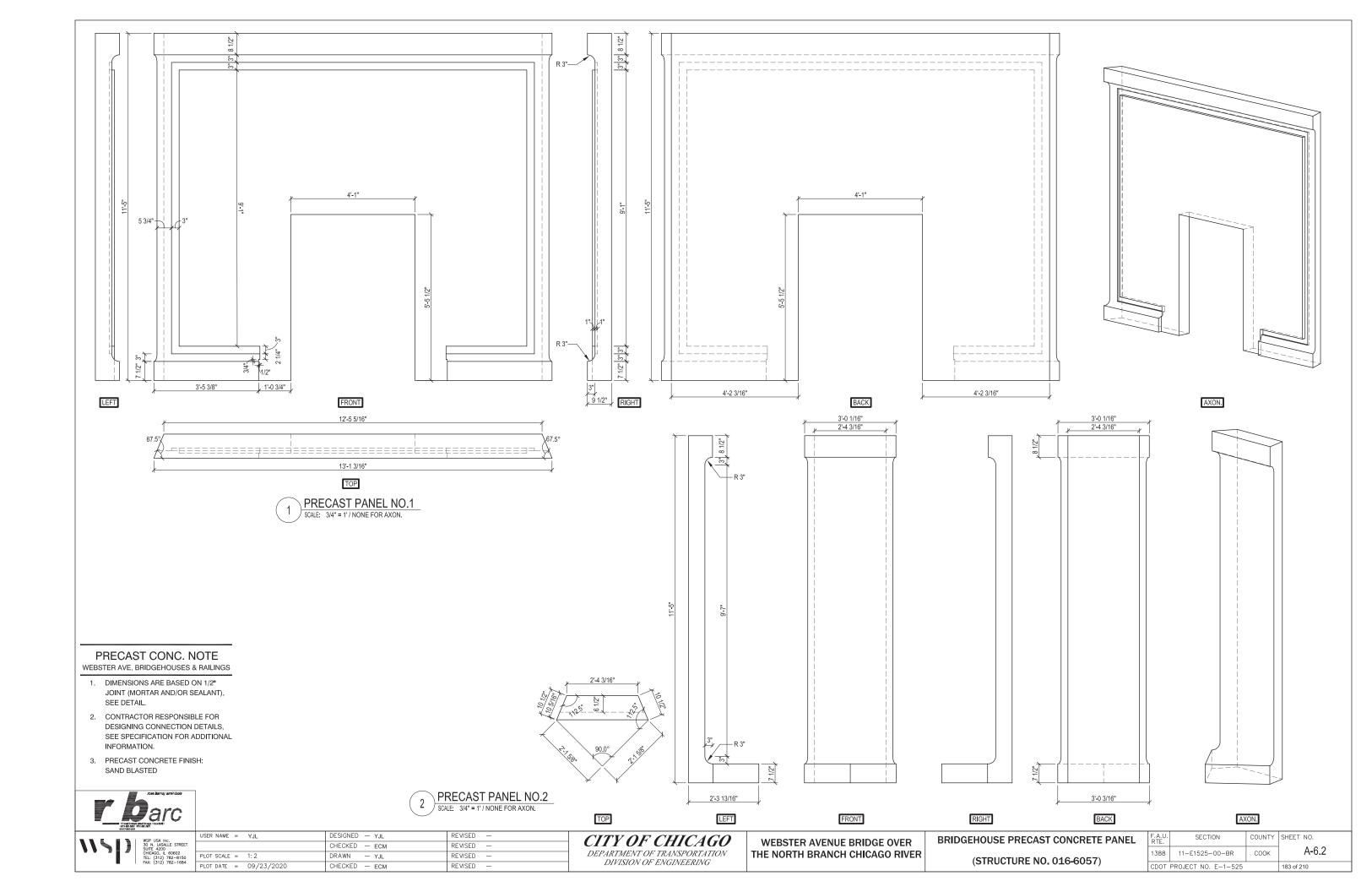
 PLOT DATE
 =
 09/23/2020
 CHECKED
 — ECM
 REVISED
 —

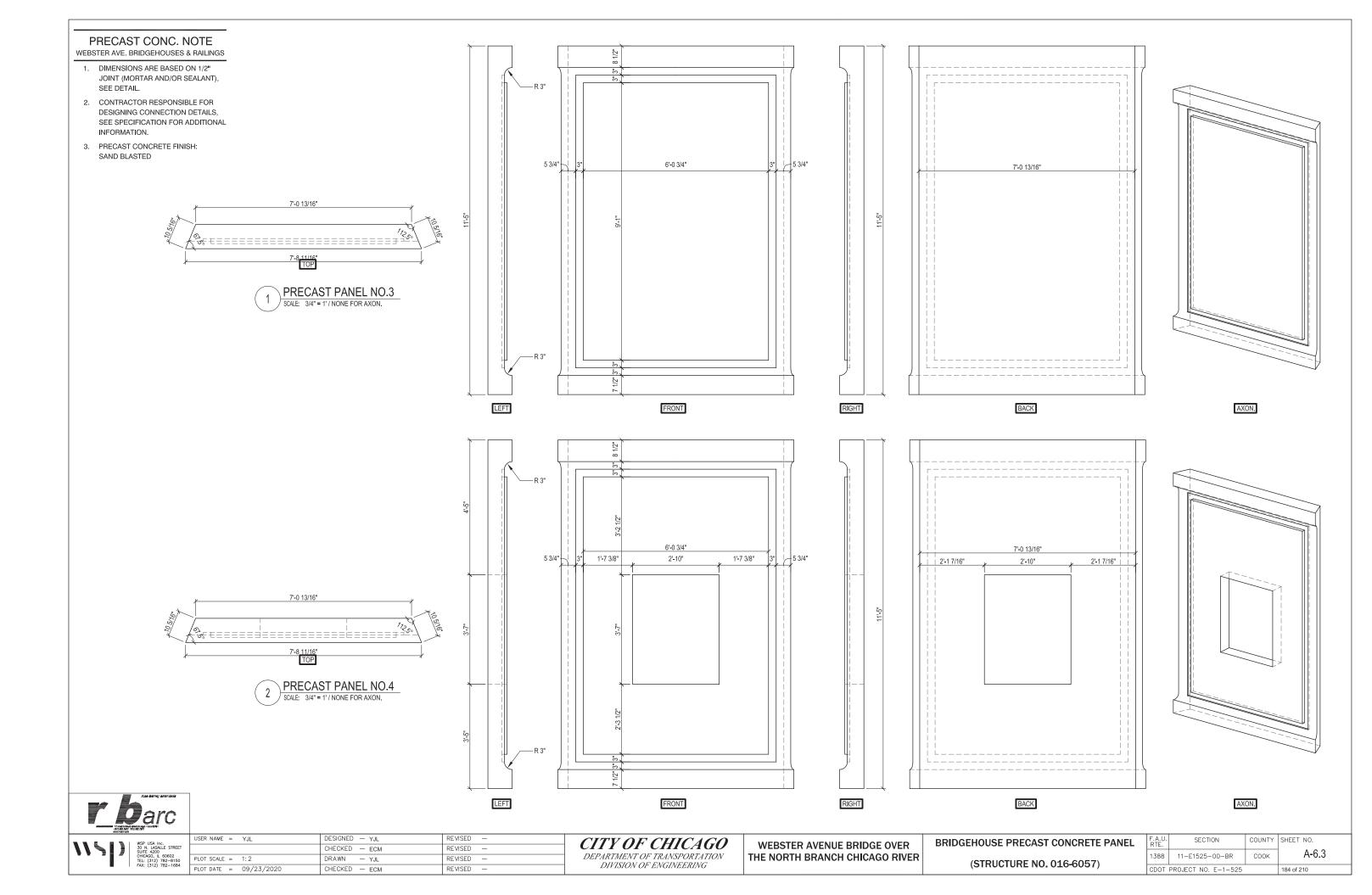
CITY OF CHICAGO

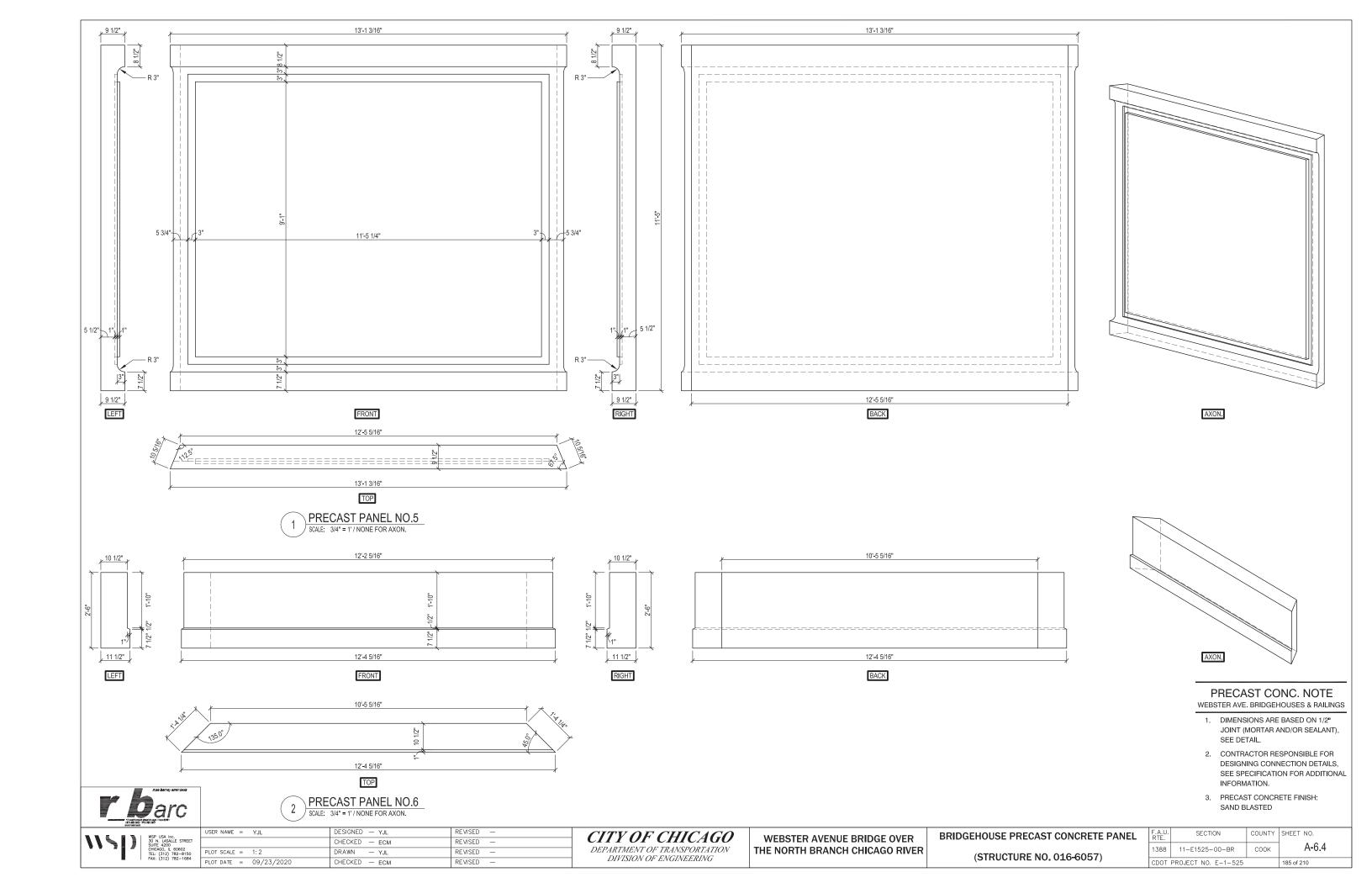
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

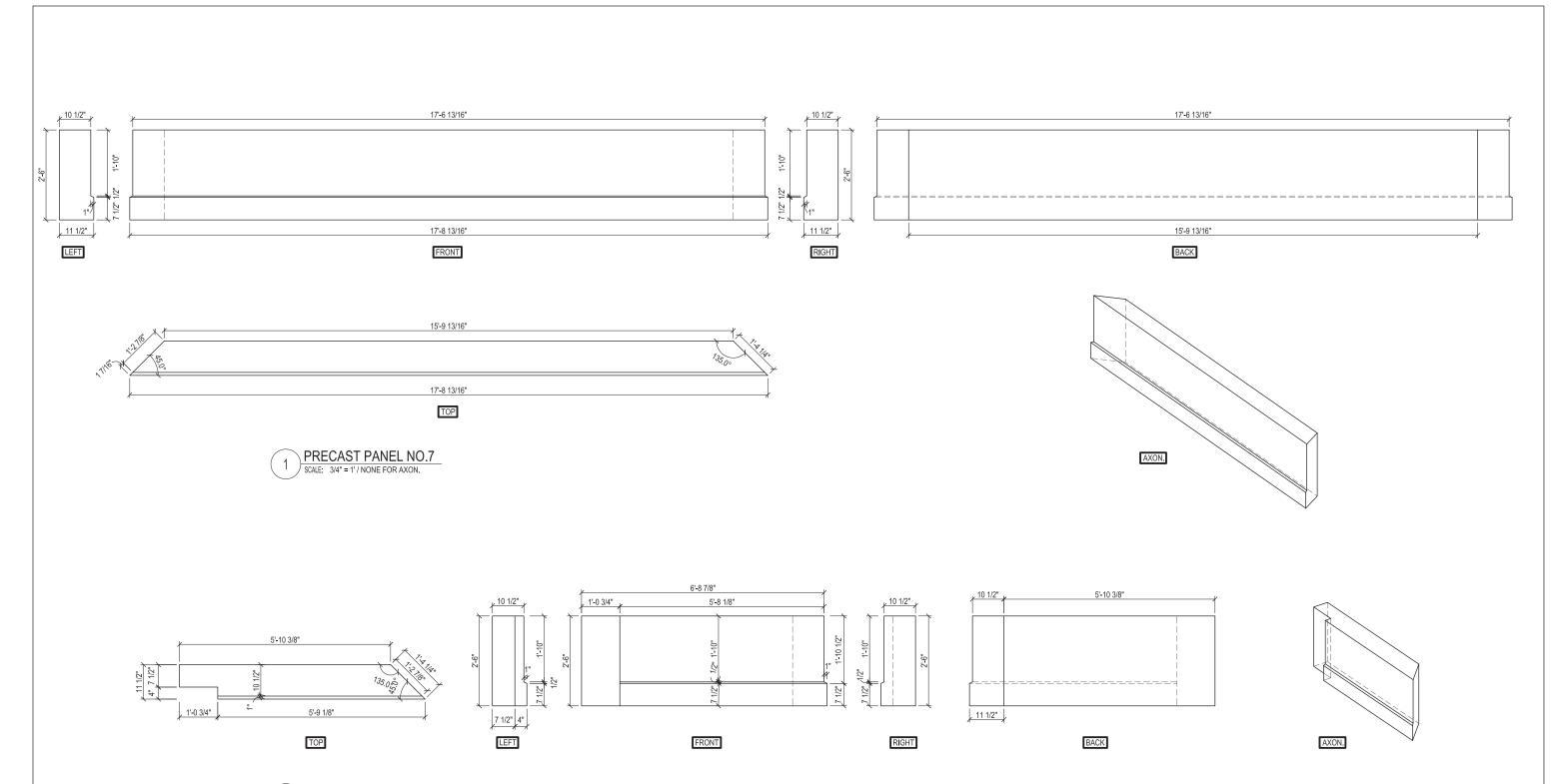
WEBSTER AVENUE BRIDGE OVER
THE NORTH BRANCH CHICAGO RIVER

BRIDGE HOUSE AXONOMETRIC VIEW (STRUCTURE NO. 016-6057)











PRECAST CONC. NOTE WEBSTER AVE. BRIDGEHOUSES & RAILINGS

- DIMENSIONS ARE BASED ON 1/2"
 JOINT (MORTAR AND/OR SEALANT),
 SEE DETAIL.
- CONTRACTOR RESPONSIBLE FOR DESIGNING CONNECTION DETAILS, SEE SPECIFICATION FOR ADDITIONAL INFORMATION.
- 3. PRECAST CONCRETE FINISH: SAND BLASTED



115])	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684
-------	---

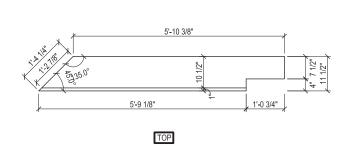
JSER NAME = YJL	DESIGNED — YJL	REVISED —
	CHECKED — ECM	REVISED —
PLOT SCALE = 1:2	DRAWN — YJL	REVISED —
PLOT DATE = $09/23/2020$	CHECKED — ECM	REVISED —

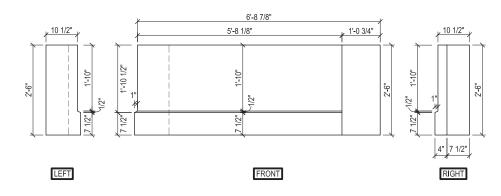


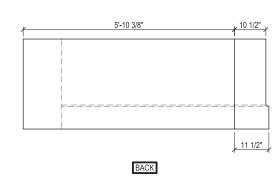


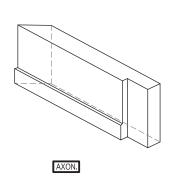
BRIDGEHOUSE PRECAST CONCRETE PANEL (STRUCTURE NO. 016-6057)

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	COOK	A-6.5
CDOT	PROJECT NO. E-1-525		186 of 210



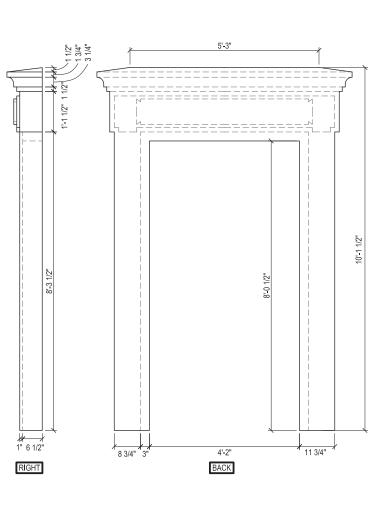


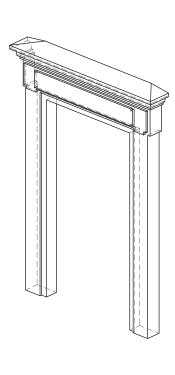




PRECAST PANEL NO.9 SCALE: 3/4" = 1' / NONE FOR AXON.

> 8 3/4" 3" 11 3/4" 4'-2" LEFT FRONT





AXON.

7'-1 1/2" TOP PRECAST PANEL NO.10

SCALE: 3/4" = 1' / NONE FOR AXON.



- 1. DIMENSIONS ARE BASED ON 1/2" JOINT (MORTAR AND/OR SEALANT), SEE DETAIL.
- 2. CONTRACTOR RESPONSIBLE FOR DESIGNING CONNECTION DETAILS, SEE SPECIFICATION FOR ADDITIONAL INFORMATION.
- 3. PRECAST CONCRETE FINISH: SAND BLASTED

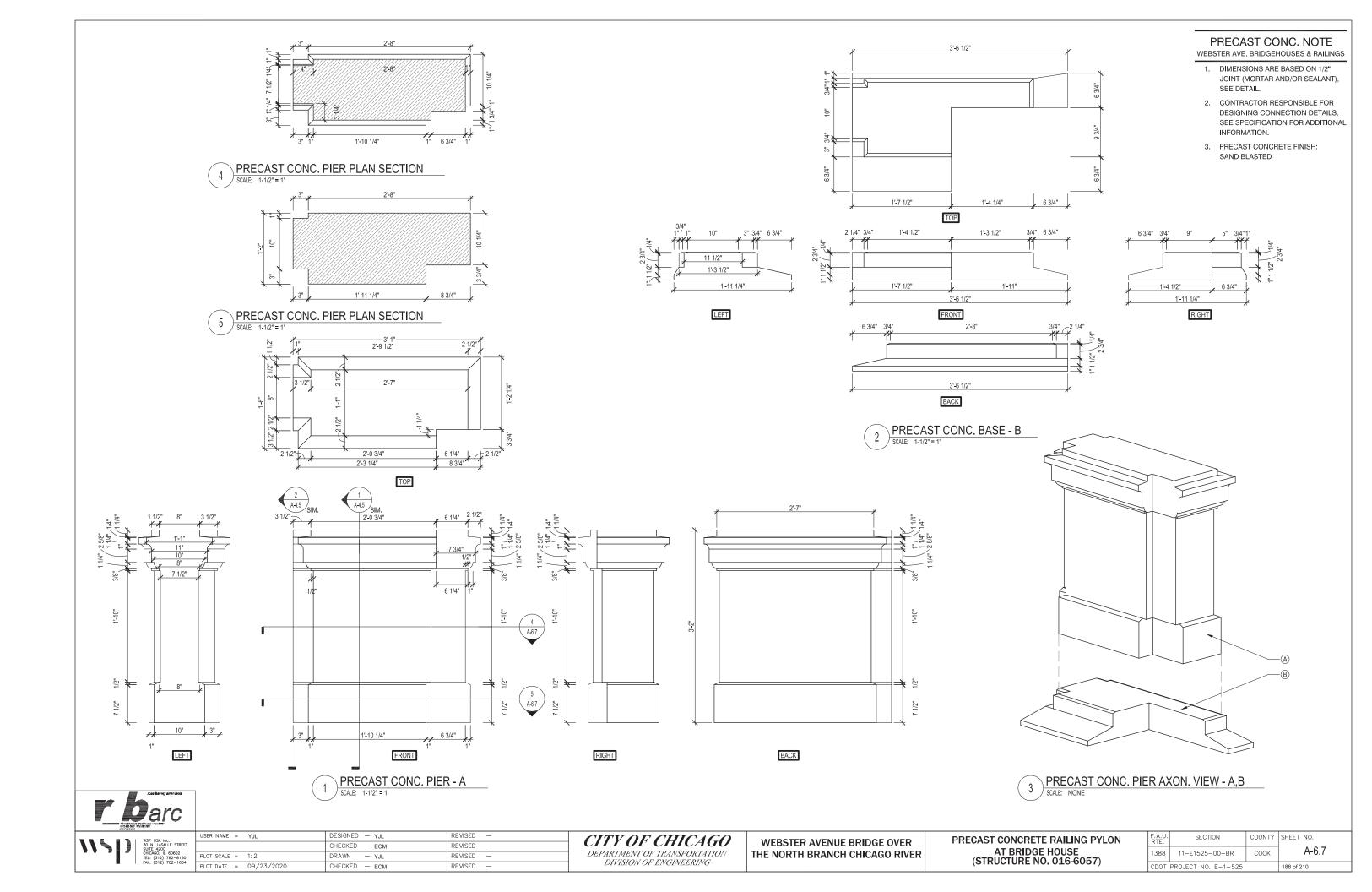


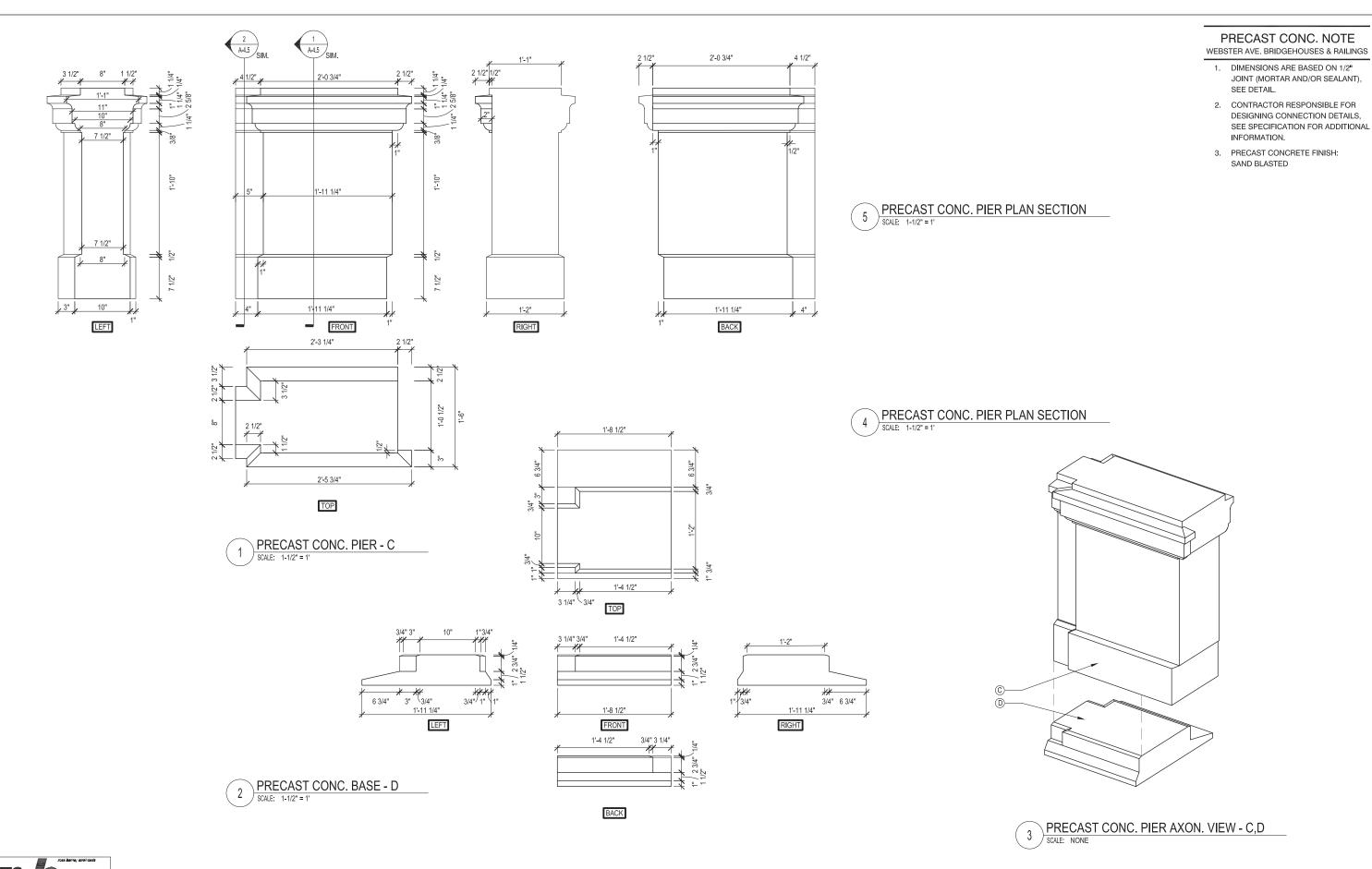
1151)	WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1684
-------	---

	USER NAME	=	YJL
T			
)	PLOT SCALE	=	1: 2
•	PLOT DATE	-	09/23/2020

USER NAME = YJL	DESIGNED — YJL	REVISED —
	CHECKED — ECM	REVISED —
PLOT SCALE = 1:2	DRAWN — YJL	REVISED —
PLOT DATE = $09/23/2020$	CHECKED — ECM	REVISED —

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	COOK	A-6.6
CDOT	PROJECT NO. E-1-525		187 of 210









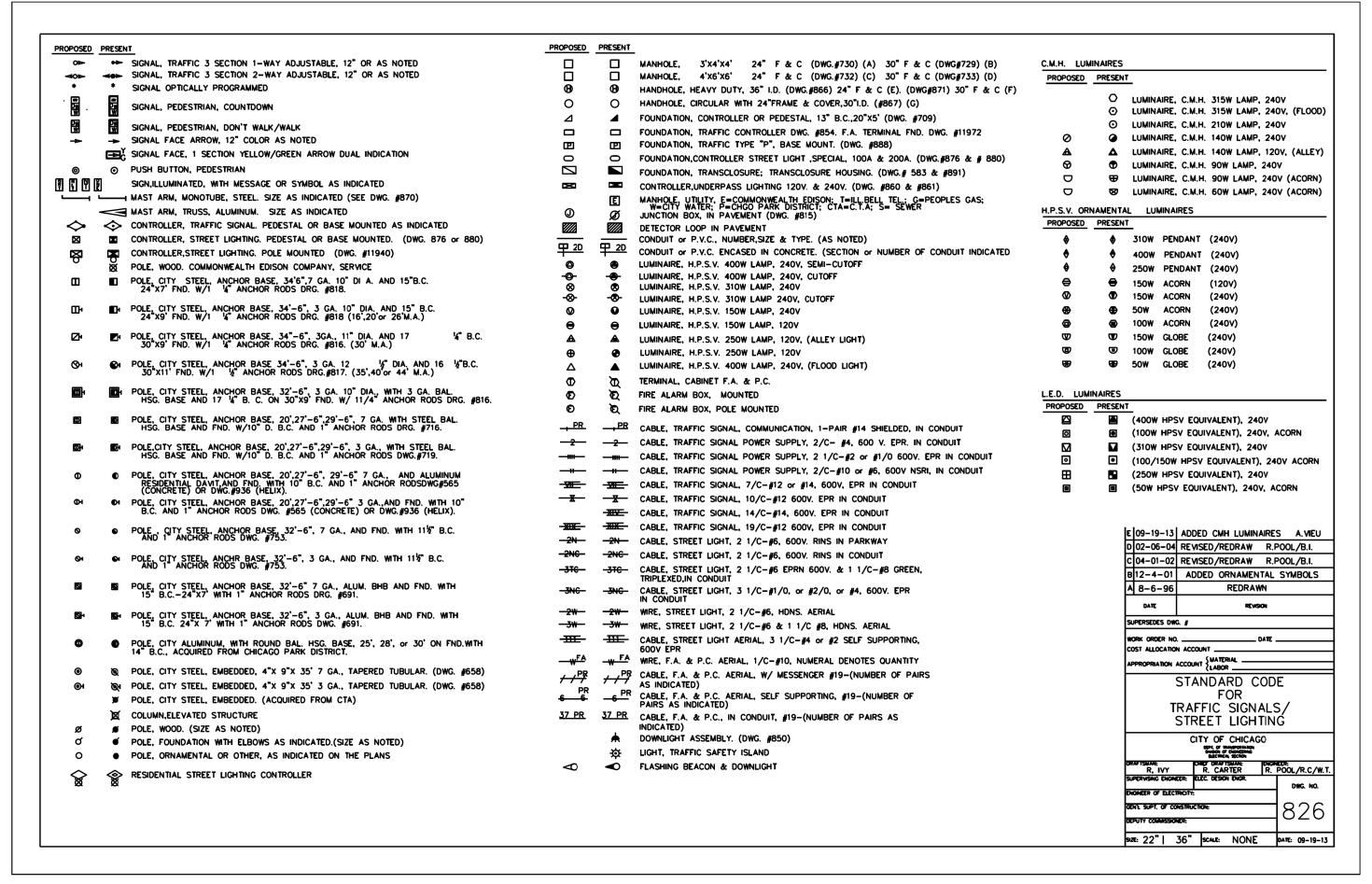
CHECKED — ECM REVISED —	•
PLOT SCALE = 1:2 DRAWN — YJL REVISED —	
PLOT DATE = 09/23/2020	

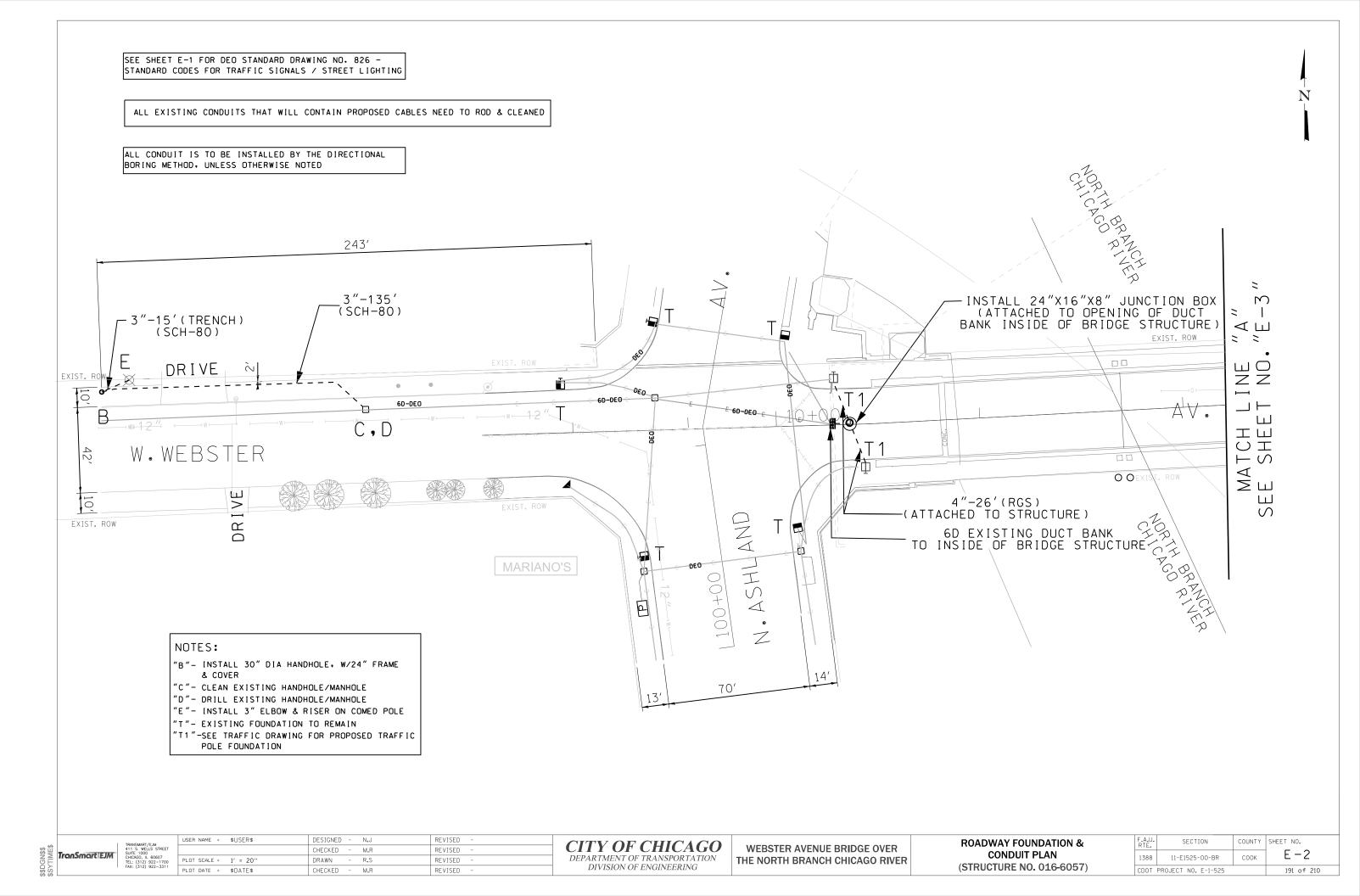
CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

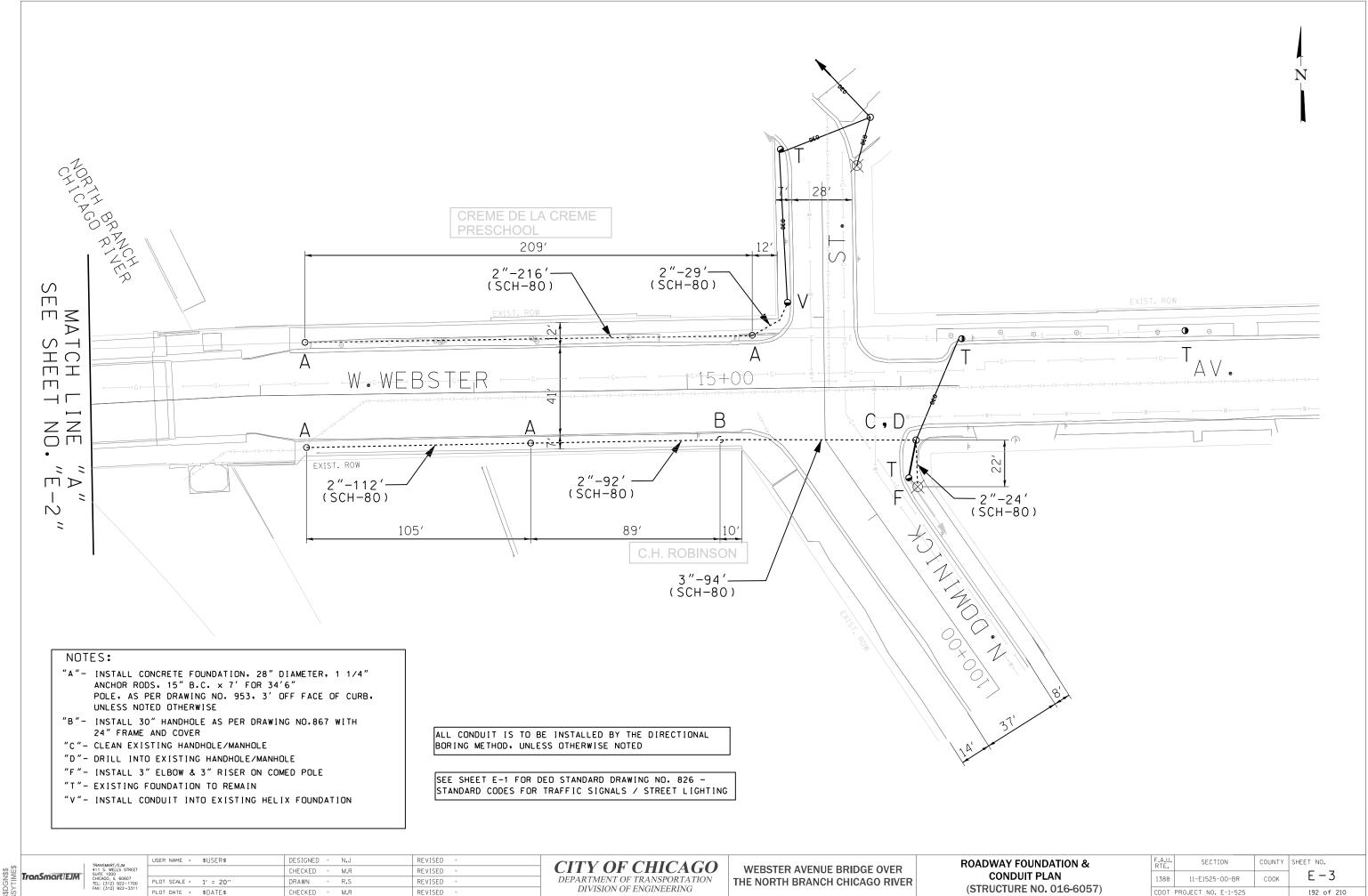
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER PRECAST CONCRETE RAILING PYLON AT BRIDGE HOUSE (STRUCTURE NO. 016-6057) F.A.U. SECTION COUNTY SHEET NO.

1388 11-E1525-00-BR COOK A-6.8

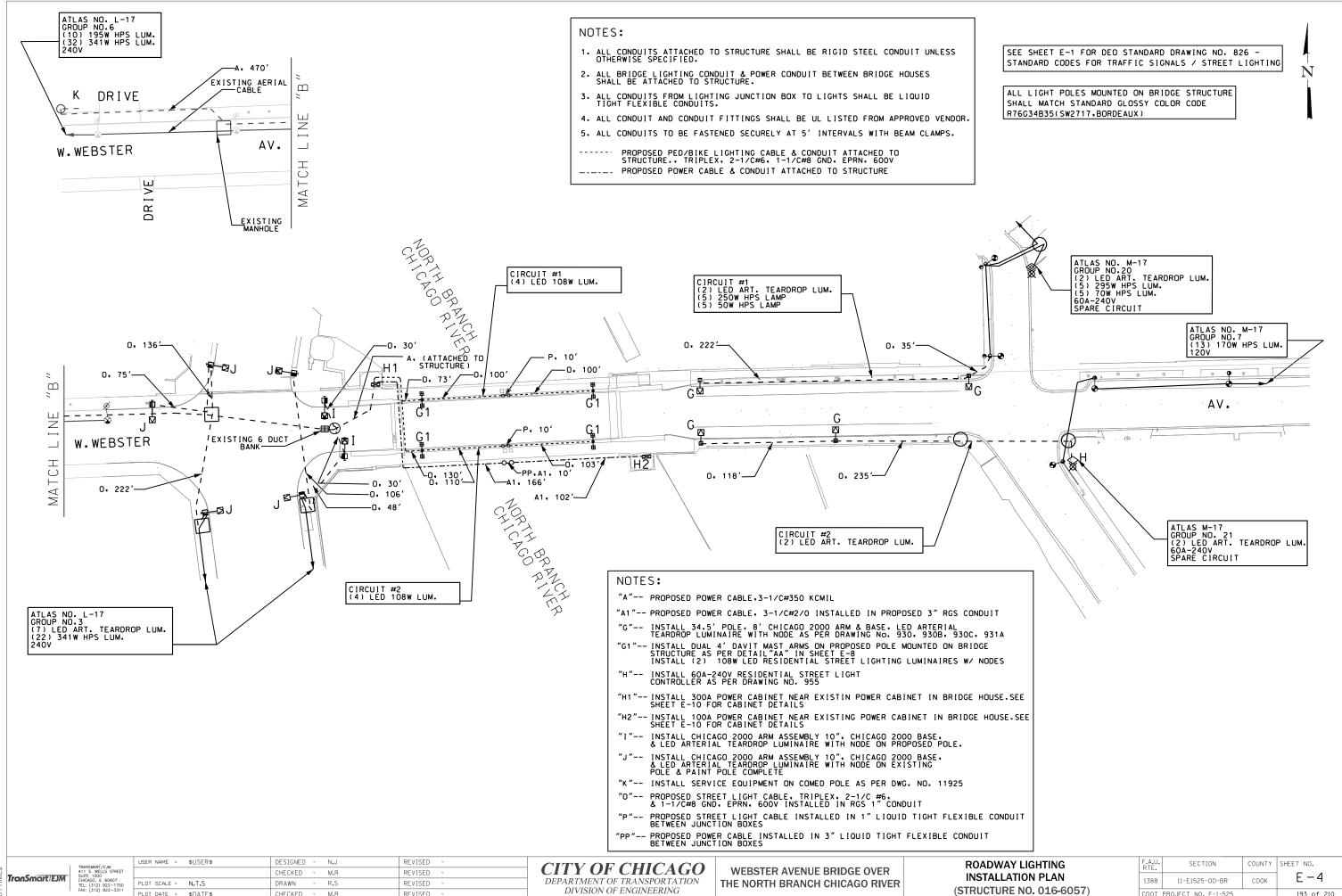
CDOT PROJECT NO. E-1-525 189 of 210



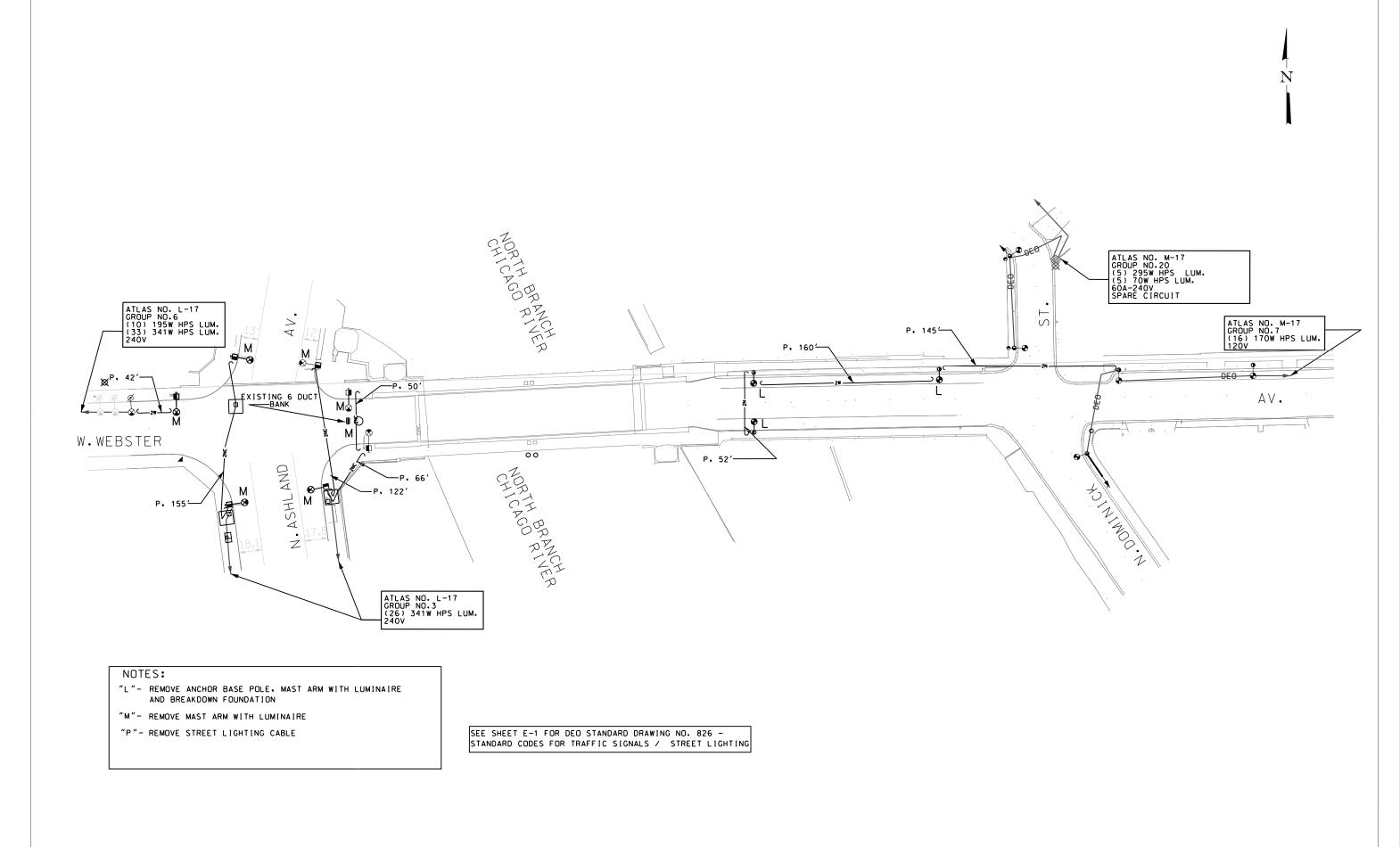




CDOT PROJECT NO. E-1-525



\$\$DGN\$\$



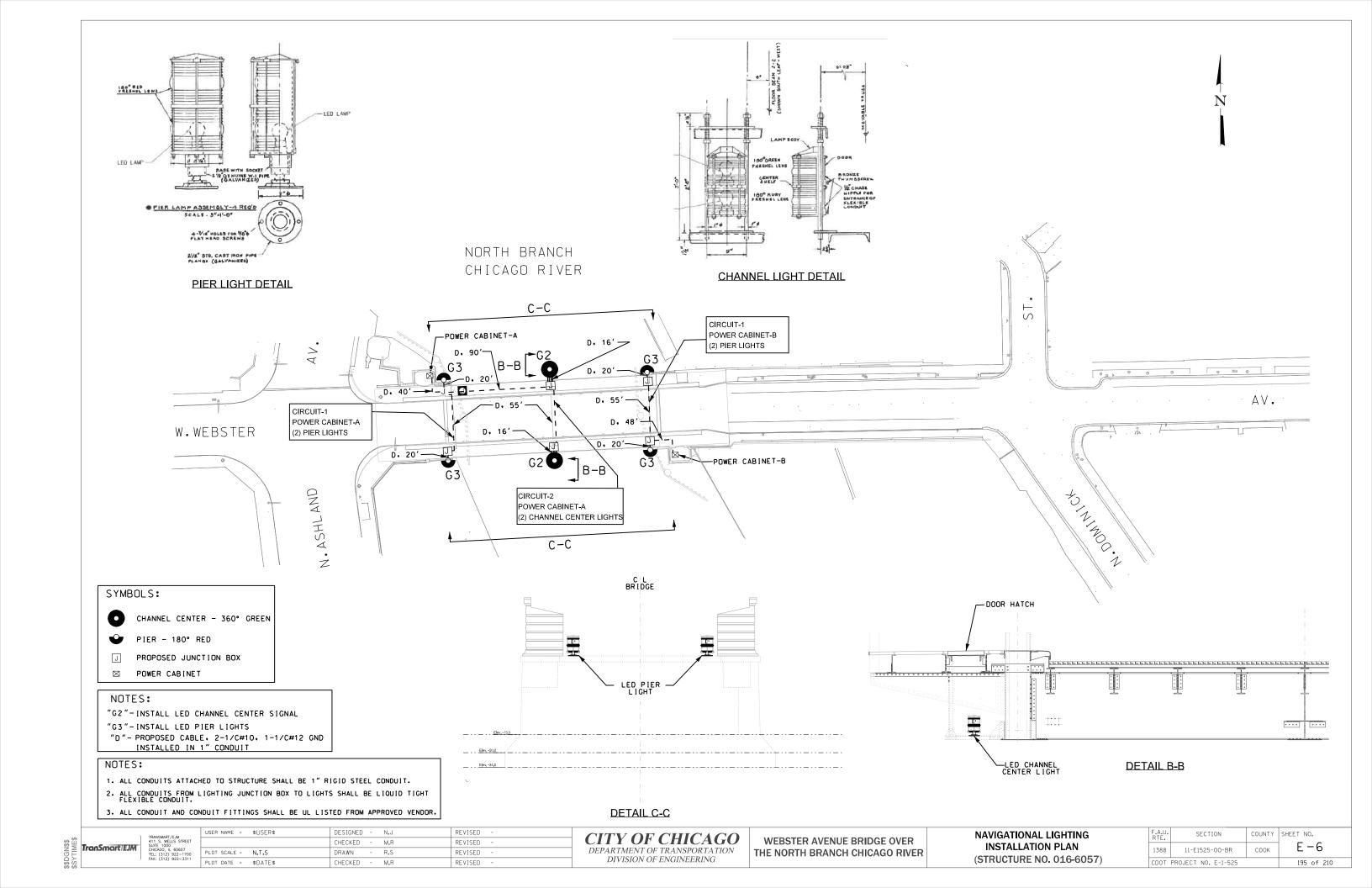
TranSmart/EJM

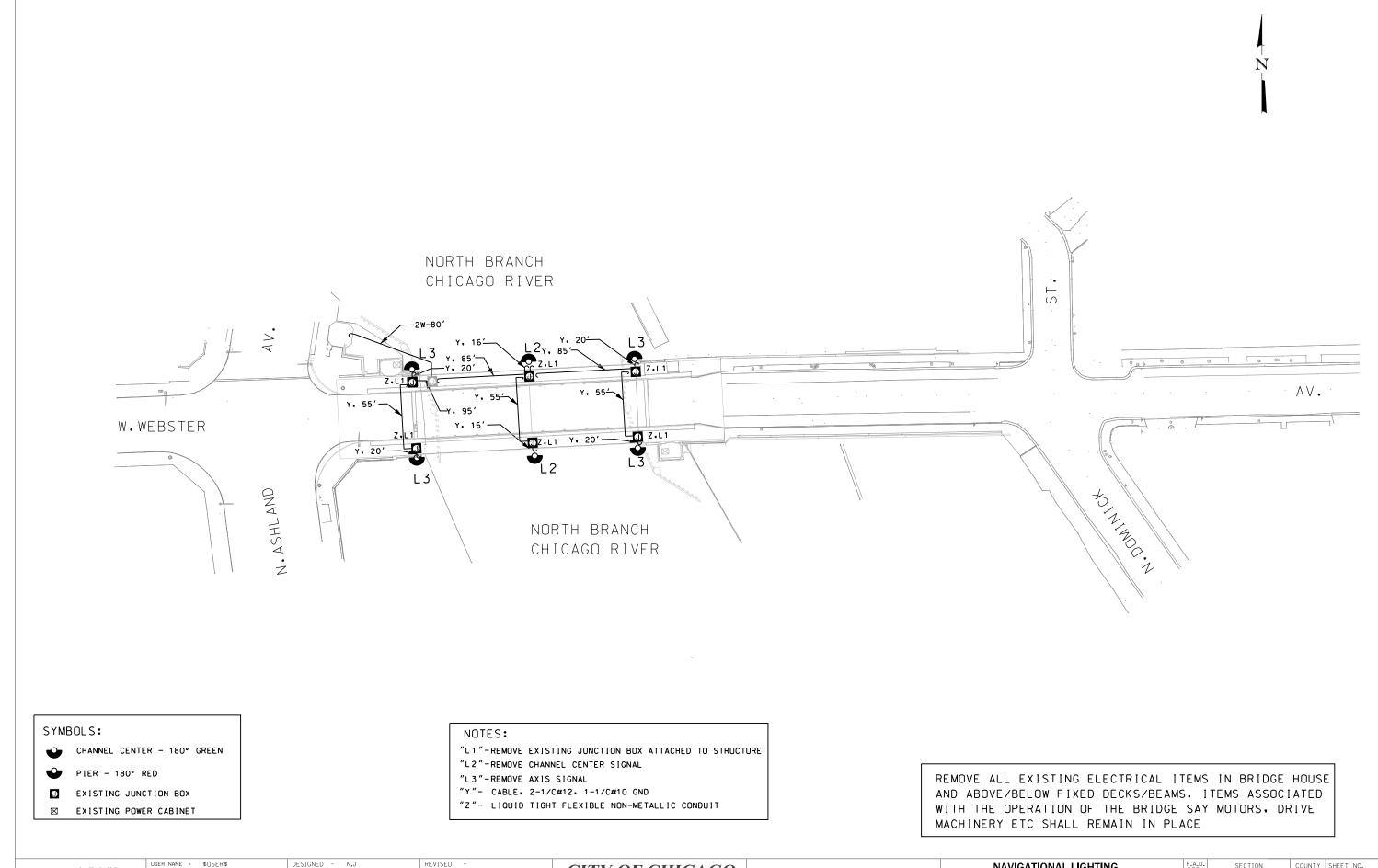
USER NAME = \$USER\$ DESIGNED REVISED REVISED CHECKED M.R PLOT SCALE = N.T.S DRAWN REVISED PLOT DATE = \$DATE\$ M.R REVISED CHECKED

CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER **ROADWAY LIGHTING REMOVAL PLAN** (STRUCTURE NO. 016-6057)

SECTION COUNTY SHEET NO. E – 5 11-E1525-00-BR CDOT PROJECT NO. E-1-525 194 of 210





\$\$DGN\$\$ \$SYTIME\$

Transmart/EJM RAME CHICA FAX: (

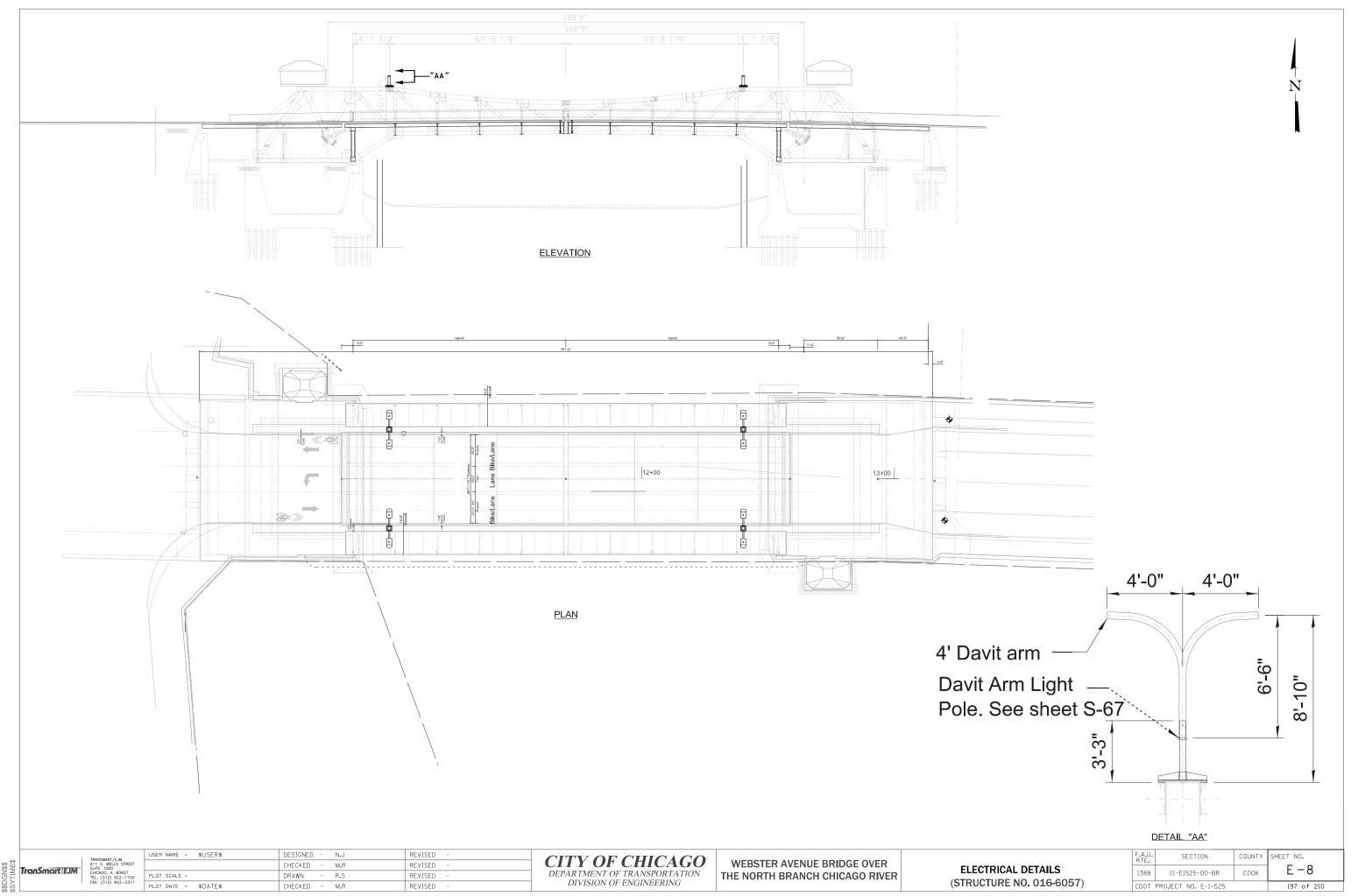
RANSMART/EJM 111 S. WELLS STREET JUITE 1000 CHICAGO, IL 60607 EL: (312) 922-1700 (AX: (312) 922-3311

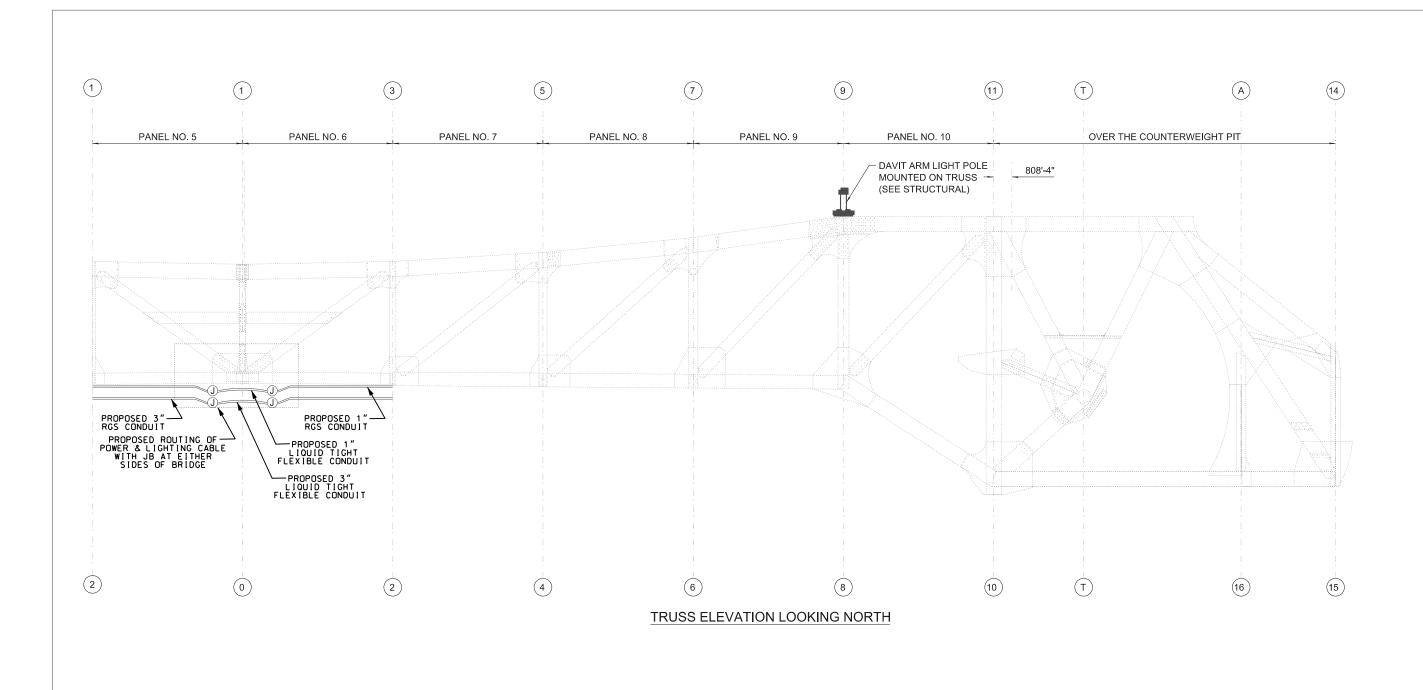
CITY OF CHICAGO

DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

NAVIGATIONAL LIGHTING REMOVAL PLAN (STRUCTURE NO. 016-6057)





TranSmart/EJM Sign

CITY OF CHICAGO
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

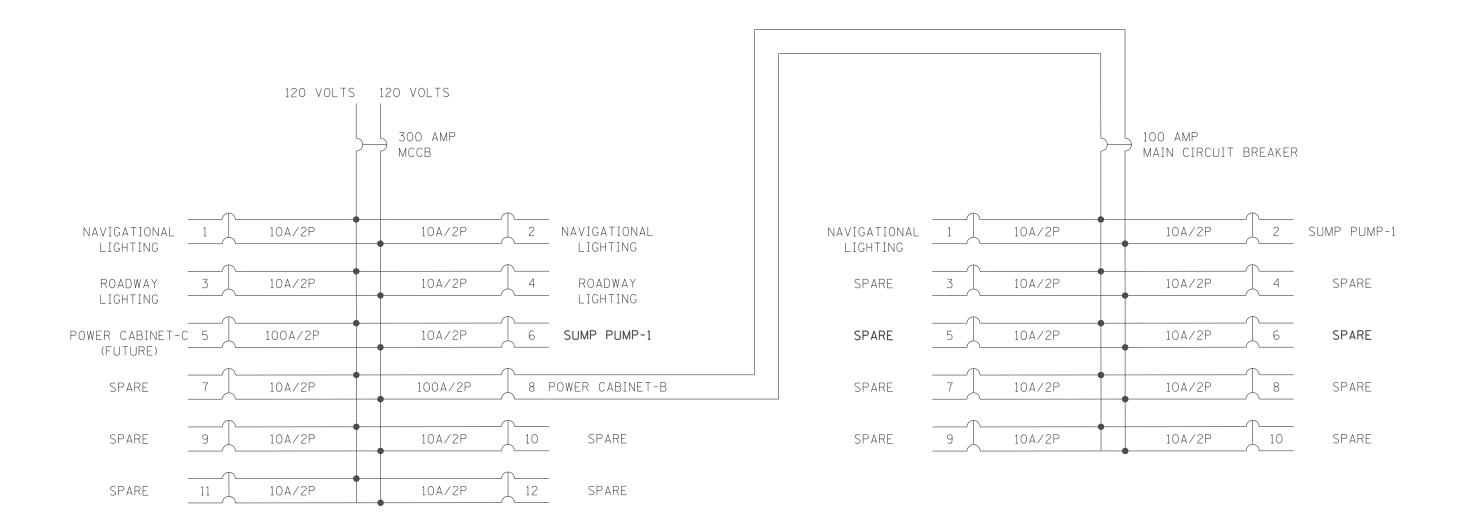
F.A.U. SECTION COUNTY SHEET NO.

1388 11-E1525-00-BR COOK E - 9

CDOT PROJECT NO. E-1-525 198 of 210

NOTES:

- 1. ALL ELECTRICAL PANELS SHALL BE 316L. SS. NEMA-4X. UL LISTED
- 2. ALL PANELS SHALL BE GROUNDED

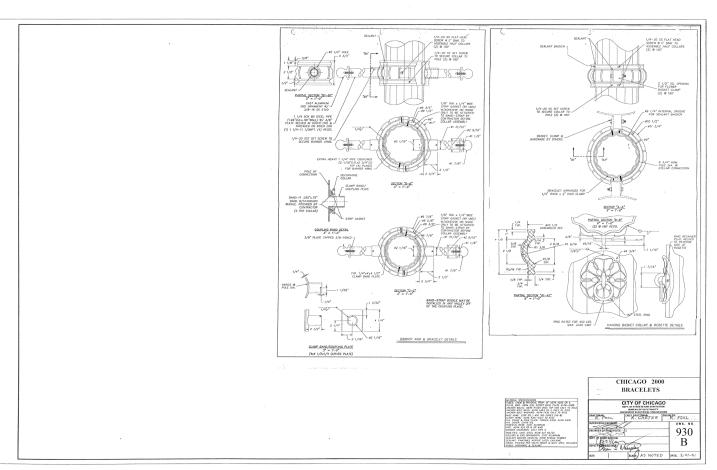


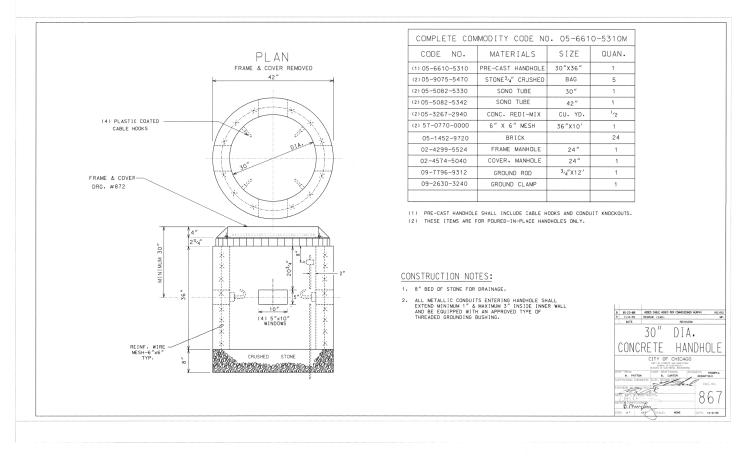
POWER CABINET-A BRIDGE HOUSE- 1 POWER CABINET-B BRIDGE HOUSE- 2

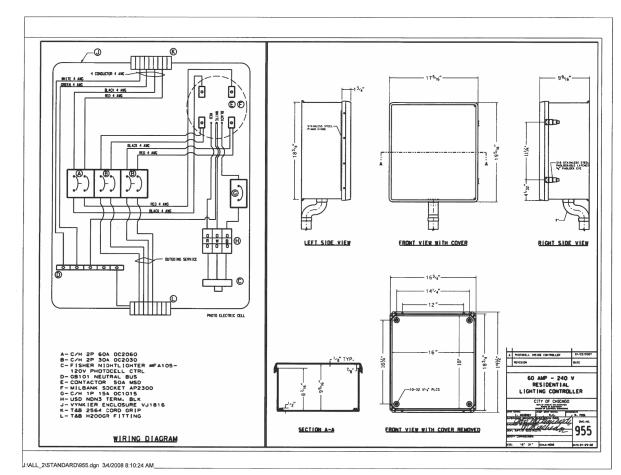
/TIME\$	TranSmart/EJM	
---------	---------------	--

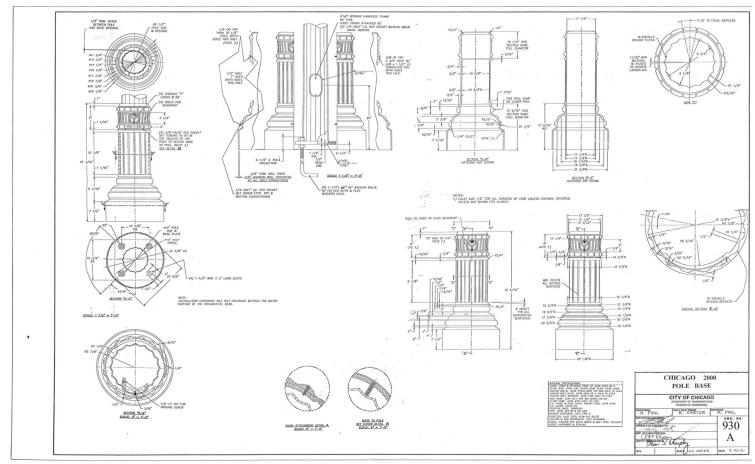
ART/EJM	USE
WELLS STREET 000	
), IL 60607 12) 922-1700	PLO
12) 922-3311	PLO

USER NAME = \$USER\$	DESIGNED - N.J	REVISED -	
	CHECKED - M.R	REVISED -	
PLOT SCALE =	DRAWN - R.S	REVISED -	D
PLOT DATE = \$DATE\$	CHECKED - M.R	REVISED -	









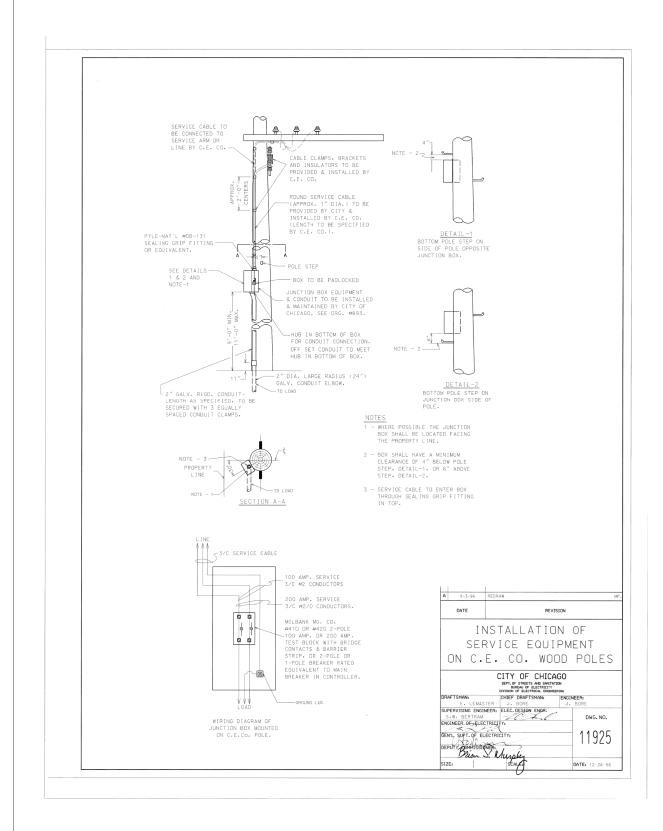
CITY OF CHICAGO

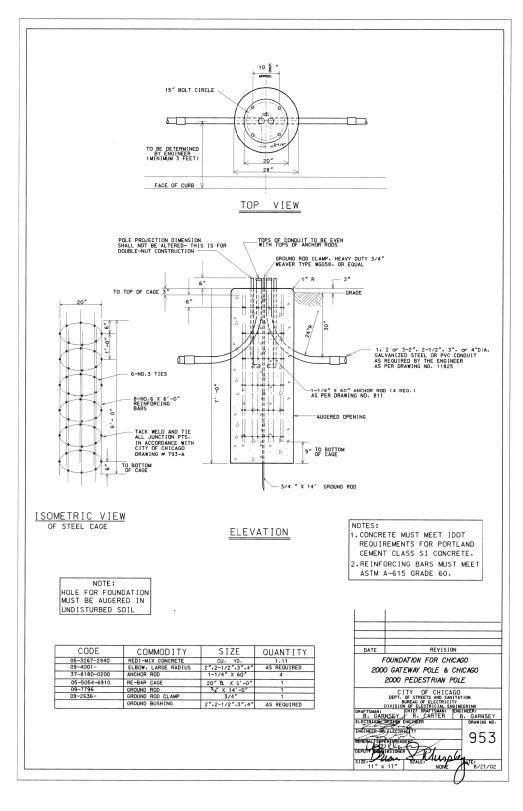
DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

ELECTRICAL DETAILS (STRUCTURE NO. 016-6057)

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	COOK	E-11
CDOT	PROJECT NO. E-1-525		200 of 210





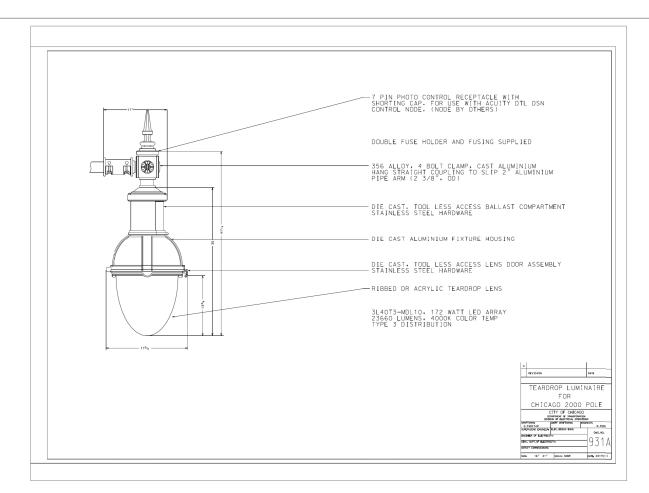
TranSmart/EJM

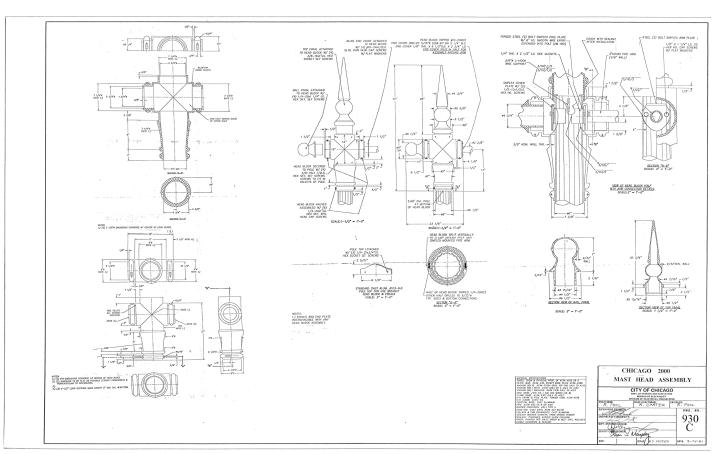
| USER NAME = \$USER\$ | DESIGNED - N.J | REVISED - | | CHECKED - M.R | REVISED - | | PLOT SCALE = N.T.S. | DRAWN - R.S | REVISED - | | PLOT DATE = \$DATE\$ | CHECKED - M.R | REVISED - | | |

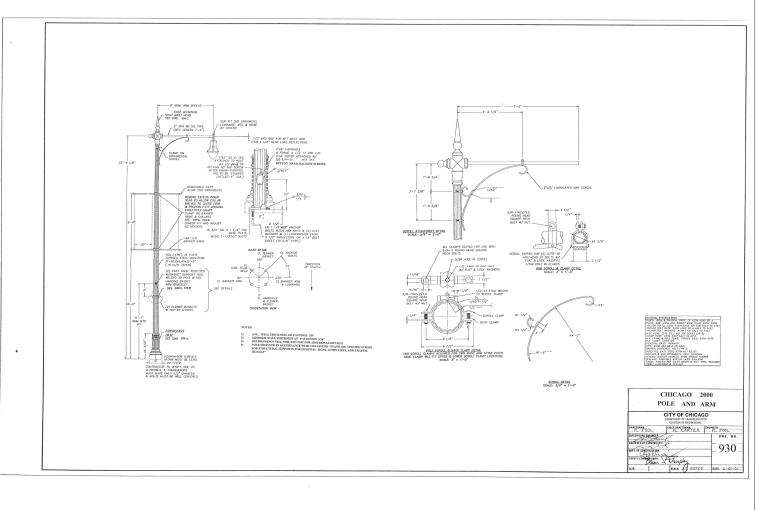
CITY OF CHICAGO

DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER







GENERAL MACHINERY NOTES

- 1. The Contractor shall perform all work in accordance with these Contact Drawings, the Contract Specifications, and all other Contract Documents as defined within the Specifications. Any reference to the specifications includes references to all supplemental specifications, special provisions, and specifications referenced herein.
- 2. The existing details, dimensions, and elevations shown on these plans have been obtained from record drawings and field measurements on the existing structure. The Contractor shall perform a field survey to verify all dimensions affecting fabrication or construction. Shop and construction drawings shall indicate field verified dimensions. Payment for completing the field survey shall be considered as included within the cost for fabrication of materials affected.
- 3. Details of machinery shall conform to the 2007 Standard Specifications for Movable Highway Bridges published by the American Association of State Highway and Transportation Officials, and all interim revisions. Welding shall be in accordance with AWS Bridge Welding Code AASHTO/AWS D1.5.M/D1.5: 2015
- 4. Materials: The following items shall be of the materials specified and conformed to the following ASTM code provisions unless otherwise noted:

Weldment and Plates: ASTM A709 Grade 50.
Structural steel supports for mechanical components: ASTM A709 Grade 50.

- 5. Provide ASTM A449 H.S. (High strength) turned bolts as required to connect machinery to structural steel. All H.S. turned bolts shall have an ANSI B4.1 LC6 clearance between the body of the bolt and the hole. All H.S. Fasteners shall have a hardened plain washer under the head and the nut. New ASTM A449 bolts shall not be torqued more than once. Replacement of turned bolts shall be of the same nominal size as existing except as shown. Bolt area and bolt holes shall be cleaned by a wire brush before new bolt installation.
- 6. All H.S. fasteners shall have nuts conforming to ASTM A563. All nuts shall be secured by effective locks. If double nuts are used, both nuts shall be of the same thickness unless otherwise noted. All H.S. fasteners shall have a hardened plain washer under the head and the nut. All hardened steel plain washers shall conform to ASTM F436.
- 7. Provide type 316 stainless steel shims for leveling and aligning all machinery components. Shims shall be $\frac{1}{2}$ inch nominal thickness, unless otherwise specified, with adjustment variations and described in the Specifications. All shims to be full footprint of machinery component. U-shaped or slotted shims are not permitted.
- 8. Machinery dimensions shown on drawings are dimensions after machining.
- All machinery support surfaces shall be flat, level, and parallel to each other and the mounting base plate. Thickness of mounting plates given are for after finishing. Machinery supports shall be machined after welding and stress relief to provide a uniform mounting surface.
- 10. All dimensions for machine finished surfaces shall be held to 0.01 inch except as otherwise required, shown on the plans, by Specifications or as directed by the Engineer.
- 11. Fits and finishes for machinery shall be as follows:

Surface	Fit (Per ANSI B4.1)	Finished (Microinches)
Machinery Parts in Fixed Contact Shaft Journal Journal Bushings Solid Bushing in Base (To 1/4" Wall) Solid Bushing in Base (Over 1/4" Wall) Hubs on Shafts (To 2" Bore) Hubs on Shafts (Over 2" Bore) Split Bushing in Base Sliding Bearings Keys and Keyways Shafts Turned Bolts in Finished Holes	- RC6 RC6 FN1 FN2 FN2 LC1 RC6 LC4 - LC6	125 8 16 63 63 32 63 125 32 63 63

The above fits for cylindrical parts shall also apply to the dimensions of non-cylindrical parts.

GENERAL MACHINERY NOTES (CONTINUED)

- 12. The Contractor shall perform all work with care such that any materials that are to remain in place, that are to be re-used, or that are to remain the property of the City of Chicago will not be damaged. If the Contractor damages any such materials, the damaged materials shall be repaired or replaced in a manner satisfactory to the Engineer, at no additional cost to the City.
- 13. Where new steel is to be connected to existing steel, the existing surfaces shall be cleaned to bare steel of all paint, loose rust, and other foreign material, then, painted with one coat of primer prior to the installation of new material. Existing paint shall be cleaned from all areas within 2 inches of high strength bolts. The cost for this cleaning shall be included in the cost for installation of new material.
- 14. The existing machinery components coating contains lead. The Contractor shall take appropriate precautions to deal with the present of lead on this project in accordance with the Mechanical Equipment Detailed Specification.
- 15. The Contractor shall submit to the Engineer final design, drawings, and design calculations of temporary access, construction platforms, and temporary protective shields
- 16. All weldments shall be stress relieved after welding and before machining.
- 17. The Contractor shall submit sketches showing the method of bringing up or lowering the materials. The Contractor shall propose locations of support or hanging of the lifting equipment. The Contractor shall obtain the Engineer's approval for these location and connection details.
- 18. Drawings shall be fully dimensioned and shall show existing members, drawings, and calculations shall each bear the signature and raised, embossed seal of the designer, who shall be a Licensed Professional Engineer in the State of Illinois. Drawings shall conform to the requirements stated in the Specifications.
- 19. The Contractor shall not disturb any existing utilities except as specifically defined within the scope of work for this Contract. Where work affects or is affected by the existing utilities, the work shall be coordinated with the Department.

PAY ITEM LIST AND SCOPE OF WORK

- ITEM 156 CLEANING, PAINTING, AND LUBRICATING OPERATING MACHINERY ASSEMBLIES
 - Clean, paint, and lubricate all existing span drive operating machinery assemblies and associated components as accessibility allows and per direction from Commissioner.
- ITEM 157 REPLACEMENT OF CENTER LOCKS
- 2 Disassemble and remove existing center lock assemblies and all associated components.
- 3 Furnish and install new center lock assemblies.
- ITEM 158 FURNISH AND INSTALL NEW SUMP PUMPS
 - 4 Disassemble and remove existing sump pump assemblies and all
 - 5 Furnish and install new sump pump assemblies and associated components.
 - Connect to Drainage System, see special provisions.
 Drainage System to exit out back to abutment.
 Drainage System to be connected to catch basin
 (by others).

INDEX OF MECHANICAL DRAWINGS

SHEET NO. SHEET TITLE

M – 1	GENERAL MACHINERY NOTES	
M-2	GENERAL PLAN AND ELEVATION	

M-3 EXISTING CENTER LOCK DEMOLITION PLAN

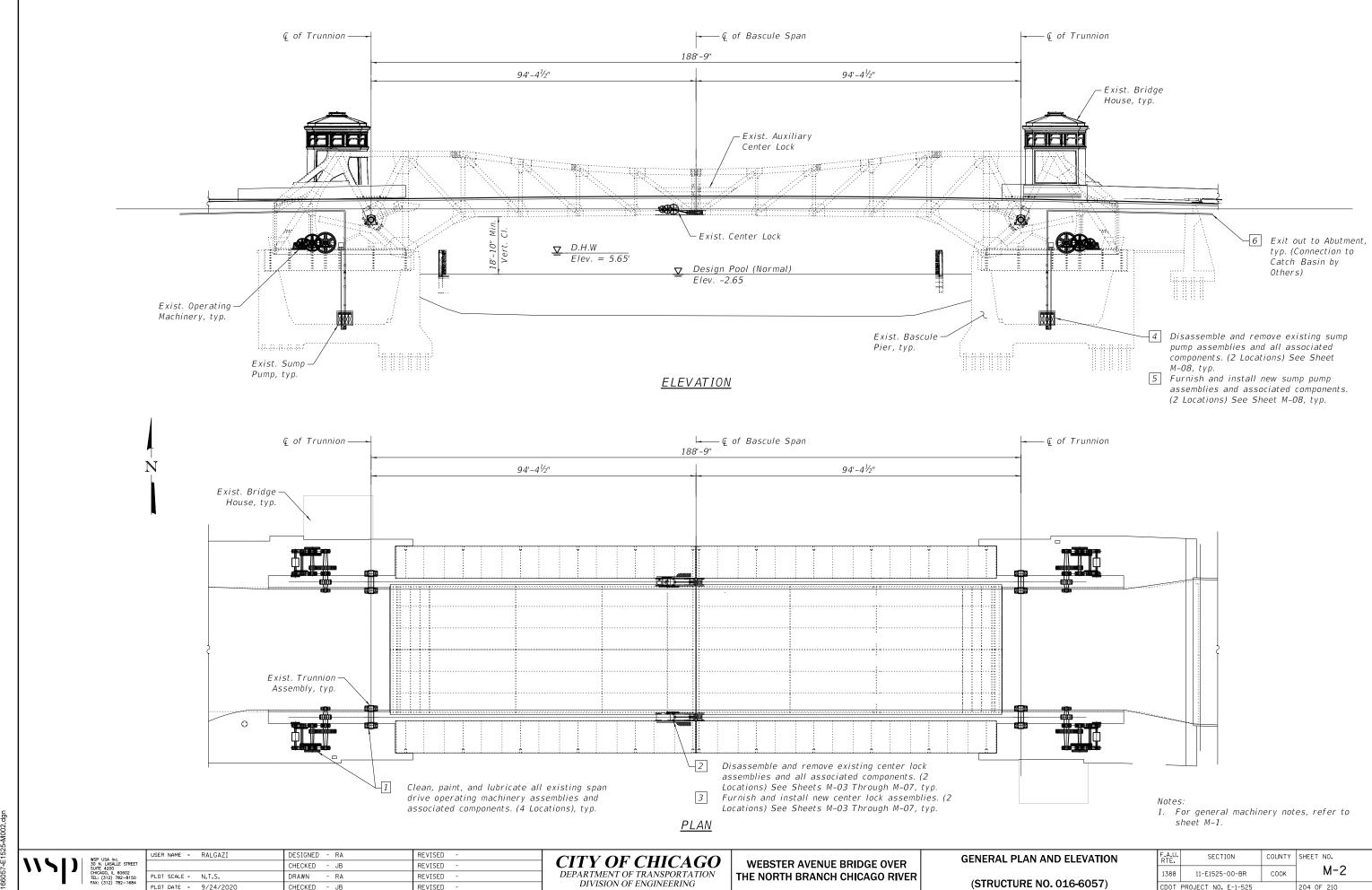
M-4 NEW CENTER LOCK ASSEMBLY
M-5 NEW CENTER LOCK DETAILS 1

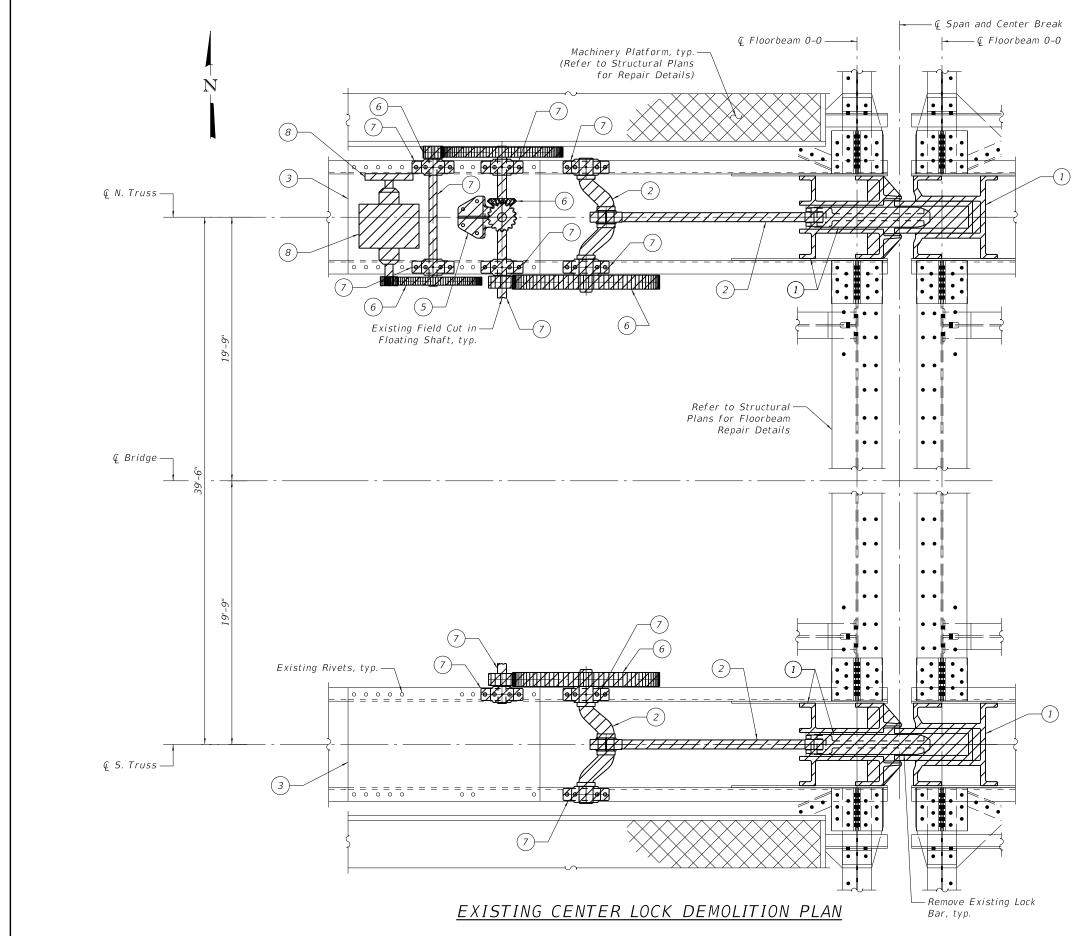
M-6 NEW CENTER LOCK DETAILS 2

M-7 NEW CENTER LOCK DETAILS 3

M-8 NEW SUMP PUMP ASSEMBLY

IOTES	F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
	1388	11-E1525-00-BR	соок	M-1
057)	CDOT	PROJECT NO. E-1-525		203 OF 210





SCOPE OF WORK FOR CENTER LOCK DEMOLITION:

Remove and dispose of all existing center lock equipment and associated supports. Demolition shall include but not limited to the following.

- 1 All lock bars, guides, and receiver assemblies and all associated components.
- All lock bar cranks, actuating linkages assemblies, and associated components.
- (3) All machinery supports.
- 4 All limit switch assemblies.
- All manual drive mechanisms, associated components, and associated supports.
- 6 All spur gears, reducers, shafts, and keys.
- All transverse line shafts, associated bearings, and couplings
- All electrical motors, brakes, and all associated electrical components.
- All lubrication lines and associated components.

LEGEND:

Remove and Discard

NOTES:

- For general machinery notes, see sheet M-1.
- Contractor shall field verify all dimensions.
- All removals and disposals shall be in accordance with all city, state, and federal regulations.
- 4. Machinery equipment contains lubricating oil or grease. Removals and disposals shall be in accordance with all city, state, and federal regulations.
- 5. All existing lock bar guides and receivers are bolted and tack welded to the bascule truss. Guide and receiver removal will require breaking welds and unbolting the guides and receivers. All tack welds shall be ground smooth and flush with the surface of the bascule truss.
- 6. Existing structural support to remain unless otherwise noted.
- 7. Refer to the Mechanical Equipment Detailed Specifications for Demolition Requirements.

REFERENCE DRAWINGS

<u>Drawing</u>

Center Lock Machinery Details of Lock and Lock Indicator

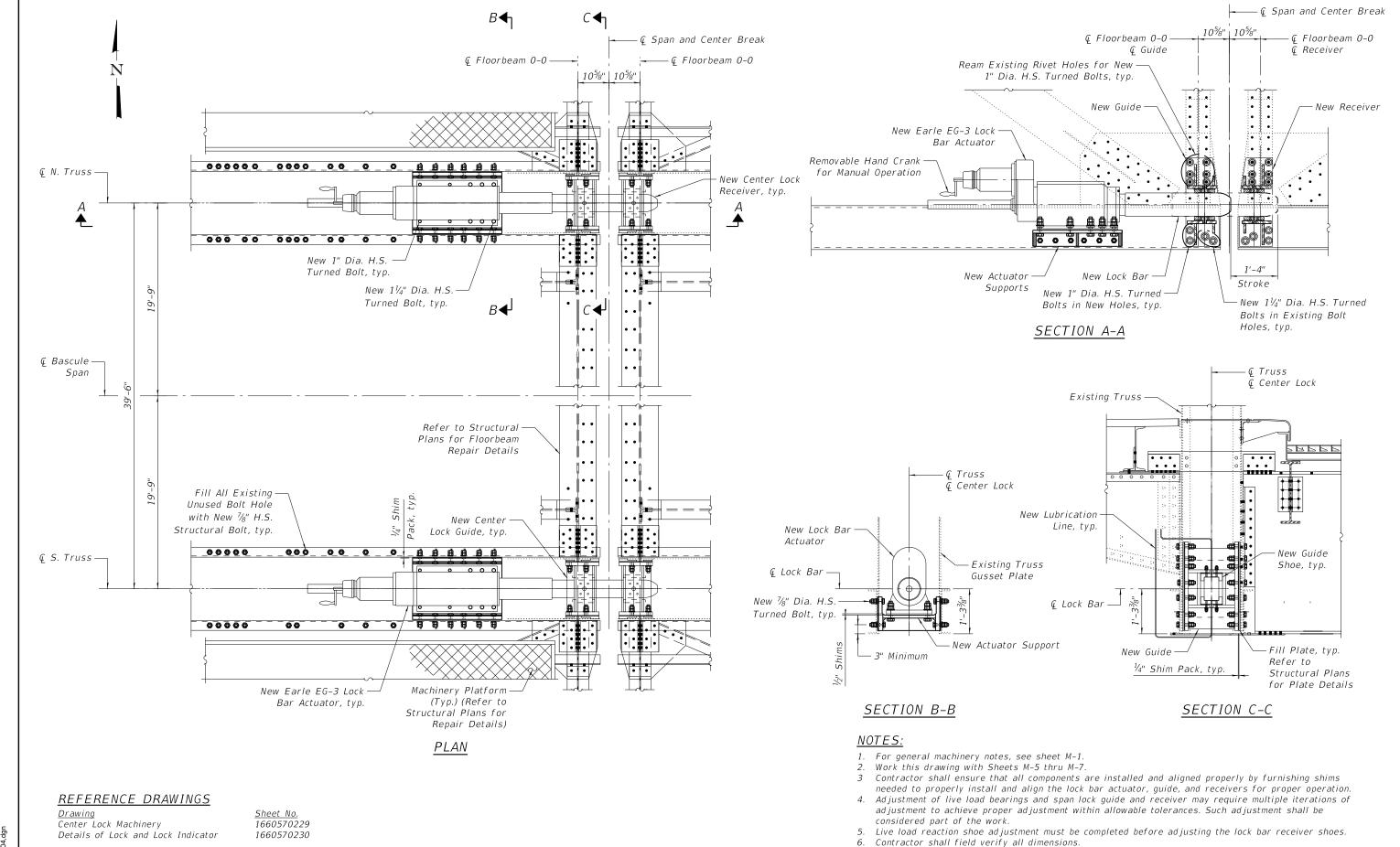
Sheet No. 1660570229 1660570230

USER NAME = RALGAZI DESIGNED - RA REVISED CHECKED - JB REVISED DRAWN - RA REVISED PLOT DATE = 9/24/2020 CHECKED - JB REVISED

CITY OF CHICAGO DEPARTMENT OF TRANSPORTATION DIVISION OF ENGINEERING

WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER **EXISTING CENTER LOCK DEMOLITION PLAN** (STRUCTURE NO. 016-6057)

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	M-3
CDOT	PROJECT NO. E-1-525		205 OF 210



0166057-E1525-M004.dgn

WSP USA Inc.
30 N. LASALLE STREET
SUITE 4200
CHICAGO, IL 60602
TEL: (312) 782-8150
FAX: (312) 782-1684

CITY OF CHICAGO

DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING

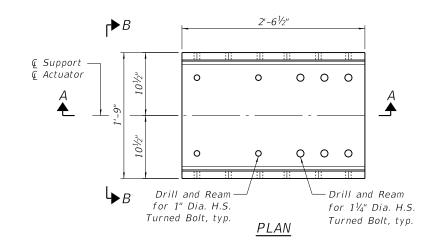
WEBSTER AVENUE BRIDGE OVER THE NORTH BRANCH CHICAGO RIVER

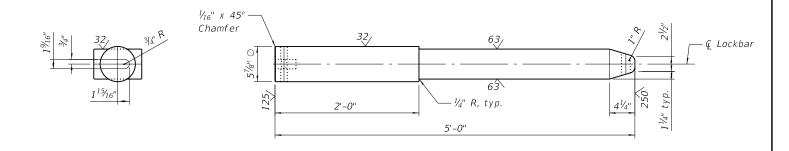
NEW CENTER LOCK ASSEMBLY
(STRUCTURE NO. 016-6057)

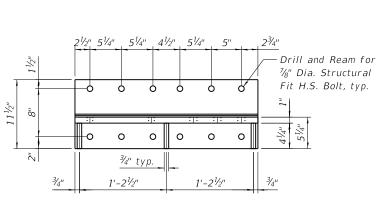
F.A.U. SECTION COUNTY SHEET NO.

1388 11-E1525-00-BR COOK M-4

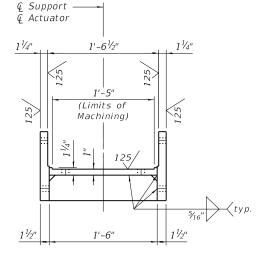
CDOT PROJECT NO. E-1-525 206 0F 210



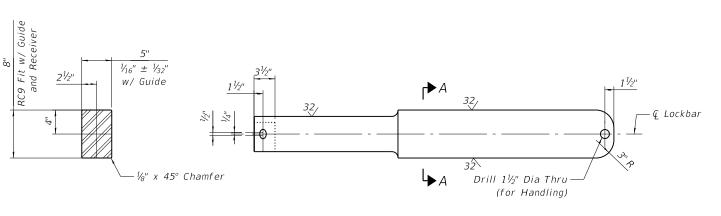




SECTION A-A



VIEW B-B



NEW ACTUATOR SUPPORT Material: ASTM A709, Grade 50

SECTION A-A

NEW LOCKBAR

Material: ASTM A668, Class M Forging Quenched and Tempered to 290–320 BHN.

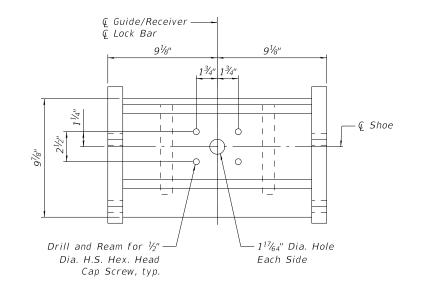
NOTES:

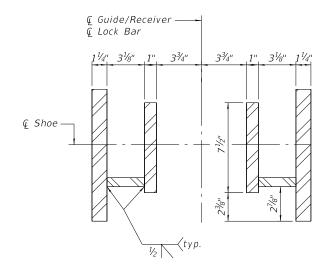
- 1. For general machinery notes, see sheet M-1.
- 2. Work this drawing with sheets M-4 thru M-7.
- Contractor shall field verify all dimensions.
 Coat machined surfaces of lock bar that does not contact the guide and receiver with anti-corrosion compound after installation.

			_
		WSP USA Inc.	L
115)	30 N. LASALLE STREET SUITE 4200	Г
• • •	_	CHICAGO, IL 60602 TEL: (312) 782-8150	Р
•		FAX: (312) 782-1684	Ρ

USER NAME = RALGAZI	DESIGNED - RA	REVISED -
	CHECKED - JB	REVISED -
PLOT SCALE = N.T.S.	DRAWN - RA	REVISED -
PLOT DATE = 9/24/2020	CHECKED - JB	REVISED -

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	M-5
CDOT	PROJECT NO. E-1-525		207 OF 210

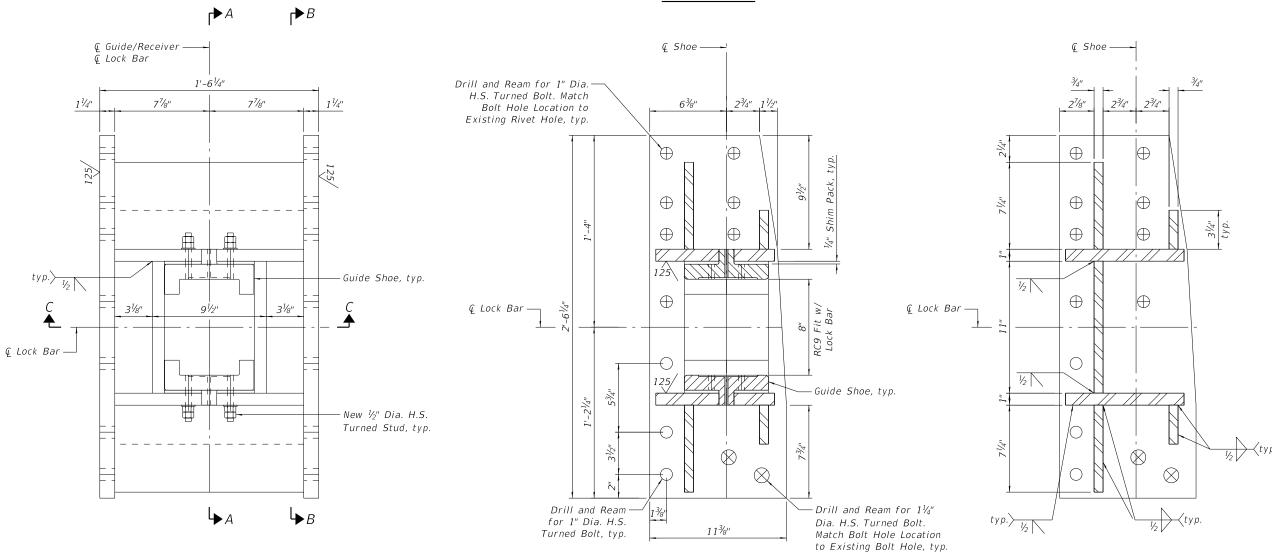




NOTES:

- 1. For general machinery notes, see sheet M-1.
- 2. Work this drawing with sheets M-4 thru M-7.
- 3. All welds not shown shall be $\frac{1}{2}$ " continuous fillet weld with no open joints. Stress relieve after welding.
- 4. Stress relieve all weldments prior to machining.
- 5. Contractor shall field verify all dimensions.

SECTION C-C



CENTER LOCK GUIDE AND RECEIVER

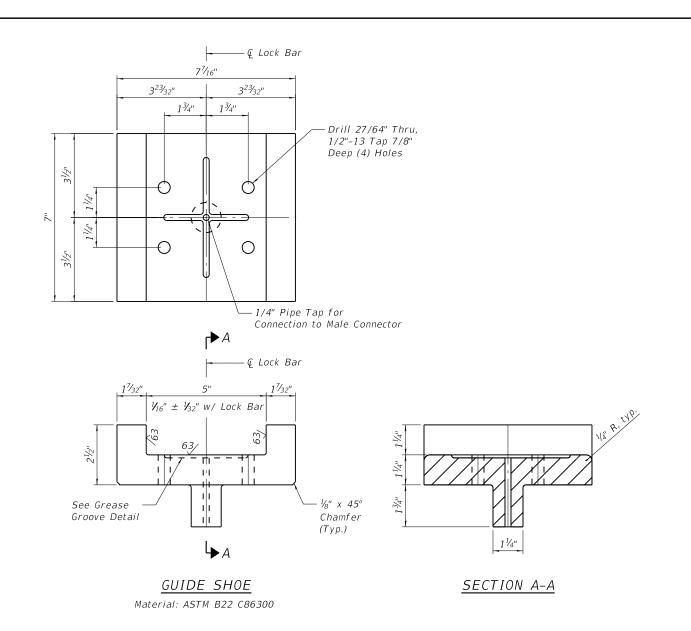
Material: ASTM A709 Gr. 50 Guide Shown, Receiver Similar SECTION A-A

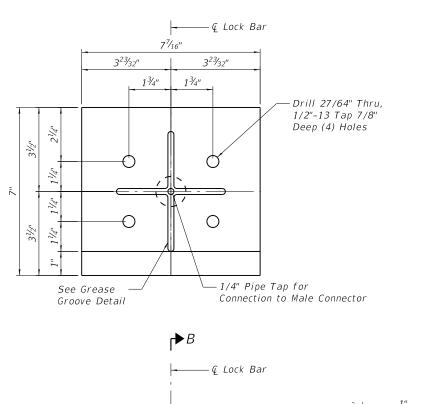
SECTION B-B

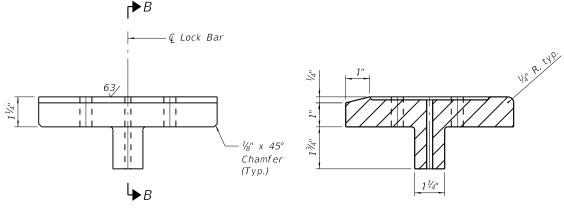


USER NAME = RALGAZI	DESIGNED - RA	REVISED -
	CHECKED - JB	REVISED -
PLOT SCALE = N.T.S.	DRAWN - RA	REVISED -
PLOT DATE = 9/24/2020	CHECKED - JB	REVISED -

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	M-6
CDOT	PROJECT NO. E-1-525		208 OF 210



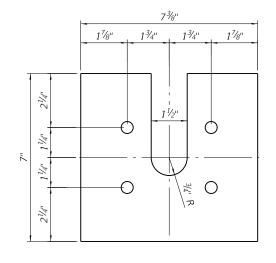




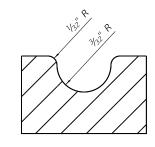
RECEIVER SHOE

Material: ASTM B22 C86300

SECTION B-B



SHIM PACK DETAIL



GREASE GROOVE DETAIL

TABLE OF CENTER LOCK COMPONENTS

Item	Quantity
Earle EG-3 Linear Actuator w/ Manual Hand Crank, 5 HP Motor, 1'-6" Stroke Manufactured by Steward Machine Co. (Or Approved Equal)	2
Lock Bar	2
Guide	2
Receiver	2
Actuator Support	2
Guide Shoe	4
Receiver Shoe	4

NOTES

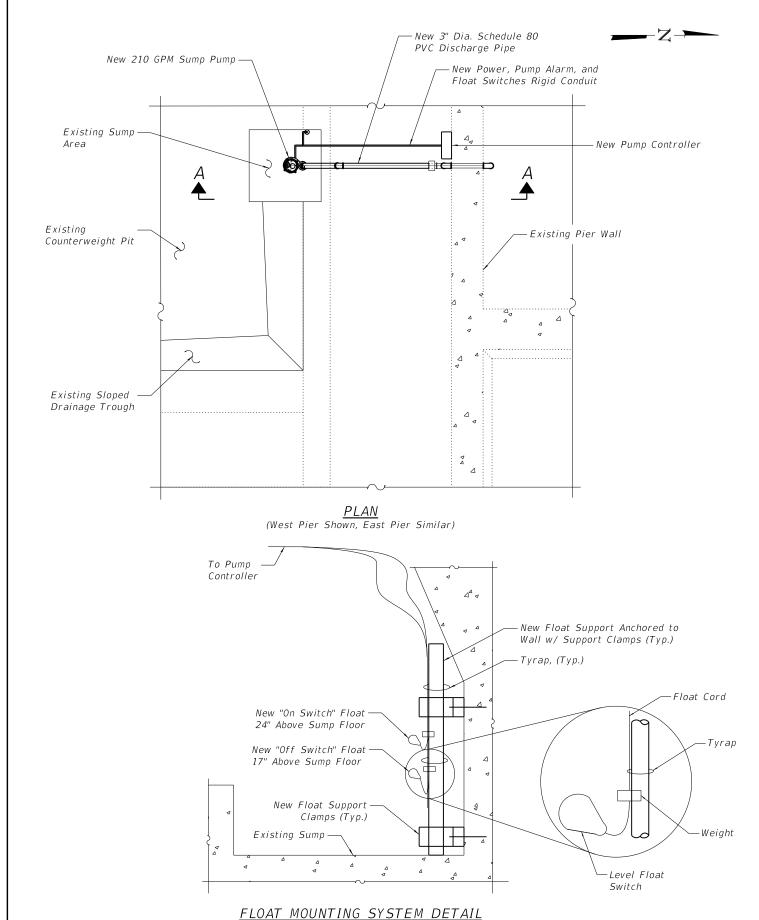
- 1. For general machinery notes, see sheet M-1.
- 2. Work this drawing with sheets M-4 thru M-7.
- Contractor shall field verify all dimensions.
 Refer to the Mechanical Equipment Detailed
- 4. Refer to the Mechanical Equipment Details Specifications for shim pack material and thickness variation requirements.

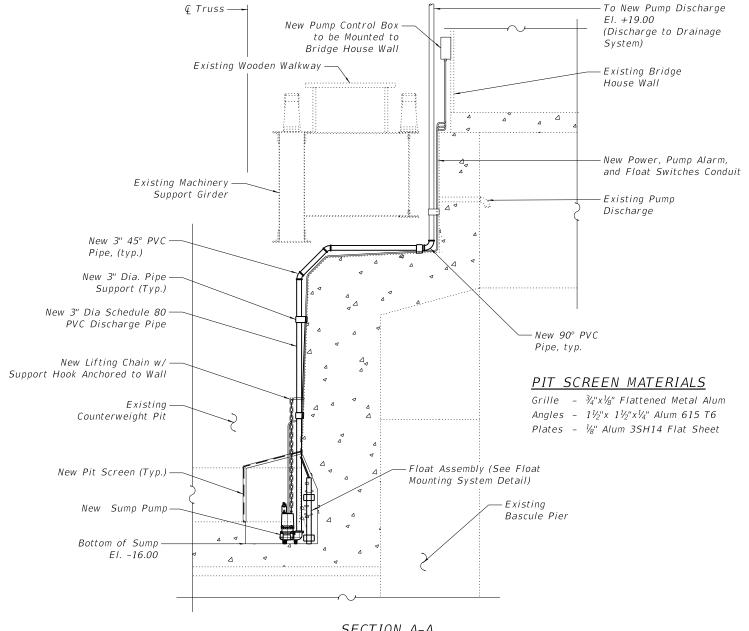
(See Note 4)

USER NAME = RALGAZI	DESIGNED - RA	REVISED -
	CHECKED - JB	REVISED -
PLOT SCALE = N.T.S.	DRAWN - RA	REVISED -
PLOT DATE = 9/24/2020	CHECKED - JB	REVISED -

l	F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
Ī	1388	11-E1525-00-BR	соок	M-7
	CDOT	PROJECT NO. E-1-525		209 OF 210

WSP USA Inc. 30 N. LASALLE STREET SUITE 4200 CHICAGO, IL 60602 TEL: (312) 782-8150 FAX: (312) 782-1884





SECTION A-A Typical for 2

PUMP SCHEDULE

Flow (GPM) Location Voltage Phase H.P. RPMТуре MFG., Model* Submersible, Non-Clog Liberty Pumps LEH202M3 2 210 East Pier 230 3450 Submersible, Non-Clog Liberty Pumps LEH202M3 210 230 West Pier

NOTES

- 1. For general machinery notes, see Sheet M-1
- 2. Contractor shall field verify all dimensions before beginning fabrication and installation. All field measurements shall be indicated on the shop drawings.
- 3. See the Mechanical Equipment Detailed Specifications for sump pump requirements.
- 4. The materials for all anchor bolts, nuts, washers, and lifting chain shall be 316 stainless steel.
- 5. All float switches shall be tied separately to support pipe.
- 6. All support clamps, pipe clamps, and bracket holders for all discharge and float mounting pieces shall be stainless steel.
- 7. Provide all support clamps, bracket holders as necessary.
- 8. Provide openings in pit screen for all float power and float cords, discharge pipes, float support pipes, and lifting chain.
- 9. Ensure all new piping and conduit do not interfere with the bascule span in any position.



USER NAME = RALGAZI	DESIGNED - DP	REVISED -
	CHECKED - RA	REVISED -
PLOT SCALE = N.T.S.	DRAWN - DP	REVISED -
PLOT DATE = 10/2/2020	CHECKED - RA	REVISED -

NEW SUMP PUMP ASSEMBLY	
(STRUCTURE NO. 016-6057)	-

F.A.U. RTE.	SECTION	COUNTY	SHEET NO.
1388	11-E1525-00-BR	соок	M-8
CDOT	PROJECT NO. E-1-525		210 OF 210

^{*} Or Approved Equal